BEFORE THE OHIO POWER SITING BOARD

)

)

)

)

In the Matter of the Application of Kensington PV 1, LLC for a Certificate of Environmental Compatibility and Public Need

Case No. 21-764-EL-BGN

NOTICE OF FILING RESPONSES TO STAFF DATA REQUESTS

On October 19, 2021, Kensington PV 1, LLC ("Kensington") filed an Application for a Certificate of Environmental Compatibility and Public Need with the Ohio Power Siting Board (the "Board"). On November 3, 2021, November 29, 2021, November 30, 2021, December 1, 2021, December 27, 2021, February 1, 2022, and April 28, 2022, Board Staff sent data requests to Kensington. Attached to this notice are copies of Kensington's responses, previously submitted to the Board's Staff via electronic mail.

Respectfully submitted,

/s/ Anna Sanyal Michael J. Settineri (0073369), Counsel of Record Anna Sanyal (0089269) Vorys, Sater, Seymour and Pease LLP 52 E. Gay Street Columbus, Ohio 43215 614-464-5462 614-719-5146 (fax) mjsettineri@vorys.com aasanyal@vorys.com (Each is willing to accept service via email)

Attorneys for Kensington PV 1, LLC

CERTIFICATE OF SERVICE

The Public Utilities Commission of Ohio's e-filing system will electronically serve notice of the filing of this document on the parties referenced on the service list of the docket card who have electronically subscribed to the case. In addition, the undersigned certifies that a courtesy copy of the foregoing document is also being served (via electronic mail) on May 3, 2022 upon all persons/entities listed below:

Thomas Lindgren thomas.lindgren@ohioattorneygeneral.gov

Thomas Shepherd thomas.shepherd@ohioattorneygeneral.gov

Counsel for Staff of the Ohio Power Siting Board

/s/ Anna Sanyal Anna Sanyal

In the Matter of the Application of)	
Kensington PV 1, LLC for a)	
Certificate of Environmental)	Case No. 21-764-EL-BGN
Compatibility and Public Need)	

Kensington PV 1, LLC's November 19, 2021 Responses to Staff's November 3, 2021 Data Requests

1. Please provide copies of comment cards received during the public informational meeting as well as comments received by mail and email.

Please find the comment cards attached. Also, no emails have been received to date in the Kensington Solar inbox

2. The pre-application notice, newspaper notice, public informational meeting notification letter, public informational meeting materials, and project website all indicate that the project will have a generating capacity of 135 MW. The application is for 145 MW. Please explain this discrepancy.

The Project was originally submitted to the PJM interconnection process in March of 2018 as a 175 MW project. After the System Impact Study (SIS) was issued and before the Facilities Study commenced, the project size was changed to 145 MW. The Project is currently listed on the PJM New Services Queue web page as a 145 MW project, which was reflected in the Application. However, through this data request, the Applicant clarifies that it intends to build the Project with a generating capacity of 135 MW only.

3. Page 1 of the Application states that the nameplate capacity would be 145 MW, but the PJM System Impact Study for AE2-194 shows the capability to be 175 MW. Please explain the disagreement.

Please see the response above for question 2.

4. Why is none of the PJM-estimated cost of \$30,187,000 to reconductor 13.8 miles of transmission line being allocated to the Kensington Solar Project, AE2-194? (page 19/36 of the SIS for AE2-194).

Initially, the PJM System Impact Study (SIS) did not account for Sammis units 5 - 7, which were originally scheduled to deactivate prior to our study year. Subsequently, these deactivations were cancelled. However, PJM failed to include the units' original load serving capabilities back into the SIS, which greatly underestimated the Sammis units 5 - 7's contributions to the Sammis – Beaver Valley 345 kV line. Once we advised PJM of this, the Sammis units 5 - 7 were fully dispatched, demonstrating that the impact preceded our project. Specifically, the Project's contribution to the Sammis – Beaver Valley 345 kV line was below 1% the line's rating. Consequently, pursuant to PJM's Open Access Transmission Tariff rules, the Project did not qualify for an allocation.

5. Please provide an estimate of the distance between the bus #239092, 02SAMMIS and bus #253902, 15BVRVAL.

This information is not provided in the AE2-194 System Impact Study and the Applicant does not have access to this information.

6. Please provide the values of the maximum wind speeds seen in the project area, along with their frequency of occurrence.

Wind velocity data from the Ohio State University College of Food, Agriculture and Environmental Science was reviewed to determine wind speeds in the Project Area. A summary of the data describing the average daily wind speeds recorded in 2020 is provided in Table 8-2, which appears in page 51 of the Application filed on October 19, 2021. This Table includes date from the closest weather station to the Project Area located in Wooster, Ohio. Average daily wind speeds were greater than 11.1 mph on less than 3% of the days of the year in 2020. The maximum average daily wind speed reported in 2020 was 13.0 miles per hour and the maximum wind speed recorded was 30.5 mph.

7. What is the anticipated effect in terms of force and loading from the maximum wind speeds on the panels, the trackers, and the support structures?

The Project will be designed according to Chapter 16 (Structural Design) of the Ohio Building Code, which specifies an ultimate design wind speed of 105 mile per hour ("mph") for a Risk Category I structure with site specific structural calculations by a Professional Engineer. The Project location is not considered a high wind speed zone and therefore high wind speeds are not expected to negatively impact the design or equipment. Exact loads and forces are dependent on final design.

8. What stresses would be induced in these various components, and how do these stresses compare to the maximum allowable stresses of the materials used for the panels and supporting structures.

Axial, bending, and torsional stresses depend on the component and are based on the loads. Maximum allowable stresses will be calculated by a professional engineer and will be within the design specifications of all final component choice.

As a best practice, the Applicant designs projects to keep components stressed to less than 90 percent of the allowable stress ratio permitted by AISC (100 percent being the limit state). This helps to account for minor variations realized during construction.

9. What standards and guidelines would be used for the design of the facility?

As noted on page seven of the Application, all Project equipment will be compliant with applicable Underwriters Laboratories, Institute of Electrical and Electronics Engineers, NEC, National Electrical Safety Code, and American National Standards Institute listings, and any other applicable industry codes and standards.

10. Will there be a stow mode for the panels?

Yes. The meteorological sensor package for the trackers continuously monitors the wind, snow and flood conditions automatically. The meteorological data puts the tracker into a safe stow position for each of these contexts using sophisticated algorithms that eliminate transients. During high wind events, the trackers will adjust to the safety stow position to limit the impact on structural components. Each tracker will also stow at the set safety stow position during nighttime and will resume tracking when the sun rises. The stow position anticipated for the Project are:

Wind Stow: 0 degree

Snow Stow: 40 degree

Hail Stow: 60 degree

Flood Stow: 0 degree

These values can be adjusted in the SCADA system once final calculation is completed and the project design is finalized. Also, the stowing approach above may be updated depending on the final design and equipment selection.

11. What would be the wind velocity that would cause the panels to become separated from the tracking system and support structures?

Project design and construction will be done based on all applicable standards and based on the wind speed in the area. Additionally, as noted in response to data requests six and seven, this area is not a high wind speed zone and therefore high wind speeds are not expected to negatively impact equipment. Therefore, we are not expecting panels become separated from the trackers.

What information would you like to hear m Do you reside or own land near the project area?	ore about? (YES)	NO
Please describe/provide location: <u>141 Sta 7 mile No</u>	*	
Would you like to discuss further one on one?	YES	x6
Was this information session effective and informative?	YES	NO
Comments: Why the Gow Choosing which We would like to keep you informed about the status of this project. Please we would like to keep you informed about the status of this project. Please and no Roma Kayp we will have we would like to keep you informed about the status of this project. Please contact information so that we can stay in touch with you. This informed marketing purposes or given to any third parties other than approved to for use on the project. Thank You!	se provide your ation will not be iberty Power con	name and used for ntractors
Name: Judie Rodney chilson		
Contact Information:		1

What information would you like to hear more about? What information would you like to hear more about?	NO
o you reside or own land near the project	
lease describe/provide location: 14310 FINE	
Kensington On 41161 (VES)	NO
Would you like to discuss further one on one?	
Year year YES	PRAILY
Was this information session encourses and a Victure 1 1/00 + 15	nain J
comments: I would like to prove where TR	enchine
this going to do to my went wine POISE,	mon
the ground Houra me the magnetic	siet
the construction, and Cause and wild	life
the plane 15 and gout the status of this project. Please provide you	r name and
contact information so that we can stay in touch with you. This information will not be contact information so that we can stay in touch with you. This information will not be required to any third parties other than approved Liberty Power c	ontractors
for use on the project. Thank You! 5-218 Cecer	. 263-
Name: Abita BAStor, Species + D	
Contract Information: 3303410221	

Do you reside or own land near the project area?	YES	NO
Please describe/provide location: <u>14410 Find Rol</u>		
Nould you like to discuss further one on one?	YES	(NO)
Was this information session effective and informative?	YES	\$
Comments: Why is Johnson doing this is has enough monupe Why do you work to destroy our b land de you work to destroy our b hand de you leve Chesepuke Come mong fundered people We and what We would like to keep you informed about the status of this project. Please contact information so that we can stay in touch with you. This information marketing purposes or given to any third parties other than approved Libe for use on the project. Thank You! Mame: Jaking Rod Chuism	earliful market my will provide yout provide	writ bund for used for fused for falone falone falone

o you reside or own land near the project area?	YES	NO
Please describe/provide location:		
Nould you like to discuss further one on one?	(ES)	NO
Was this information session effective and informative?	YES	NC
We would like to keep you informed about the status of this project. Plea	ase provide your r nation will not be u	ame an used for
contact information so that we can stay in touch with you. This inform	Liberty Power con	tractors
contact information so that we can stay in touch with you. This inform marketing purposes or given to any third parties other than approved for use on the project. Thank You!		
contact information so that we can stay in touch with you. This inform marketing purposes or given to any third parties other than approved for use on the project. Thank You! Name: Larry Fenbers, Carroll Electric Cooperative, Tr.		

Do you reside or own land near the project area?	YES)	NO
Please describe/provide location: <u>14392</u> Fink Rd	Kensington	e Stus
Would you like to discuss further one on one?	YES	NO
Was this information session effective and informative?	YES	NO
Comments: A put up with this chineses of the found thank right to set on my pour pour and beautiful courses of the set on my pour and beautiful courses an	vlaire Pa reh and news	inelo
We would like to keep you informed about the status of this project. Plec contact information so that we can stay in touch with you. This inform marketing purposes or given to any third parties other than approved for use on the project. Thank You! Name: Sadu Chulson Rodnug Chu	ise provide your ation will not be Liberty Power co	name and used for ntractors
Contact Information: 330-341-2018		

10

1.1

. .

What information would you like to hear mo Do you reside or own land near the project area?	re about?	NO
Please describe/provide location: 15173 Emerick Hensination C	Rd.	
Would you like to discuss further one on one?	YES	NO
Was this information session effective and informative?	YES	NO
Comments:		
We would like to keep you informed about the status of this project. Plaga		
contact information so that we can stay in touch with you. This informat marketing purposes or given to any third parties other than approved Lik for use on the project. Thank You!	ion will not be u perty Power con	ised for tractors
Name: Gretchen & Eric Wentz		
1 7 7 7 7 0 0 0		
Contact Information: 325-121-0809		

BEFORE THE OHIO POWER SITING BOARD

In the Matter of the Application of)	
Kensington PV 1, LLC for a)	
Certificate of Environmental)	Case No. 21-764-EL-BGN
Compatibility and Public Need)	

Kensington PV 1, LLC's January 6, 2022 Response to Staff's December 27, 2021 Data Request

1. Please list the IDs of the wetlands which would be impacted by the proposed solar array.

Response: The wetlands impacted by the proposed solar arrays are:

Wetland_ID	Component
W-1	Solar Array
W-2	Solar Array
W-23	Solar Array
W-24	Solar Array
W-35	Solar Array
W-36	Solar Array
W-38	Solar Array
W-6	Solar Array
W-7	Solar Array
W-70	Solar Array
W-71	Solar Array
W-95	Solar Array

BEFORE THE OHIO POWER SITING BOARD

In the Matter of the Application of)	
Kensington PV 1, LLC for a)	
Certificate of Environmental)	Case No. 21-764-EL-BGN
Compatibility and Public Need)	

Kensington PV 1, LLC's December 14, 2021 Responses to Staff's November 29, 2021, November 30, 2021, and December 1, 2021 Data Requests

- 1. The vegetation management plan states that the Applicant will not be impacting any wetlands due to solar modules, access roads, inverters, or substations. The application narrative states that the Applicant is currently planning 0.6 acres of permanent impact to wetlands due to solar modules. Please clarify whether permanent impacts to wetlands are anticipated.
 - a. If impacts are planned, please provide information on which wetlands are to be impacted.

The information in the application narrative is correct. There are anticipated to be approximately 0.6 acres of permanent impacts to wetlands from solar modules. The 0.6 acres of wetland impacts from solar modules that are placed over six different palustrine emergent (PEM) wetlands depicted as in Figure 8-5 of the application.

- 2. The vegetation management plan states that the project has been sited to avoid impacts to forested areas. The application states that 1.9 acres of forested land will be cleared. Please clarify whether clearing of forested land is anticipated.
 - a. If impacts are planned, please provide a figure depicting the forested area to be cleared along with the current planned layout of the project.

The information in the application narrative is correct. The project has been sited to avoid impacts to forested areas except for approximately 1.9 acres of forested areas that will need to be cleared as part of the Project. Figure 1 shows the locations where forested areas will be cleared for aboveground infrastructure.

3. Please explain the nature of the proposed stream crossings, including number of crossings, which streams will be crossed, and the method of crossing.

There are 29 stream crossings anticipated based on the current site layout. Stream crossings are limited to collector line and fence crossings. For all streams to be crossed by collector cables, horizontal directional drilling (HDD) will be utilized to avoid stream impacts. For streams to be crossed by Project fencing, the fencing will be constructed so that it spans the stream. The table below summarizes the information for each stream.

Stream ID	Infrastructure	Anticipated	
	Impact	Crossing Method	
S-1	Fence	Span	
S-1	Fence	Span	
S-2	Fence	Span	
S-28	Collector	HDD	
S-40	Collector	HDD	
S-45	Fence	Span	
S-45	Collector	HDD	
S-49	Fence	Span	
S-49	Collector	HDD	
S-51	Fence	Span	
S-51	Collector	HDD	
S-54	Collector	HDD	
S-56	Collector	HDD	
S-56	Collector	HDD	
S-KP06	Fence	Span	
S-KP10	Collector	HDD	
S-KP11	Collector	HDD	
S-KP24	Collector	HDD	
S-SZ01-DN-RB	Fence	Span	
S-SZ01-DN-RB	Collector	HDD	
S-SZ02	Collector	HDD	

- 4. This project is within range of the northern harrier, a state endangered bird. Table 8-3 lists pasture as the primary habitat type present in the project area (738 acres), which provides nesting and hunting habitat for this species. The ODNR DOW has contacted Staff expressing concerns about potential impacts to this species and recommends that construction in this habitat be avoided during the species' nesting period of May 15 to August 1. If construction during this time cannot be avoided, ODNR may require a presence/absence survey for the northern harrier. Does the Applicant intend to conduct construction between May 15 and August 1?
 - a. If yes, please detail the applicant's plans to avoid impacts to this species. These plans should be coordinated with the ODNR DOW as soon as possible.

The Applicant only plans to disturb 0.2 acres of pasture habitat. The Applicant anticipates undertaking work in this area prior to May 15th, which is outside of the breeding/ nesting season

for the northern harrier. If the project schedule is impacted beyond control of the contractor (i.e. weather conditions), a qualified consultant will conduct reconnaissance, in coordination with ODNR, to assess if nesting is occurring. If nests are discovered then best practices will be followed to avoid the nest, until fledglings leave the nest.

Note that, the Applicant's consultant, Stantec, determined that all disturbed pasture habitat with the exception of 0.2 acres would not constitute nesting habitat. The pasture habitat within the Project area is heavily grazed. The vegetation is low growing herbaceous species with some trees/shrubs near streams or on the fence lines between the fields used for pasture. Dominant species in areas identified as pasture included English plantain (Plantago lanceolata), white clover (Trifolium repens), red clover, alsike clover (Trifolium hybridum), perennial ryegrass (Lolium perenne), curly dock (Rumex crispus), Kentucky bluegrass (Poa pratensis), multiflora rose (Rosa multiflora), Queen Anne's lace (Daucus carota), and Canada thistle (Cirsium arvense). As the Habitat Assessment Memo provided as Exhibit Q in the application states, the pasture within the Project area is likely hunting/feeding habitat for northern harriers, especially in the winter and during migration. However, as stated by ODNR in the environmental response letter provided for the Project on March 16, 2021, breeding habitat for this species is grasslands and large marshes. Ehrlich et al. (1988) also lists prairie, savanna, slough, wet meadows, and marsh as breeding habitat for northern harriers. They also state that nests are typically elevated at a height of 5 feet or less from the ground, such as on a mound (Ehrlich et al. 1988). The Cornell Lab (2021) describes northern harrier nesting/breeding habitat as large and undisturbed wetlands and grasslands with low and thick vegetation. Other habitats this bird will use for breeding includes marshes, lightly grazed meadows, old fields, tundra, dry upland prairies, drained marshes, high-desert shrub-steppe, and riverside woodlands. They also describe the wintering grounds as more open areas such as deserts, sand dunes, pasturelands, croplands, dry plains, grasslands, old fields, open floodplains, estuaries, and marshes (The Cornell Lab 2021). Dechant et al. (2002), in their thorough literature review of suitable habitat for the northern harrier, states that the species build well-concealed nests on the ground in tall and dense vegetation including living and dead grasses, forbs, low shrubs. Nest locations are undisturbed areas with abundant residual cover, typically a minimum of 40% cover. Dechant et al. (2002) summarizes nesting locations in dry areas and they are quite variable. Nests can be in western snowberry or other shrubs (North Dakota/northern Great Plains), large blackberry patches (Missouri), planted grass/legume fields with cover approximately 1.5 - 2 feet tall (North and South Dakota), and idle or fallow fields dominated by native or non-native grass species (Illinois).

The common parameters throughout the sites mentioned in Dechant et al. (2002) were little or no disturbance and good cover, which is why Stantec did not consider pasture at the Project to be preferred breeding habitat for northern harrier. As documented in the Habitat Assessment, there are limited areas of Old Field (15.2 acres) in the Project Area, which could potentially provide suitable nesting although those areas are fragmented and small in size and are largely outside the area proposed for disturbance (approximately 0.2 acres proposed for disturbance).

Literature Cited

The Cornell Lab. 2021. All About Birds: Northern Harrier. Available at: https://www.allaboutbirds.org/guide/Northern_Harrier/lifehistory. Accessed December 9, 2021.

Dechant, J.A., M.L. Sondreal, D.H. Johnson, L.D. Igl, C.M. Goldade, M.P. Nenneman, B.R. Euliss. 1998 (revised 2002). Effects of management practices on grassland birds: Northern harrier. Northern Prairie Wildlife Research Center, Jamestown, North Dakota. 15 pp.

eBird. 2021. Northern Harrier. Available at:

https://ebird.org/map/norhar2?neg=true&env.minX=&env.minY=&env.maxX=&env.maxY=&z h=false&gp=false&ev=Z&mr=1-12&bmo=1&emo=12&yr=all&byr=1900&eyr=2021. Accessed December 9, 2021.

Ehrlich, P.R., D.S. Dobkin, D. Wheye. 1988. The Birder's Handbook: A Field Guide to the Natural History of North American Birds. Published by Simon & Schuster/Fireside Books, New York, New York.

5. What is the manufacturer and model of the substation transformer used in noise model?

A substation transformer model has not yet been selected for the Project so the noise model utilized a generic transformer model. The proposed Project substation includes one step-up transformer with an estimated sound power level of approximately 91.3 dBA. A tonal penalty of 5 dBA was added to each octave band resulting in an overall sound power level of 96.3 dBA for the substation transformer.

6. What is the rating of the substation transformer used in noise model?

The generic substation transformer model utilized is rated as a 175 mega volt ampere model.

7. Please provide Appendix C of the sound report in excel format.

Appendix C of the sound report is provided as an Excel sheet.

- 8. On page 8 of the noise report you state," The following filters were applied to the data: the first and last hour of data at each location was removed to avoid set-up and break-down noise, any readings over 60 dB during possible weather events, and readings over 50 dB associated with wind gusts."
 - a. Anomalous events should be filtered out of ambient sound level results. Anomalous events include the nearby use of lawnmowers, tractors, or other noisy equipment. Anomalous events can also include high frequency biogenic noise as high frequency biogenic noise does not occur during the winter months.
 - b. Did anomalous events exist outside of those mentioned above (set-up/ breakdown, weather/wind)? If so, were these anomalous events filtered out of

ambient sound level results? If not, please filtered out those anomalous events from ambient sound level results and state the corrected ambient Leq levels.

Readings greater than 5dB higher than temporally adjacent readings were reviewed and filtered out if suspected to remove anomalous readings.

9. On page 10 of the noise report the Leq daytime levels are listed as 44.2, 42.3, 45.2, 41.0, and 43.9. These values average 43.3 dBA. But on page 14 it is stated that the average daytime Leq is 43.6 dBA. If the ambient levels do not need to be corrected in accordance with question 4, what is the correct daytime ambient Leq average?

The averaged 43.3 dBA Leq is an arithmetic average, 43.6 dBA is the logarithmic average. Since decibels are a logarithmic value, the latter was used within the sound report.

10. Has the Applicant conducted any field surveys to confirm the locations of the oil and gas wells identified in Table 8-1 of the application?

To date, field surveys have not been completed to confirm the locations of the oil and gas wells identified within Table 8-1 of the application. The location of these wells will be confirmed as part of the ALTA survey that will be completed for the Project. The Applicant currently projects the survey will be completed around Spring 2022.

11. Please specify minimum distances between the oil and gas well features and the nearest proposed solar facility infrastructure.

The table below provides a summary of the distance from each well within the Project Area, to the nearest infrastructure component.

Well API #	Well Type and Status	Nearest	Approximate
		Infrastructure Type	Distance (ft.)
34029209050000	Vertical well, Producing	Fence	18
34029217670000	Vertical well, Producing	Fence	374
34029217800000	Vertical well, Producing	Fence	373
34029607990000	Vertical well, Plugged	Fence	12
34029608090000	Vertical well, Plugged	Fence	1,123
34029607980000	Vertical well, Plugged	Solar Array	0
34029608100000	Vertical well, Plugged	Solar Array	24
34029607970000	Vertical well, Plugged	Fence	19
34029203130000	Vertical well, Plugged	Solar Array	0
	Vertical well, Expired Permit		0
34029216260000	(not drilled)	Solar Array	
34019224730000	Horizontal well, Producing	Solar Array	0
34029216960000	Horizontal well, Producing	Solar Array	0
34029217670000	Horizontal well, Producing	Fence	1
34029217730000	Horizontal well, Producing	Inverter	0
34029217800000	Horizontal well, Producing	Fence	177

Well API #	Well Type and Status	Nearest	Approximate
		Infrastructure Type	Distance (ft.)
34029218140000	Horizontal well, Producing	Fence	836
34029218150000	Horizontal well, Producing	Solar Array	0
34029218390000	Horizontal well, Producing	Solar Array	0
34029218760000	Horizontal well, Producing	Inverter	0
34029218950000	Horizontal well, Producing	Solar Array	0
34029218960000	Horizontal well, Producing	Solar Array	0

12. Table 4-1 of the application lists proposed project setbacks. 25 feet from active well pads has been proposed. Have setbacks from oil and gas related lease/access roads been considered? Have setbacks from oil and gas wells identified as plugged and abandoned been considered?

The Applicant has setback 25 feet from well pads (active, plugged, and/or abandoned) associated with the horizontal wells, although Project infrastructure has been placed over the horizontal pipe as noted in the table above.

13. Exhibit R, which provides the ODNR geology review of the application, indicates two mines are present within the project area. Please specify minimum distances between the mine boundaries and the nearest proposed solar facility infrastructure.

The Summitville Tiles, Inc. surface mine and Dome Materials, Inc. shale quarry have both been avoided with Project infrastructure. Project fencing is within approximately 10 feet of the Summitville Tiles, Inc. mine and Project fencing is more than 510 feet from the Dome Materials, Inc. boundary.

14. In the northern portion of the project area (west of test boring 2), the ODNR interactive mines viewer map indicates a historic coal mining site existed at one time. Is facility infrastructure planned in this area?

This particular parcel has been reclaimed by a participating landowner and the land is currently used for agriculture. Facility infrastructure is planned in this area, as we are planning to install modules.

15. What consideration was given to the project area's USDA websoil survey suitabilities and limitations for building site development?

USDA websoil suitabilities and limitations will be considered in the final design of the facility. A preliminary geotechnical survey was completed and was submitted as Exhibit K to the application.

16. What is the maximum slope where solar facility infrastructure is proposed?

As noted in page 17 of the application, the maximum slope for the typical racking technology is 17.5 degree.

17. The application indicates figure 8-4 includes delineation of both highly erodible soils and slopes exceeding 12%. Review of the figure indicates steep slopes were omitted. Please revise as necessary.

Figure 2 is included within this data request submittal and provides the locations of steep slopes (>12%) and highly erodible soils within the Project Area and a 0.5 mile radius.

18. Page 50 of the application indicates pile load testing was part of the preliminary geotechnical work (Exhibit K). Please confirm that site specific pile load testing has in fact occurred and provide the results of that testing.

Pile load testing has not been completed and is planned to be completed in 2022. The preliminary geotechnical work was completed to fulfill the OPSB permitting requirements and preliminary engineering requirements to comply with precedent. More comprehensive geotechnical works will be performed prior to the finalization of project design.

19. Page 10 of Exhibit K indicates that difficulties installing traditional driven piles are expected in some portions of the project area. The report recommends that a pile driving and testing program be developed to assess the difficulty of piles penetrating the soil conditions. The report goes on to indicate that predrilling may be necessary to anchor piles at the appropriate depths (expected to range from 6-10 feet bgl). In reviewing the boring logs, "highly weathered and very weak" bedrock was encountered at depths within the range of the expected pile embedment depths. What special engineering considerations have been given to ensuring these pile foundations will provide adequate support of the facility infrastructure during its operational life?

Based on the preliminary geotechnical investigation and the amount of cobble stones encountered, helical piles would be utilized for most of the site. Furthermore, once pile load testing is complete, additional measures can be considered such as 'dead man' piles (structured footings) or remedial helical (excavate and backfill with preferred material).

20. Please provide Staff with an unanticipated discovery plan. This plan would account for course/s of action to be taken in the event previously unidentified subsurface features which are or could be considered hazards are encountered during the proposed construction. e.g. oil and gas well infrastructure, abandoned mines, contaminated soils, etc.

An unanticipated discovery plan will be prepared during final engineering design by the engineering, procurement, and construction contractor and will be provided to OPSB Staff at least 30 days before the pre-construction meeting.

21. Exhibit L (Glare Hazard Analysis) refers to three types of glare. Are there any minutes of "red" type predicted for the project?

As stated within the application and Exhibit L, no glare is anticipated from the Project at nearby residences, roadways, railroad tracks, or airports. This includes zero minutes of red, yellow, or green glare.

22. Referring to page 7 of the application and Exhibit M (Notice Criteria Tool) indicate a maximum height of 110 for substation components or transmission line structures. What would the height of the tallest structure at the solar farm be?

The 110-feet for the notice criteria was included as a conservative assumption and to reflect maximum gen-tie pole height or transformer height at the substation. The maximum height for the solar farm facility equipment's would be:

- 4 meters (13.1 feet) at each inverter location.
- 3 meters (9.8 feet) for the PV arrays.
- Please refer to response 23 for substation components.

23. Please provide what the height of the following structures at the solar farm would be

- a. substation support structures,
- **b.** substation lightning mast
- c. transmission line support structures

The highest structure in the substation will be the H-Frame steel dead-end structure with lightning mast on top. Approximate total height of the steel structure will be 60 feet with an additional 10 feet for the lightning rod for a total height of 70 feet from the ground. The next highest structure will be the medium voltage (34.5 kV) buswork support structure at a height of about 30 feet with a single lightning mast that would make the total height 65 feet above the ground. The highest structure in the transmission owner switching station is expected to be the H-Frame steel structure with lightning rod for a total height of approximately 70 feet from the ground, though final design will be provided by the transmission owner.

24. The Application mentions a 138-kV three breaker ring bus POI substation, and 375foot loop in/out line to be built, owned, and operated by ATSI. Please confirm that these are not included as part of this Application.

The Application includes the POI switchyard and the Applicant expects to assign the portion of the certificate covering the POI switchyard and line loop to ATSI.

25. The Application mentions a 175-foot-long gen-tie from the collector substation to the POI substation. Is Kensington PV 1, LLC is requesting approval of this component as part of this Application?

The 175-foot gen tie from the collector substation to the POI substation is part of this Application.

- 26. If included in this Application, please provide the following information for the 175 feet long gen tie transmission line:
 - a. Tower designs, pole structures, conductor size and number per phase, and insulator arrangement.
 - b. Base and foundation design.
 - c. Cable type and size, where underground.
 - d. Other major equipment or special structures.

The gen-tie line is planned as a single slack span between an H-Frame steel dead-end structure inside the project substation and an H-Frame steel dead-end structure inside the POI substation. No additional poles/towers are planned. There will be three phases, one conductor per phase. The conductor size will be 795MCM ACSR "DRAKE". There will also be two shield wires (7-#8 Alumoweld).

27. If included in this Application, please provide the shapefiles and depiction on Figure 3-2 (Project Site Layout Map) for the 175 feet long gen tie transmission line.

These will be uploaded to OPSB staff's sharefile site.

28. Please provide the following information for the substation support structures/poles referenced on page 7 of the Application:

- a. Tower designs, pole structures, conductor size and number per phase, and insulator arrangement.
- b. Base and foundation design.
- c. Cable type and size, where underground.
- d. Other major equipment or special structures.

Please refer to responses #23 and #26. All steel structures within the substation will be mounted on concrete foundations.

29. Is the gen tie transmission line design capacity one hundred kilovolts or more and within one hundred feet of an occupied residence or institution? If yes, please provide that voltage and the calculated electric and magnetic field strength levels at one meter

above ground, under the conductors and at the edge of the right-of-way for (i) Winter normal conductor rating, (ii) Emergency line loading, and (iii) Normal maximum loading.

Yes, the gen-tie line nominal voltage is above 100 kV. This line will not be within 100 feet of an occupied residence or institution.

30. What is the distance between the solar farm equipment and the 13 private water wells mentioned on page 47 of the Application?

A summary of the distance to each water well within the Project Area, and the type of infrastructure nearest to it, is provided in the table below. Of note, all of these wells are owned by project participating landowners. As part of the ALTA Survey, well status will be confirmed, and as part of final engineering design, the layout will be revised accordingly to ensure all wells are avoided.

Well #	Well Depth (ft.)	Nearest	Approximate
		Infrastructure Type	Distance (ft.)
25302	148	Solar Array	0
		AC Underground	
443634	227	Collection Line	113
89151	190	Fence	29
71429	53	Fence	16
387890	193	Fence	78
223166	100	Fence	15
955601	135	Fence	121
143790	72	Solar Array	0
301931	270	Fence	305
15217	105	Solar Array	0
25331	74	Fence	913

31. Please explain what possible avoidance, minimization, and/or mitigation measures Kensington PV 1, LLC will employ during construction for water well locations in the project area.

In consultation with participating landowners, all water well locations will be flagged prior to construction. In addition, as part of the ALTA Survey, water well locations within the project area will be identified. As noted in page 47 of the application, all water well locations will be avoided as part of final engineering design.

32. Would any of the 13 water wells be properly abandoned and sealed in accordance with Ohio Admin. Code 3701-28?

The Applicant anticipates engaging in discussions with participating landowners to determine if any of these wells need to be abandoned or sealed. Additionally, water wells will be verified as part of the final geotechnical survey to determine if these wells were properly abandoned and sealed as per Ohio Admin. Code 3701-28.

33. Please explain what setback Kensington PV 1, LLC would implement in its final design to a water well?

As currently designed, with the exception of three wells identified above (and of which participating landowners are aware), all project infrastructure is at least 15 feet from the identified well. For the three wells that are identified as being under the solar array, final engineering will micro-site the trackers so that these wells can be avoided during the installation of the tracker posts.

34. Please provide the current draft emergency response plan or an example emergency response plan referenced on page 42 of the Application.

A draft emergency response plan is attached.

35. The proposed decommissioning plan (Exhibit B) appears to be insufficient. Please submit an updated decommissioning plan and total decommissioning cost estimate without regard to salvage value but includes: (a) a provision that the decommissioning financial assurance mechanism include a performance bond where the company is the principal, the insurance company is the surety, and the Ohio Power Siting Board is the obligee; (b) a timeline of up to one year for removal of the equipment; (c) a provision to monitor the site for at least one additional year to ensure successful revegetation and rehabilitation; (d) a provision where the performance bond is posted prior to the commencement of construction; (e) a provision that the performance bond is for the total decommissioning cost and excludes salvage value; (f) a provision to coordinate repair of public roads damaged or modified during the decommissioning and reclamation process; (g) a provision that the decommissioning plan be prepared by a professional engineer registered with the state board of registration for professional engineers and surveyors; and (h) a provision stating that the bond shall be recalculated every five years by an engineer retained by the Applicant.

An updated Decommissioning Plan is included as an attachment to this data request. The updated Decommissioning Plan reflects conditions (a) - (h) listed above, with the exception of (c). The Applicant will attempt to monitor the site and ensure revegetation has been completed if permitted by the landowner who retains control of the land following decommissioning of the Project.

36. The proposed decommissioning plan (Exhibit E, page 2) indicates that decommissioning activities are anticipated to be completed in 12 to 18 months. Please explain what activities would extend after 12 months.

The timing is an estimate and could be influenced by circumstances beyond control of the developer, such as but not limited to weather or permit delays. Nonetheless, minimal activity is

expected to go beyond 12 months and would involve minor tasks such as complying with landowner requests and any potential remediation work.

37. Please describe the "Gas/Hazardous Liquid Pipeline" identified on Figure 3-1 by providing, to the extent known, the:

- a. the pipeline company owner;
- b. the name, number, and designator of the pipeline;
- c. the diameter of the pipeline;

There are three pipelines depicted in Figure 3-1, however information is only available for two of the lines. A summary of the available information from the U.S. Energy Information Administration for each pipeline is provided in the table below.

Pipeline/Location in	Туре	Operator
Project Area		
East Sparta – Heath		
(East/West through		
Project)	Petroleum	Marathon
Interstate		
(North/South through		
Project)	Natural Gas	Columbia Gas Trans Co.
Southeast/Northwest		
through Project	Natural Gas	Unknown

Utility pipeline easements/right of ways would be identified as part of the title process and ALTA Survey work, as well as coordination with the Ohio Utilities Protection Services prior to construction.

38. What is the distance between solar equipment and the gas/hazardous liquid pipeline(s) right-of-way?

No information pertaining to the right-of-way width is available for the three pipelines through the Project Area, however the centerline of the East Sparta – Heath and Interstate pipelines cross collector lines, fences, access roads, and solar arrays. The Applicant will ensure that final project design avoids these pipelines. The unknown natural gas line, which is routed southeast/northwest, is approximately 65 feet from the nearest solar modules.

39. Please describe work procedures and safety precautions that will be implemented while working near the existing gas/hazardous liquid pipeline(s).

At a minimum, the following protocols will be followed:

- Industry best practices related to working near pipelines will be implemented. The selected EPC contractor safe work procedure will identify the exact safety precautions when working near existing gas/hazardous liquid pipelines
- As per state requirements, prior to any civil work, the contractor will have to contact Ohio Utilities Protections Services, Ohio 811 'call before you dig'
- Consultation with pipeline owner is continuous and compliance with their procedures will be required. If a collector line crosses a pipeline via directional bore then standard separation distance will be required
- All pipelines will be identified on ALTA survey maps

40. Please submit any correspondence sent to and received from the pipeline(s) owner or operator.

No correspondence has been sent or received from the pipeline owners or operators to date referenced in Question 37. Ongoing discussions will occur as necessary with these owners/operators.

41. Please identify the number of adjacent non-participating parcels containing a residence with a direct line of sight to facility components.

Creating a viewshed model using National Land Cover Data, with an assumed vegetation height of 40 feet for areas identified as forested, 13 feet for the panel height, and a 12-foot observer height (one-story building), we have estimated that 61 non-participating adjacent parcels with a residence would have a limited view of the solar project to some degree. As indicated in the application, further potential reasonable mitigation measures could be offered in the form of vegetative screening.

42. Please provide an estimate of the nature and amounts (in cubic yards) of debris and other solid waste generated during construction.

As stated in the application, the Applicant estimates that one 30 cubic yard roll-off dumpster will be required for waste collection every two weeks during the 12 to 18 month construction period. It is assumed that this dumpster would be full when emptied each week. Much of the construction waste consists of recyclable materials which the Applicant will collect and divert from the waste stream. This will be accomplished by using a 30 cubic yard recycling dumpster which will be collected monthly during the construction period and is assumed to be full each month when collected.

43. In June of 2021, it appears that the Applicant received approval from the OHPO of the Applicant's workplans to study for the presence/absence of architectural and archaeological resources. Preliminary architectural results indicated the need for mitigation or avoidance of above-ground resources. What is the <u>current status</u> of the

Applicant's architectural/archaeological studies and when does the Applicant anticipate their completion?

Archaeology surveys within the Project, following the methodology approved by OHPO, commenced on September 28, 2021 and have been ongoing since then as field conditions and crop harvest schedules have allowed. The Applicant anticipates completion of the surveys by the end of December 2021, provided the remaining land can be disced to allow for suitable surface visibility. Of the project area, approximately 300 acres remain which still need to be surveyed within the fenced area.

Provide an proposed timeline for:

a. Submission of all studies to the OHPO;

The architecture/history report was submitted to OHPO on October 15, 2021. It is estimated that the archaeology report will be submitted to OHPO in early January 2022.

b. Receipt of concurrence from the OHPO regarding the Applicant's final studies;

We estimate that with submittal of the archaeology report to OHPO by January 2022, that concurrence from OHPO on the archaeology report will be provided in February 2022.

c. Submission of any needed mitigation (and/or avoidance) recommendations to the OHPO;

The Applicant will work with OHPO to design an acceptable screening plan to mitigate potential impacts to architecture/history resources by February 2022.

d. Anticipated date of any required execution of MOUs with the SHPO.

The Applicant will execute a Memorandum of Understanding to finalize the agreement with OHPO regarding the implementation of the screening plan by March 2022.

44. Provide a large scale map which depicts participating and adjacent non-participating parcels containing a residence with a direct line of sight to facility components. This map should be superimposed upon the Applicant's proposed landscaping mitigation map. Identify and label nearby adjacent roads, recreational facilities, schools, cemeteries and any other sensitive land uses. Please also include a KMZ layer of the adjacent non-participating parcels containing a residence with a direct line of sight to facility components.

This figure has been provided as requested as Figure 3 in the data request response and the corresponding kmz provided.

















Preliminary Decommissioning Plan -Kensington Solar Project Columbiana County, Ohio



Prepared for: Kensington PV 1, LLC 354 Davis Road, Suite 100 Oakville, ON L6J 2X1

Prepared by: Stantec Consulting Services Inc. 1165 Scheuring Road De Pere, Wisconsin 54115

Project No: 2028113260 December 13, 2021

PRELIMINARY DECOMMISSIONING PLAN KENSINGTON SOLAR PROJECT, COLUMBIANA COUNTY, OHIO

This document entitled Preliminary Decommissioning Plan – Kensington Solar Project, Columbiana County, Ohio, was prepared by Stantec Consulting Services Inc, ("Stantec") for the use of Kensington PV 1, LLC (The "Client"), and the applicable regulatory agencies. Any reliance on this document by any other third party is strictly prohibited. The material in this document reflects Stantec's professional judgment in light of the scope, schedule and other limitations stated in the document and in the contract between Stantec and the Client. The opinions in this document are based on conditions and information existing at the time this document was published and do not take into account any subsequent changes.

Matthew A Clementi, PE Senior Project Manager

Associate, Senior Scientist and Project Manager

signature

Courtney Dohoney, PMP Senior Associate, Project Manager


Table of Contents

1.0	INTRODUCTION
1.1	SOLAR FARM COMPONENTS 1
1.2	TRIGGERING EVENTS AND EXPECTED LIFETIME OF PROJECT
1.3	DECOMMISSIONING SEQUENCE
2.0	PROJECT COMPONENTS AND DECOMMISSIONING ACTIVITIES
2.1	OVERVIEW OF SOLAR FACILITY SYSTEM
2.2	SOLAR MODULES
2.3	TRACKING SYSTEM AND SUPPORT
2.4	INVERTER STATIONS
2.5	ELECTRICAL CABLING AND CONDUITS
2.6	PROJECT SUBSTATION
2.7	OVERHEAD GENERATION TIE-IN TRANSMISSION LINE
2.8	OPERATIONS AND MAINTENANCE BUILDING
2.9	PERIMETER FENCING AND ACCESS ROADS
3.0	LAND USE AND ENVIRONMENT
3.1	SOILS AND AGRICULTURAL LAND
3.2	RESTORATION AND REVEGETATION
3.3	SURFACE WATER DRAINAGE AND CONTROL
3.4	MAJOR EQUIPMENT REQUIRED FOR DECOMMISSIONING
4.0	DECOMMISSIONING COST ESTIMATE SUMMARY
4.1	DECOMMISSIONING RISK OVER THE LIFECYCLE OF A PROJECT
4.2	DECOMMISSIONING EXPENSES
4.3	DECOMMISSIONING REVENUES11
4.4	DECOMMISSIONING COST SUMMARY
4.5	FINANCIAL ASSURANCE
LIST OI	TABLES
Table	1 Primary Components of Solar Farm to be Decommissioned
Table	2 Typical Access Road Construction Materials
Table	3 Estimated Decommissioning Expenses – 145 MW _{AC} Solar Array10
Table	4 Estimated Decommissioning Revenues12
Table	5 Net Decommissioning Summary12
Table	6 Financial Assurance Summary Error! Bookmark not defined.

LIST OF FIGURES

Figure 1 Proposed Project Layout

1.0 INTRODUCTION

Kensington Solar PV 1, LLC, (Kensington Solar) is proposing to construct and operate the Kensington Solar Project (the Project) in Franklin Township in Columbiana County, Ohio. The Project footprint encompasses approximately 1,132 acres. The maximum nameplate generating capacity of the Project will be up to 145 megawatts, alternating current (MW)_[AC]. Kensington Solar is currently considering bifacial monocrystalline solar panels for the Project.

This Decommissioning Plan (Plan) provides a description of the decommissioning and restoration phase of the Project. Start-of-construction is planned for the first quarter of 2023, with a projected Commercial Operation Date in the first quarter of 2024. The Project will consist of the installation of the perimeter fencing; solar arrays and associated trackers, concrete pads for substation, and steel piles; inverter stations; access and internal roads; electrical collection system and substation (Figure 1).

This Plan is applicable to the decommissioning/deconstruction and restoration phases of the Project. A summary of the components to be removed is provided in Section 1.1. A summary of estimated costs associated with decommissioning the Project is provided in Section 4.0.

1.1 SOLAR FARM COMPONENTS

The main components of the Project include:

- Solar modules
- Tracking system and steel piles
- Inverter stations on piers
- Electrical cabling and conduits
- Site access roads
- Perimeter fencing
- Project substation and short transmission tie-in line

1.2 TRIGGERING EVENTS AND EXPECTED LIFETIME OF PROJECT

Project decommissioning may be triggered by an event such as the end of the power purchase agreement with no plans for further marketing of power sales, abandonment, or when the Project reaches the end of its operational life. The Project will be considered to be abandoned if facilities are non-operational for a period of twelve (12) consecutive months. Project facilities will be removed from the site in accordance with a timeframe agreed upon by Kensington Solar, the Ohio Power Siting Board Staff (OPSB), and the respective county administrators.



If properly maintained, the expected lifetime of the Project is 40 years. In the event that the modules are not retrofitted, or at the end of the Project's useful life, the panels and associated components will be decommissioned and removed from the Project site.

Components of the solar facility that have resale value may be sold in the wholesale market. Components with no wholesale value will be salvaged and sold as scrap for recycling or disposed of at an approved offsite licensed solid waste disposal facility (landfill). Decommissioning activities will include removal of the arrays and associated components as listed in Section 1.1 and described in Section 2.

Kensington Solar is committed, where possible, to recycling all solar panels. This commitment includes panels damaged during construction and operation, as well as panels at the end of Project life/ decommissioning.

1.3 DECOMMISSIONING SEQUENCE

Decommissioning activities will begin within twelve months of the Project ceasing operation and are anticipated to be completed in 12 to 18 months. Kensington Solar will be the responsible party. Monitoring and site restoration may extend beyond this period to ensure successful revegetation and rehabilitation. The anticipated sequence of decommissioning and removal is described below; however, overlap of activities is expected.

- Reinforce access roads, if needed, and prepare site for component removal
- Install erosion control fencing and other best management practices (BMPs) to protect sensitive resources and control erosion during decommissioning activities
- De-energize solar arrays
- Dismantle panels and above ground wiring
- Remove tracking and piles
- Remove inverter/transformer stations along with support system and foundation pads
- Remove electrical cables and conduits
- Remove access and internal roads and grade site (if required)
- Remove substation and associated transmission tie-in line
- De-compact subsoils as needed, restore and revegetate disturbed land to preconstruction conditions to the extent practicable



2.0 PROJECT COMPONENTS AND DECOMMISSIONING ACTIVITIES

The solar facility components and decommissioning activities necessary to restore the Project area, as near as practicable, to pre-construction conditions are described within this section.

2.1 OVERVIEW OF SOLAR FACILITY SYSTEM

Kensington Solar anticipates utilizing approximately 353,684 solar modules, with a total nameplate generating capacity of approximately 168 MW, direct current (DC) (145 MW_[AC]). The Project footprint encompasses approximately 1,132 acres and will be bounded by perimeter fencing as shown on Figure 1 (preliminary design; subject to modification). The land within the perimeter fencing is predominantly agricultural land. Statistics and estimates provided in this Plan are based on Jinko Solar Eagle 475-watt bifacial module or a similar module.

Foundations, steel piles, and electric cabling and conduit installed below the soil surface will be removed. Access roads may be left in place if requested and/or agreed to by the landowner; however, for purposes of this assessment, all access roads are assumed to be removed. Public roads damaged or modified during the decommissioning and reclamation process will be repaired upon completion of the decommissioning phase and in compliance with the Road Use Agreement that is expected to be implemented between Kensington Solar and the Columbiana County Engineers. An estimated cost of public road repair is included in Project decommissioning overhead costs.

Estimated quantities of materials to be removed and sold, salvaged, or disposed of are included in this section. Many of the materials described have salvage value; although, there are some components that will likely have none at the time of decommissioning. Removed materials that cannot be sold on the resale market will be salvaged or recycled to the extent possible. Other waste materials will be disposed of in accordance with state and federal law in an approved licensed solid waste facility.

Solar panels may have value in a resale market, depending on their condition at the end of the Project life. If the Project is decommissioned prior to the anticipated 40-year timeframe, the resale value of components will be substantially higher than at the end of the projected Project.

Table 1 presents a summary of the primary components of the Project included in this decommissioning plan.



Component	Quantity	Unit of Measure
Solar Modules (approximate)	353,684	Each
Tracking System (equivalent full trackers)	5,895	Tracker
Steel Piles	42,801	Each
Inverter Stations with Foundations	128	Each
Electrical Cables and Conduits	98,425	Linear Foot (estimated)
Perimeter Fencing	207,630	Linear Foot
Access Roads (approximate)	196,850	Linear Foot
Project Substation	1	Each

Table 1 Primary Components of Solar Farm to be Decommissioned

2.2 SOLAR MODULES

Kensington Solar intends to use a bifacial monocrystalline panel (475 watt) for the Project. Each module assembly (with frame) will have a total weight of approximately 58.4 pounds (26.5 kg). The modules will be approximately 87 inches by 41 inches in size and are mainly comprised of non-metallic materials such as silicon, glass, composite film, plastic, and epoxies, with an anodized aluminum frame.

At the time of decommissioning, module components in working condition may be refurbished and sold in a secondary market yielding greater revenue than selling as salvage material. The estimates in this report have been calculated using a conservative approach, considering revenue from salvage only, rather than resale of Project components.

Kensington Solar is committed, where possible, to recycling all solar panels. This commitment includes panels damaged during construction and operation, as well as panels at the end of Project life/ decommissioning.

2.3 TRACKING SYSTEM AND SUPPORT

The solar modules will be mounted on a single-axis, two-in-portrait tracking system. Kensington Solar will utilize the Voyager tracker manufactured by FTC Solar, the Gemini by NEXTracker or similar system for the tracking units. Each full, two-string tracker will be approximately 39.6 meters (130 feet) in length and will support 60 solar modules. Smaller trackers, supporting 30 panels each, will be employed at the edges of the layout to efficiently utilize available space. The tracking system is mainly comprised of high-strength galvanized steel and anodized aluminum; steel piles that support the system are assumed to be comprised of galvanized steel.

The solar arrays will be deactivated from the surrounding electrical system and made safe for disassembly. Liquid wastes, including oils and hydraulic fluids will be removed and



properly disposed of or recycled according to regulations current at the time of decommissioning. Electronic components, and internal electrical wiring will be removed and salvaged. The steel piles will be completely removed from the ground.

The supports, tracking system, and posts contain salvageable materials which can be sold to provide revenue to offset the decommissioning costs.

2.4 INVERTER STATIONS

The combined inverters/transformers (inverter stations) generally sit on small concrete footings or piers on steel piles within the array. The inverters will be deactivated, disassembled and removed. For purposes of this report, it is assumed that inverters will be constructed on concrete pads which will be completely removed during decommissioning. Depending on condition, the equipment may be sold for refurbishment and re-use. If not re-used, they will be salvaged or disposed of at an approved solid waste management facility.

2.5 ELECTRICAL CABLING AND CONDUITS

The Project's underground electrical collection system will be placed at a depth of at least 18 inches below the ground surface. For purposes of this report, it is assumed that all subsurface cabling will be removed and salvaged. Recovery cost has been conservatively based on aluminum wiring; however, the salvage value of copper, if used, would be far greater.

2.6 PROJECT SUBSTATION

A Project substation will be part of the Project within an approximately 375-foot by 375foot footprint. The substation will contain within its perimeter, a gravel pad, power transformers and footings, electrical control house and concrete pads, as needed. The substation transformers may be sold for re-use or salvage. Components of the substation that cannot be salvaged will be transported off-site for disposal at an approved waste management facility. Although there is some potential that the Project substation may remain at the end of the Project life, an estimated decommissioning cost has been included in this Plan.

2.7 OVERHEAD GENERATION TIE-IN TRANSMISSION LINE

There is an approximate 160-foot-long overhead transmission line to be constructed between the Project substation and a utility substation (the point of interconnection). Removal of the overhead generation tie-in transmission line is included in this Plan.

2.8 OPERATIONS AND MAINTENANCE BUILDING

There is no onsite Operations and Maintenance (O&M) building planned; therefore, no O&M building removal is included in this Plan.

2.9 PERIMETER FENCING AND ACCESS ROADS

The Project will include a security fence around the perimeter of the site and exclusionary area. The fence will total approximately 207,630 feet in length.

Access drives will provide direct access to the solar facility from local roads and along the inner perimeter of the arrays. Internal roads will be located within the array to allow access to the equipment. The site access drives will be approximately 16 feet in width and total approximately 196,850 feet (37.28 miles) in length. The access road lengths may change with final Project design. To be conservative, the decommissioning estimate assumes that all access roads will be completely removed.

Topsoil removed during Project construction will be redistributed on the Project site to be utilized during site restoration. It is assumed that 90 percent of the topsoil required to restore the site will be drawn from on-site sources. An additional 10 percent of native topsoil (approximately 5,833 cubic yards) will be purchased to augment on site soils.

During installation of the Project access roads, the existing topsoil will be excavated to a depth of six inches, the subgrade will be compacted, and then six inches of granular fill will be placed. The estimated quantity of these materials and the required topsoil to replace them is provided in Table 2.

Item	Quantity	Unit
Compacted granular fill, 6-inch thick – to be removed	58,326	Cubic Yards
Topsoil replacement	58,326	Cubic Yards (90% from on-site locations; 10% to be purchased to supplement)

Table 2 Typical Access Road Construction Materials

Decommissioning activities include the removal and stockpiling of aggregate materials onsite for salvage preparation. It is conservatively assumed that all aggregate materials will be removed from the Project site and hauled up to five miles from the Project area. Following removal of aggregate, the access road areas will be graded, de-compacted with deep ripper or chisel plow (ripped to 18 inches), backfilled with native subsoil and topsoil, as needed, and graded as necessary.

3.0 LAND USE AND ENVIRONMENT

3.1 SOILS AND AGRICULTURAL LAND

Areas of the Project that were previously utilized for agricultural purposes will be restored to their pre-construction condition and land use as dictated by landowner lease agreements. Restored areas will be revegetated in consultation with the current landowner and in compliance with regulations in place at the time of decommissioning. Land disturbed by Project facilities will be restored in such a way to be used in a reasonably similar manner to its original intended use as it existed prior to Project construction.

3.2 RESTORATION AND REVEGETATION

Areas of the Project that have been excavated and backfilled will be graded as previously described. Soils compacted during de-construction activities will be decompacted, as necessary, to restore the land to pre-construction land use. If present, drain tiles that have been damaged will be restored to pre-construction condition. Topsoil will be placed on disturbed areas and seeded with appropriate vegetation or in coordination with landowners within agricultural land. Work will be completed to comply with the conditions agreed upon by Kensington Solar and the OPSB or as directed by regulations in affect at the time of decommissioning.

3.3 SURFACE WATER DRAINAGE AND CONTROL

The proposed Project is predominantly located in agricultural land. The terrain is rolling hills. The Project facilities are being sited to avoid all impacts to wetlands and waterways. The existing Project site conditions and proposed BMPs to protect surface water features will be detailed in a Project Stormwater Pollution Prevention Plan (SWPPP) for the Project prior to the commencement of construction activities.

Surface water conditions at the Project site will be reassessed prior to the decommissioning phase. Kensington Solar will obtain the required water quality permits from the Ohio Environmental Protection Agency (OEPA) and the U.S. Army Corp of Engineers (USACE), if needed, before decommissioning of the Project. Construction storm water permits will also be obtained and a SWPPP prepared describing the protection needed to reflect conditions present at the time of decommissioning. BMPs may include construction entrances, temporary seeding, permanent seeding, mulching (in non-agricultural areas), erosion control matting, silt fence, filter berms, and filter socks.

3.4 MAJOR EQUIPMENT REQUIRED FOR DECOMMISSIONING

The activities involved in decommissioning the Project include removal of the above and below-ground ground components of the Project, and restoration as described in Sections 2 and 3.2.



Equipment required for the decommissioning activities is similar to what is needed to construct the solar facility and may include, but is not limited to: small cranes, low ground pressure (LGP) track mounted excavators, backhoes, LGP track bulldozers, LGP off-road end-dump trucks, front-end loaders, deep rippers, water trucks, disc plows and tractors to restore subgrade conditions, and ancillary equipment. Standard dump trucks will be used to transport material removed from the site to disposal facilities.



4.0 DECOMMISSIONING COST ESTIMATE SUMMARY

Expenses associated with decommissioning the Project will be dependent on labor costs at the time of decommissioning. For the purposes of this report approximate 2021 average market values were used to estimate labor expenses. Fluctuation and inflation of the labor costs were not factored into the estimates.

The value of the individual components of the solar facility will vary with time. In general, the highest component value would be expected at the time of construction with declining value over the life of the Project. Over most of the life of the Project, components such as the solar panels could be sold in the wholesale market for reuse or refurbishment. As efficiency and power production of the panels decrease due to aging and/or weathering, the resale value will decline accordingly. Secondary markets for used solar components include other utility scale solar facilities with similar designs that may require replacement equipment due to damage or normal wear over time; or other buyers (e.g., developers, consumers) that are willing to accept a slightly lower power output in return for a significantly lower price point when compared to new equipment.

4.1 DECOMMISSIONING RISK OVER THE LIFECYCLE OF A PROJECT

The probability of an event that would lead to abandonment or long-term interruption is extremely low during the first 15 to 20 years of the Project life. Accordingly, the risk of decommissioning the Project is extremely low during this time frame. The reasons why the risk to decommission the Project is extremely low in the early phases of the Project include, but are not limited to:

- In the early stages of the Project the resale value of the equipment is significantly higher than the decommissioning costs, resulting in a net positive (revenue).
- Project owners have sophisticated financing structures that allow the lender or tax equity partner to step in and rectify the event that may lead to abandonment.
- A Power Purchase Agreement (PPA) is a legal contract between an electricity generator (project owner) and a power purchaser (typically a utility or large power buyer/trader). The typical contractual term for a PPA can be many years, during which time the power purchaser buys energy, capacity and ancillary services, from the Project. Such PPAs play a key role in the financing of independently owned (i.e., not owned by a public utility) electricity generating assets since it defines the revenue terms for a project and the credit quality.
- Most critical solar components have original equipment manufacturer (OEM)
 warranties with terms in excess of five years that include labor and parts. A
 warranty is an agreement or guarantee outlined by a manufacturer to a customer
 that defines performance requirements for a product or service. Warranties give
 customers a form of insurance if the purchased product or service does not
 adhere to quality standards. These warranties assure the project owner, financing

parties, and other stakeholders, that equipment will perform as expected which minimizes the risk of a decommissioning event. Warranty lengths on critical components vary by component and include performance guarantees on solar modules.

- Solar projects consist of many networked components designed to convert solar radiation into electrical energy. The failure of any single component will not result in a substantial reduction of energy generation that could lead to a decommissioning event.
- Solar projects are required to maintain replacement value property damage insurance coverage and business interruption insurance coverage. Business interruption insurance covers the loss of income that a business suffers after a disaster or equipment failure. Typical solar business interruption insurance covers income loss for twelve months from the date of the event triggering the loss.
- The replacement costs of solar components will typically decline over time, and accordingly, costs to replace failed or damaged equipment after lapsed OEM warranties will not create large financial hurdles for the Project.
- Solar power is an increasingly popular form of renewable energy around the world and as an alternative to the burning of fossil fuels, solar ranks alongside wind and hydropower as essential energy options for the future of the planet. Solar also offers the additional benefit of being easier to build, operate and decommission with minimal environmental risks. Recent rises in popularity and use can be linked to lower installation and operation costs and it is expected that this pattern will continue into the future, further reducing the risk of a decommissioning event.

4.2 DECOMMISSIONING EXPENSES

Project decommissioning will incur costs associated with disposal of components, including materials which will be disposed of at a licensed facility, as required. Decommissioning costs also include backfilling, grading and restoration of the Project site as described in Section 2. Table 3 summarizes the estimates for activities associated with the major components of the Project.

Activity	Unit	Quantity	Cost per Unit	Total
Overhead and management (includes estimated permitting required)	Lump Sum	1	\$685,000	\$685,000
Solar modules; disassembly and removal	Each	353,684	\$4.00	\$1,414,736
Tracking System disassembly and removal (equivalent full trackers)	Each	5,895	\$440.00	\$2,593,800
Steel pile/post removal	Each	42,801	\$12.50	\$535,013
Remove buried AC cable	Linear Feet	98,425	\$0.40	\$39,370

Table 3	Estimated	Decommissioning	Expenses -	• 145 MWac S	Solar Array



PRELIMINARY DECOMMISSIONING PLAN KENSINGTON SOLAR PROJECT, COLUMBIANA COUNTY, OHIO

Activity	Unit	Quantity	Cost per Unit	Total		
Inverter removal with foundation	Each	126	\$1,700	\$214,200		
Access road excavation and removal	Lump Sum	1	\$360,350	\$360,350		
Perimeter fence removal	Linear Feet	207,630	\$2.80	\$581,364		
Topsoil replacement for roads and rehabilitation of site	Lump Sum	1	\$805,300	\$805,300		
Removed above ground transmission line and poles	Linear Mile	0.03	\$250,000	\$7,500		
Project substation	Each	1	\$300,000	\$300,000		
Total estimated decommissioning cost						

4.3 DECOMMISSIONING REVENUES

Potential revenue from decommissioning the Project could be realized through the sale of the solar facility components and construction materials. As previously described, the value of the decommissioned components will be higher in the early stages of the Project and decline over time. Resale of components such as solar panels is expected to be greater than salvage (i.e., scrap) value for most of the life of the Project.

Modules and other solar plant components may be sold within a secondary market or as salvage. A current sampling of reused solar panels indicates a wide range of pricing depending on age and condition (\$0.10 to \$0.40 per watt). Future pricing of solar panels is difficult to predict at this time, due to the relatively young age of the market, changes to solar panel technology, and the ever-increasing product demand. A conservative estimation of the value of solar panels at \$0.10 per watt would yield approximately \$16,800,000. Increased costs of removal, for resale versus salvage, would be expected in order to preserve the integrity of the panels; however, the net revenue would still be substantially higher than the estimated salvage value.

The resale value of components such as trackers, may decline more quickly; however, the salvage value of the steel that makes up a large portion of the tracker is expected to stay at or above the value used in this report.

The market value of steel and other materials fluctuates daily and has varied widely over the past five years. Salvage value estimates were based on an approximate five-yearaverage price of steel and copper derived from sources including on-line recycling companies and United States Geological Survey (USGS) commodity summaries. The price used to value the steel used in this report is \$204 per metric ton; aluminum at \$0.40 per pound; silicon at \$0.40 per pound and glass at \$0.05 per pound. The main component of the tracking system and piles is assumed to be salvageable steel. Solar panels are estimated to contain approximately 75 percent glass, 8 percent aluminum and 5 percent silicon. A 50 percent recovery rate was assumed for aluminum and all panel components,



due to the processing required to separate the panel components. Alternative and more efficient methods of recycling solar panels are anticipated before this Project is decommissioned, given the large number of solar facilities that are currently being developed. Table 4 summarizes the potential salvage value for the solar array components and construction materials.

Item	Unit of Measurement	Quantity per Unit	Salvage Price per Unit	Total Salvage Price per Item	Number of Items	Total
Panels - Silicon	Pounds per Panel	1.5	\$0.40/lb	\$0.600	353,684	\$212,210
Panels - Aluminum	Pounds per Panel	2.3	\$0.40/lb	\$0.920	353,684	\$325,389
Panels - Glass	Pounds per Panel	21.9	\$0.05/lb	\$1.095	353,684	\$387,284
Collection Cabling - Aluminum	Pounds per foot		\$0.19/lb	\$0.190	98,425	\$18,701
Tracking System and Posts	Metric tons per MW[DC]	43.2	\$204/MT	\$8,812.8	168.00	\$1,480,550
Substation	Each	1	\$50,000	\$50,000	1	\$50,000
						\$2,474,134

Table 4	Estimated	Decommissioning	Revenues
		J	

* Revenue based on salvage value only. Revenue from used panels at \$0.10 per watt could raise \$16,800,000 as resale versus the estimated salvage revenue.

4.4 DECOMMISSIONING COST SUMMARY

Table 5 provides a summary of the estimated cost to decommission the Project, using the information detailed in Section 4.2. Potential revenue as detailed in Section 4.3 is not included within this cost estimate. Estimates are based on 2021 prices, with no market fluctuations or inflation considered.

Table 5 Decommissioning Summary

Item	Cost/Revenue
Decommissioning Expenses	\$7,536,633

4.5 FINANCIAL ASSURANCE

Kensington Solar proposes to post decommissioning funds in the form of a performance (surety) bond prior to construction, to cover 100 percent of the decommissioning expenses listed in Table 5. No salvage value of will be considered in the bond amount. The final Decommissioning Plan will be prepared by a professional engineer registered with the state board of registration for



PRELIMINARY DECOMMISSIONING PLAN KENSINGTON SOLAR PROJECT, COLUMBIANA COUNTY, OHIO

professional engineers and surveyors. The applicant will be listed as the Principal, the insurance company as the Surety, and the OPSB as the Obligee. Kensington Solar proposes that the cost of decommissioning and bond amount be updated every 5 years by an engineer paid for by Kensington Solar and submitted to the OPSB.

Kensington Solar will also be responsible for coordinating the report of public roads damaged or modified during the decommissioning process. All Project equipment will be removed within one year.



FIGURES



Figure 1 Proposed Project Layout









Version 1.0 DRAFT December 13, 2021



FIRE AND EMERGENCY SERVICES MANUAL

Kensington Solar PV I, LLC

Columbiana County Kensington Solar

Responsibility for Fire Response Decision Making

Fire Department should be present when there is a fire at the Kensington Solar Project to ensure that the fire does not leave the property boundary of PV Solar project. Fire department should maintain a safe perimeter to make sure the fire does not "jump" the fence. Fire Department should watch the fire burn and not intervene because the fire is an electrical fire. Fire department should never enter the plant and operate equipment.

If there is a fire, we are confident that it will never get to the point where local Fire Department must cut locks to enter the project. Fire Department personnel should never operate equipment. If an electrical fire occurs, the breakers will automatically trip, all circuits will open, and the inverters will shut down in the affected arrays. This will occur to prevent further damages and possibly another fire. Liberty Power will provide the local Fire Department with an access key to the plant.

In the event the PV Plant must be De-Energized please use the following steps:

- Contact Liberty Power personnel (contact will be identified prior to construction) along with Operations & Maintenance responsible for maintaining the plant operations, numbers can be found on Contact list in manual.
- Non-Electrical Fire
 - Contact Local Utility, LP Dispatch in contact list, to open the Circuits Feeding our Plant, if they haven't already in the location of the fire.
 - Contact Plants Operations & Maintenance Team to access remotely and possibly locally to facilitate a site shut down
- Electrical Fire
 - Contact Local Utility, LP Dispatch in contact list, to open the Circuits feeding our plant, if they haven't already
 - Protection should be in place to isolate an issue if it occurs.
 - o Breakers will Trip, Circuits will Open, Inverters will shutdown

Site Access

Controlled access points (gates with keyed locks) will be installed at the site prior to energization. Address signage (along with arrays served from each entrance) will be posted at each entrance so that it is visible from the road. See Figure 1, below, for an example of proposed address signage. Knox boxes will be mounted on posts (marked with blue reflective tape) outside of each gate.

Non-Fire Related Emergency

In the event a non-fire related emergency, EMS and Police department should rely on instructions provided by the personnel calling 9-1-1. Personnel shall give explicit instructions to EMS and/or Police dispatcher on where and how to access the project site.

	Position											
	Email											
sington Solar Contact List	Phone											
Kens												
	8/EPC/EOR		ntact	iject Manager	iject Engineer	nstruction	er	mmissioning	M	ety Contact	ions and	nance Contact
	OWNER	Owner	Site Con	EPC Proj	EPC Proj	EPC Con	Manage	EPC Con	and O&I	Site Safe	Operatic	Mainter

[Placeholder Kensington Project Sign]

[Placeholder Final Site Plan]

BEFORE THE OHIO POWER SITING BOARD

In the Matter of the Application of)	
Kensington PV 1, LLC for a)	
Certificate of Environmental)	Case No. 21-764-EL-BGN
Compatibility and Public Need)	

Kensington PV 1, LLC's February 9, 2022 Responses to Staff's February 1, 2022 Data Requests

- 1. In follow-up to Kensington PV 1, LLC's Responses to Staff's data requests dated November 29, 2021, November 30, 2021, and December 1, 2021, DR#27. Please depict the 175-ft long gen-tie electric transmission line on Figure 03-2 and confirm this has also been depicted on shapefiles provided to OPSB.
 - a. Updated Figure 3-2 depicts the 175-feet long gen-tie line. The Applicant has also uploaded updated shapefiles to OPSB Staff's SharePoint.
- 2. In follow-up to Kensington PV 1, LLC's Responses to Staff's data requests dated November 29, 2021, November 30, 2021, and December 1, 2021, DR#24. Please depict the 375-ft line loop on Figure 03-2 and confirm this has also been depicted on shapefiles provided to OPSB.
 - **a.** Updated Figure 3-2 depicts the 375-feet line loop. The Applicant has also uploaded updated shapefiles to OPSB Staff's SharePoint.
- 3. Is the 375-ft. long line loop design capacity one hundred kilovolts or more and within one hundred feet of an occupied residence or institution? If yes, please provide that voltage and the calculated electric and magnetic field strength levels at one meter above ground, under the conductors and at the edge of the right-of-way for (i) Winter normal conductor rating, (ii) Emergency line loading, and (iii) Normal maximum loading.
 - *a.* There are no occupied residences or institutions within 100 feet of the 375-feet line loop. The Applicant purchased the house currently located where the switching station is proposed and will remove it prior to construction of the station.
- 4. In follow-up to Kensington PV 1, LLC's Responses to Staff's data requests dated November 29, 2021, November 30, 2021, and December 1, 2021, DR#37. It is unclear to Staff which pipeline is which, please denote on a map the Kensington Solar Project overlaying the three pipelines identified in the answer to DR#37 and label each pipeline.
 - a. Updated Figure 3-1 denotes the East Sparta Heath (Marathon), Interstate (Columbia Gas Transmission Company), and the unknown pipeline.

















BEFORE THE OHIO POWER SITING BOARD

In the Matter of the Application of)	
Kensington PV 1, LLC for a)	
Certificate of Environmental)	Case No. 21-764-EL-BGN
Compatibility and Public Need)	

Kensington PV 1, LLC's April 29, 2022 Response to Staff's April 28, 2022 Data Request

1. Staff has requested a pile load testing report for the Project.

Response: A pile load testing report is attached.



Solar Pile Load Testing Report

Kensington Solar Facility

Summitville, Columbiana County, Ohio

April 7, 2022 Revised April 21, 2022 Terracon Project No. N6225000

Prepared for:

Liberty Power Oakville, Ontario

Prepared by:

Terracon Consultants, Inc. Parma, Ohio

Facilities



Revised April 21, 2022

Liberty Power 354 Davis Road, Suite 100 Oakville, Ontario L6J2X1



Attn:Mr. Neal Cumming – Project ManagerP:(416)-453-3651E:Neal.Cumming@algonquinpower.com

Re: Solar Pile Load Testing Report Kensington Solar Facility 4765 Old William Penn Highway Summitville, Columbiana County, Ohio Terracon Project No. N6225000

Dear Mr. Cumming:

We have completed the Solar Pile Load Testing services for the above referenced project. This study was performed in general accordance with Terracon Proposal No. PN6225000 dated February 1, 2022. This report presents the findings of the full-scale pile load testing program for the proposed solar project.

We appreciate the opportunity to be of service to you on this project. If you have any questions concerning this report or if we may be of further service, please contact us.

Sincerely, Terracon Consultants, Inc.

Joseph P. Corrigan, P.E. Assistant Project Manager

Chet A lat

Chett A. Siefring, P.E. Manager – Geotechnical Services



SME Review by: Arin Barkataki, P.E. (Texas)

Terracon Consultants, Inc. 12460 Plaza Drive Parma, Ohio P (216) 459-8378 F (216) 459-8954 terracon.com
REPORT TOPICS

INTRODUCTION	1
SITE CONDITIONS	1
PROJECT DESCRIPTION	2
FULL-SCALE PILE LOAD TESTING (PLT) PROGRAM	3
PV SOLAR ARRAY FIELD - RECOMMENDATIONS FOR DESIGN	AND
CONSTRUCTION	8
GENERAL COMMENTS	13
ATTACHMENTS	15

Note: This report was originally delivered in a web-based format. For more interactive features, please view your project online at <u>client.terracon.com</u>.

ATTACHMENTS

APPENDIX A – FIELD EXPLORATION APPENDIX B – TEST PILE DRIVING DATA APPENDIX C – PILE LOAD TEST RESULTS – AXIAL COMPRESSION LOAD APPENDIX D – PILE LOAD TEST RESULTS – AXIAL TENSION LOAD APPENDIX E – PILE LOAD TEST RESULTS – LATERAL LOAD

Note: Refer to each individual Attachment for a listing of contents.



REPORT SUMMARY

Topic ¹	Overview Statement ²		
Project Description	The project site will be developed as a 135 MWac photovoltaic (PV) solar power facility.		
Subsurface Conditions	During the previously completed geotechnical investigation at the site, the surface layer contained approximately 3 to 14 inches of topsoil. Underlying the topsoil, subsurface soils consisted of interbedded cohesive and granular soils overlying weathered shale and sandstone bedrock at depths ranging between approximately 8 inches and 17 feet below existing grade.		
Pile Load Testing	This section provides the results of the full-scale uplift, compression, and lateral pile load testing.		
PV Array	The steel pile foundations for the solar array are anticipated to consist of wide flange steel piles (W6x9, or similar).		
General Comments	This section contains important information about the limitations of this geotechnical engineering report.		
 If the reader is reviewing this report as a pdf, the topics above can be used to access the appropriate section of the report by simply clicking on the topic itself. This summary is for convenience only. It should be used in conjunction with the entire report for design purposes. 			

Solar Pile Load Testing Report

Kensington Solar Facility 4765 Old William Penn Highway Summitville, Columbiana County, Ohio Terracon Project No. N6225000 Revised April 21, 2022

INTRODUCTION

This report presents the results of our full-scale pile load testing program performed for the proposed solar array to be located at 4765 Old William Penn Highway in Summitville, Columbiana County, Ohio.

The scope of services performed as part of this study for the Kensington Solar project are shown in the following table:

Type of Exploration / Test	Number
Lateral Pile Load Tests	12
Axial Tension Pile Load Tests	12
Axial Compression Pile Load Tests	6

The scope of services performed during the previously performed study at this site (Terracon report no. N6205251, dated December 3, 2020) are shown below in the table:

Type of Exploration / Test	Number
SPT Borings (B-1 through B-12) – Photovoltaic (PV) array field	12
Field Electrical Resistivity Tests – PV array field	7
Corrosion Testing – PV array field	12

Maps showing the pile load test locations are shown in the **Site Location** and **Pile Load Test Location Plan** sections, respectively. The results of the pile load tests are included in the report and in the appendices to the report.

SITE CONDITIONS

The following description of site conditions is derived from our site visit in association with the pile load testing and our review of publicly available geologic and topographic maps.

Solar Pile Load Testing Report

Kensington Solar Facility Summitville, Columbiana County, Ohio Revised April 21, 2022 Terracon Project No. N6225000



ltem	Description
Parcel Information	The project site consists of several parcels totaling about 2,264 acres, with approximately 1,132 acres being develop as part of the current project. The project site approximate coordinates are 40.6744 N, -80.8975 W. See Site Location
Existing Improvements/ Current Ground Cover	Project area mainly consists of an agricultural farm field, and some residential areas located throughout the proposed site.
Existing Topography	Surface grades across the planned solar facility range from approximately 1056 feet, MSL near where boring B-6 was advanced to approximately 1220 feet, MSL near where boring B-9 was advanced.
Geology	During the previously completed geotechnical investigation at the site, the surface layer contained approximately 3 to 14 inches of topsoil. Underlying the topsoil, subsurface soils consisted of interbedded cohesive and granular soils overlying weathered shale and sandstone bedrock at depths ranging between approximately 8 inches and 17 feet below existing grade.

PROJECT DESCRIPTION

Our initial understanding of the project was provided in our proposal and was discussed during project planning. A period of collaboration has transpired since the project was initiated, and our final understanding of the project conditions is as follows:

ltem	Description
Project Description	The project site will be developed as a 135 MWac photovoltaic (PV) solar power facility.
Proposed Structure	The steel pile foundations for the solar array are anticipated to consist of wide flange steel piles (W6x9, or similar).
Below Grade Structures	None
Maximum Loads (Estimated by Terracon)	Structural loads were not provided, but have been estimated based on our experience on projects using single axis tracking rack systems: Downward: 1.5 to 4 kips; Uplift: 2 kips (does not include frost heave loads); and Shear (Lateral): 1 kip to 3.5 kips
Grading/Slopes	It is anticipated that the site work will involve a nominal amount of cut / fill of ± 2 feet to develop final grade for the solar facility in Array areas. Final slope angles no steeper than $3H:1V$ (Horizontal: Vertical) nor taller
	than 5 feet are anticipated.
Estimated Start of Construction	TBD



Revised April 21, 2022 Terracon Project No. N6225000

FULL-SCALE PILE LOAD TESTING (PLT) PROGRAM

We completed a full-scale pile load testing program that included:

- The installation of a group of two or three test piles at 12 test locations. The piles were installed into oversized predrilled holes that were about 12 inches in diameter and about 2 feet deep.
- Performing testing under axial compressive loads for one test pile at 6 out of 12 test location (6 tests).
- Performing testing under axial tensile loads for two test piles in each group (24 tests).
- Performing testing under lateral loads for two test piles in each group (24 tests).

These activities are further described in the following sections.

PILE LOCATION PROCEDURES

The field-testing locations are indicated on the attached Pile Load Test Location Plan (Exhibit A-2) in **Appendix A**. These locations were established in the field by using a hand-held GPS (accurate to about 10 feet) and existing site features as reference points. The mapped test locations should be considered accurate only to the degree implied by the means and methods used to define them.

Test Pile Installation

The test piles consisted of wide-flange, bare steel W6x9 sections. A group of two test piles were installed at 6 test locations and a group of three test piles were installed at 6 test locations. The test piles have been identified using an alphanumeric system. The pile identification system for each location begins with "PLT" and is followed by the number corresponding to the test pile group location while the assigned letters "A", "B", and "C" indicate pile depths of 7, 10 and 7 feet below ground surface (bgs), respectively for test group numbers PLT-1, 3, 5, 9 & 11 and pile depths of 8, 11, 8 feet bgs, respectively for test group numbers 2, 4, 6, 7, 8, 10 and 12. The planned embedment depths were based on our experience with pile load testing in Ohio, the adfreeze depth, and assumed skin friction and end-bearing values.

The piles were advanced on March 15, 2022 with a track mounted Vermeer PD10 equipped with a hydraulic hammer. All the driven piles were installed into the soils at the bottom of the over-sized pre-drilled holes (approximately 12 inches in diameter and about 2 feet deep, to bypass adfreeze depth). After the 2-foot pre-drilling, five of the PLT locations (PLT-1, PLT-3, PLT-5, PLT-6, and PLT-12) had piles installed via direct-drive methods, and seven of the PLT locations (PLT-2, PLT-4, and PLT-7 through PLT-11) had piles installed in 'full-depth' pre-drilled holes. The 'full-depth' pre-drilled holes were drilled to three inches above the planned installation depth, using 6-inch-diameter augers. Prior to pile installation, the pre-drilled holes were backfilled with the auger cuttings without compaction.

Kensington Solar Facility Summitville, Columbiana County, Ohio Revised April 21, 2022 Terracon Project No. N6225000



The seven PLT locations selected for full-depth pre-drilling were based on the shallow rock depths (ranging from less than 1 foot to 6 feet below surface grade) encountered in the corresponding borings performed during the initial geotechnical exploration performed for this project (Terracon report No. N6205251). The purpose of full-depth pre-drilling was to achieve the planned pile installation depths for testing, or the minimum 5-foot embedment depth required for testing.

The time rate of installation was recorded with a stopwatch. The total time required to advance each pile to its specified embedment depth was recorded and is summarized in the following table:

Pile Location ¹	Planned Embedment Depth (feet)	Actual Embedment Depth (feet)	Drive Time ² (seconds)	Average Drive Time (seconds/foot)
PLT-1A	7.0	7.0	81	16.2
PLT-1B	10.0	8.6 *	258	39.1
PLT-2A ³	8.0	7.9 *	130	22.0
PLT-2B ³	11.0	8.6 *	146	22.1
PLT-3A	7.0	7.0	68	13.6
PLT-3B	10.0	10.0	175	21.9
PLT-4A ³	8.0	7.1*	130	25.5
PLT-4B ³	11.0	11.0	38	4.2
PLT-4C ³	8.0	8.0	19	3.2
PLT-5A	7.0	5.6 *	197	54.7
PLT-5B	10.0	5.6 *	209	56.7
PLT-5C	7.0	5.8 *	178	46.8
PLT-6A	8.0	8.0	97	16.2
PLT-6B	11.0	8.0	249	27.7
PLT-7A ³	8.0	8.0	6	1.0
PLT-7B ³	11.0	11.0	94	10.4
PLT-7C ³	8.0	8.0	23	3.8
PLT-8A ³	7.0	7.0	12	2.4
PLT-8B ³	10.0	9.1 *	143	20.1
PLT-8C ³	7.0	7.0	19	3.8
PLT-9A ³	7.0	7.0	22	4.4
PLT-9B ³	10.0	10.0	19	2.4
PLT-9C ³	7.0	7.0	8	1.6
PLT-10A ³	8.0	8.0	112	18.7
PLT-10B ³	11.0	11.0	24	2.7
PLT-11A ³	7.0	7.0	49	9.8
PLT-11B ³	10.0	9.1 *	138	19.4

Solar Pile Load Testing Report

Kensington Solar Facility Summitville, Columbiana County, Ohio Revised April 21, 2022 Terracon Project No. N6225000



Pile Location ¹	Planned Embedment Depth (feet)	Actual Embedment Depth (feet)	Drive Time ² (seconds)	Average Drive Time (seconds/foot)
PLT-12A	8.0	8.0	187	31.2
PLT-12B	11.0	7.1 *	218	42.7
PLT-12C	8.0	7.6 *	212	37.9

1. All piles were installed into approximate 12-inch-diameter pre-drilled holes to the 2-foot adfreeze depth.

2. The average time per foot excludes the upper 2 feet where piles were not in contact with soil.

3. Pile was installed into an approximate 6-inch-diameter pre-drilled hole that terminated about 3 inches above the planned pile installation depth.

"*" - Indicates pile encountered refusal prior to reaching the planned embedment depth.

Pile installation records showing individual pile drive times per foot are included in Attachments. The average drive time was about 13 seconds per foot but the maximum was about 100 seconds per foot. For purposes of this study and in accordance with industry standard, pile driving refusal has been defined as 120 seconds per foot. During driving operations, piles PLT-1B, 2A, 2B, 4A, 5A, 5B, 5C, 8B, 11B, 12B and 12C refused advancement prior to reaching planned depths due to encountering shallow bedrock.

Testing Under Axial Tensile ("pull-out") Load

We performed testing under axial tensile load for the piles at each location in accordance with ASTM D3689, *Standard Test Methods for Deep Foundations under Static Axial Tensile Load*, using the procedures generally outlined below.

Forty piles, two piles at each PLT location, were tested under axial tensile ("pull-out") load. The test piles with the designations "A" and "B" were tested under axial tensile load with the designation "A" being embedded either 7 or 10 feet below the ground surface or prior refusal, and the designation "B" being embedded either 8 or 11 feet below the ground surface or prior refusal.

Two piles at each location were tested under axial tensile ("pull-out") load. The "pull-out" load reaction was supported using an excavator. Axial loads were applied to the test pile using a Harrington 10-ton chain fall. Connections to the test piles were made using a 5-ton plate clamp (vertical) designed for connection to W-sections. The chain fall and load cell were connected in series with chains and clevises to the test piles, and the load was applied by pulling the chain through the chain fall in successive 500 lb. increments from 0 to 10,000 lbs or until ³/₄-inch of vertical movement was reached, whichever occurred first, for each test pile. Each load increment was sustained for about 60 seconds and the stabilized deflection reading of both indicator gauges was recorded.

Deflections were measured with digital gauges and loads were measured with a Digital Dynamometer 25-kip electronic load cell. The gauges and load cell were read, and the data was recorded manually by Terracon field personnel.



Testing Under Lateral Load

After testing under axial tensile load, the piles at each location were then tested under lateral load in accordance with ASTM D3966, *Standard Test Methods for Deep Foundations under Lateral Load*, using the procedures generally outlined below.

The piles were connected together to provide a reaction for the opposite post and tested simultaneously in the strong axis direction at the majority of locations. At some locations the piles were unable to be connected and were therefore tested individually using the excavator as the reaction force.

For lateral testing, the pair of piles were pulled toward each other or toward the excavator and deflections of each pile were measured. The load for the lateral tests was applied at about 3 feet above the ground surface. The loads were applied in 500-pound increments in a maximum 5 cycles from 0 pounds to a maximum lateral load of 7,000 pounds or until 2 inches of lateral movement measured at 6 inches above the ground surface was reached. Each load increment was held for at least 1 minute and the stabilized deflection readings of both indicator gauges were recorded.

Deflections were measured with digital gauges and loads were measured with a Digital Dynamometer 25-kip electronic load cell. The gauges and load cell were read, and the data was recorded manually by Terracon field personnel.

Testing Under Axial Compressive Load

Ten piles were tested under axial compressive load. Please note that test piles with the designation "C" were tested under axial compressive load and were embedded either 6 or 8 feet below the ground surface.

We performed tests under axial compressive loads as generally described below. These procedures were developed with reference to ASTM D1143, *Standard Test Methods for Deep Foundations under Static Axial Compressive Load.*

An excavator was mobilized to the site to provide a reaction for the applied vertical compression test loads. A load cell on the top of the pile, a hydraulic cylinder (jack) was placed above the load cell and under excavator bucket.

The loads were applied in 500-pound to 1,000-pound increments up to a maximum load of 13,000 pounds. Terracon utilized 13,000 pounds as a maximum load based on the equipment's capacity and safe working load. Each load increment was held for about 30 seconds and the stabilized deflection reading of both indicator gauges were recorded.



Deflections were measured with digital gauges and loads were measured with a 25-kip electronic load cell. The gauges and load cell were read, and the data was recorded manually by Terracon field personnel.

SUMMARY OF PILE LOAD TEST RESULTS

In general, the axial compressive, tensile, and lateral loads were applied at approximately 500-pound increments. The maximum applied load during the axial compression test was 13,000 pounds or until the deflection exceeded 1 inch. The maximum applied load during the axial tension test was 10,000 pounds or until the deflection exceeded 1 inch. The maximum applied load during the lateral load test was 7,000 pounds or until the deflection exceeded one inch when measured at 6 inches above the ground surface.

The individual pile load test results are provided in **Appendix C** (uplift), **Appendix D** (lateral), and **Appendix E** (compression). The following table provides a summary of each test pile location, embedment depth, total drive time, compressive load at ¼ of an inch of vertical displacement, uplift load at ¼ of an inch of vertical displacement, and the lateral load at ½ of an inch of lateral displacement:

Pile Location ¹	Actual Embedment Depth (feet)	Drive Time ² (seconds)	Uplift Load at ¼ of an inch Displacement (lbs.)	Lateral Load at ½ of an inch Displacement (lbs.)	Compressive Load at ¼ of an inch Displacement, (lbs.)
PLT-1A	7.0	81	7,350	3,050	-
PLT-1B	8.6 *	258	>10,000	3,330	-
PLT-2A ³	7.9 *	130	2,210	2,260	-
PLT-2B ³	8.6 *	146	7,110	1,570	-
PLT-3A	7.0	68	4,070	2,120	-
PLT-3B	10.0	175	7,740	1,850	-
PLT-4A ³	7.1*	130	>10,000	2,560	-
PLT-4B ³	11.0	38	8,610	760	-
PLT-4C ³	8.0	19	-	-	>13,000
PLT-5A	5.6 *	197	>10,000	3,250	-
PLT-5B	5.6 *	209	>10,000	2,850	-
PLT-5C	5.8 *	178	-	-	>13,000
PLT-6A	8.0	97	6,640	2,340	-
PLT-6B	11.0	249	>10,000	3,730	-
PLT-7A ³	8.0	6	1,130	1,270	-
PLT-7B ³	11.0	94	7,080	2,080	-
PLT-7C ³	8.0	23	-	_	9,540

Solar Pile Load Testing Report

Kensington Solar Facility Summitville, Columbiana County, Ohio Revised April 21, 2022 Terracon Project No. N6225000



Pile Location ¹	Actual Embedment Depth (feet)	Drive Time ² (seconds)	Uplift Load at ¼ of an inch Displacement (Ibs.)	Lateral Load at ½ of an inch Displacement (Ibs.)	Compressive Load at ¼ of an inch Displacement, (lbs.)
PLT-8A ³	7.0	12	4,500	1,820	-
PLT-8B ³	9.1 *	143	5,630	1,400	-
PLT-8C ³	7.0	19	-	-	>13,000
PLT-9A ³	7.0	22	5,090	1,430	-
PLT-9B ³	10.0	19	4,640	1,120	-
PLT-9C ³	7.0	8	-	-	6,750
PLT-10A ³	8.0	112	>10,000	2,660	-
PLT-10B ³	11.0	24	3,510	1,260	-
PLT-11A ³	7.0	49	5,790	2,870	-
PLT-11B ³	9.1 *	138	>10,000	2,240	-
PLT-12A	8.0	187	>10,000	1,860	-
PLT-12B	7.1 *	218	>10,000	1,880	-
PLT-12C	7.6 *	212	-	-	>13,000

1. All piles were installed into approximate 12-inch-diameter pre-drilled holes to the 2-foot adfreeze depth.

2. The average time per foot excludes the upper 2 feet where piles were not in contact with soil.

3. Pile was installed into an approximate 6-inch-diameter pre-drilled hole that terminated about 3 inches above the planned pile installation depth.

Note: The ">" sign indicates the load was achieved prior to reaching the noted deflection.

PV SOLAR ARRAY FIELD – RECOMMENDATIONS FOR DESIGN AND CONSTRUCTION

GEOTECHNICAL CONSIDERATIONS

We anticipate the preferred foundation choice for the PV panels will be driven steel piles. The inverters in the array field and the equipment pad are anticipated to be supported on similar driven steel piles or mat foundations. Settlement and strength parameters were analyzed using soil compressibility properties derived from the soil borings.

Geotechnical engineering recommendations for foundation systems and other earth-connected phases of the project are outlined in this report. The recommendations contained in this report are based upon the results of field, laboratory, and pile load testing, engineering analyses, and our current understanding of the proposed project.



SOLAR PANEL SUPPORT PILE DESIGN RECOMMENDATIONS

Adfreeze Stress

It is Terracon's professional opinion that the overburden soils encountered in the borings are frost susceptible. In cold weather climates, design to resist frost heave forces exerted on foundations is often the limiting factor in the foundation design. Specifically, pile lengths will need to be long enough to counteract potential heave forces in the seasonal frost zone.

As the frost penetrates deeper into the soil and the ground swells due to freezing, the ground surface will rise due to frost heaving. The upward displacement is due to freezing water contained in the soil voids along with the formation of ice lenses in the soil. The freezing material grips the steel pile and exerts an uplift force due to the adfreeze stress developed around the surface area of the pile. The amount of upward force depends on the following:

The thickness of ice lenses formed in the seasonal frozen ground The bond between the steel pile surface and the frozen ground The surface area of the steel pile in the seasonally frozen ground

Based on our review of soil samples, we recommend an adfreeze stress of 1,500 psf be considered when determining the adfreeze heave load on a pile. The box perimeter of the pile (two times the depth plus two times the flange width) acting over a maximum depth of about 2 feet below ground surface should be considered when determining the adfreeze heave load on a pile.

Uplift forces will likely govern the design and length of the panel array piles; therefore, uplift will likely be the primary factor in foundation costs. The factor of safety against uplift should be determined based on discussions with the owner and design engineer considering the desired level or risk, construction costs, and the long-term maintenance program.

Axial Capacity Recommendations

The axial uplift capacity of driven piles may be estimated based on skin friction developed along the perimeter of the pile, while the compression capacity may be estimated using the skin friction and end bearing. When determining embedment depths, the perimeter of a wide flange beam should be taken as twice the sum of the flange width and section depth. The upper 2 feet of soil for each pile should be neglected in the axial capacity analyses for uplift. For compression capacity, only the upper one foot should be neglected.

Based on the results of the pile load testing program, we have divided the site into four Axial Zones as shown on Exhibit A-2. Below are tables of values recommended for the different axial zones:

Solar Pile Load Testing Report

Kensington Solar Facility Summitville, Columbiana County, Ohio Revised April 21, 2022 Terracon Project No. N6225000



Direct Embedment ¹				
Axial Zone	Minimum Drive Time (seconds)	Embedment Depth (ftbgs)	Ultimate Uplift and Compression Skin Friction q _s (psf) ^{1, 2}	Ultimate End Bearing (Ibs) ^{2, 3}
Zone 1 (PLT-1, 5, 6 and 12)	187	2 to 9	800	3,000
Zone 3 (PLT-3)	68	2 to 10	500	3,000

 The ultimate skin friction is based on the results of the uplift load testing. The upper 2.0 feet should be neglected in pile design for axial loading conditions for uplift due to loss of soil strength or contact with pile due to freeze-thaw action, seasonal water content variations, and other potential surface disturbances.

For Allowable Strength Design (ASD), we recommend the allowable capacity be determined by applying a minimum factor of safety of at least 1.5 to the ultimate values for the non-adfreeze effected loading conditions.
 Values applies has to pilos applied do a minimum of C fact has

3. Values applicable to piles embedded a minimum of 6 feet bgs.

Pre-drilled Condition ¹				
Axial Zone	Minimum Drive Time (seconds)	Embedment Depth (ftbgs)Ultimate Uplift and Compression Skin Friction qs (psf) 1, 2		Ultimate End Bearing (Ibs) ^{2, 3}
Zone 2 (PLT-4, 10 and 11)	112	2 to 11	600	3,000
Zone 4 (PLT-2, 7, 8 and 9)	104	2 to 11	350	3,000

 The ultimate skin friction is based on the results of the uplift load testing. The upper 2.0 feet should be neglected in pile design for axial loading conditions for uplift due to loss of soil strength or contact with pile due to freeze-thaw action, seasonal water content variations, and other potential surface disturbances.

For Allowable Strength Design (ASD), we recommend the allowable capacity be determined by applying a minimum factor of safety of at least 1.5 to the ultimate values for the non-adfreeze effected loading conditions.
 Values applicable to piles embedded a minimum of 6 feet bgs.

The above values are to be used in the following equations to obtain the ultimate uplift or compression load capacity of a pile:

 $Q_{ult \ (compressive)} = Q_{ult \ (end)} + Q_{ult \ (uplift)}$

 $Q_{ult (uplift)} = H x P x q_s$

Q_{ult} = Ultimate uplift or compression capacity of post (lbs.)

Kensington Solar Facility Summitville, Columbiana County, Ohio Revised April 21, 2022 Terracon Project No. N6225000



 $Q_{ult (end)}$ = Ultimate end bearing capacity per table above (lbs.) H = Depth of embedment of pile (ft.) P = Perimeter area/ft. of pile. (i.e. W6x9 = 1.64 sf/ft.)

 q_s = Unit skin friction per table above (psf).

An example calculation to determine the allowable capacity for a W6x9 pile in tension and founded at a depth of 8 feet in the area of Zone 2 would be as follows:

allowable (uplift) =
$$(8 - 2.0) x 1.64 x \frac{500}{1.5} = 3,610 lbs.$$

Should different pile sizes be used, such as W6x15 piles, the pile load test results from the W6x9 piles may be applied in the above equation, provided the correct embedment depths and pile perimeter areas are used to calculate the axial capacity. The provided skin friction values are applicable for piles that are driven using equipment similar to a Vermeer PD10 pile driver with a hydraulically operated hammer. If a smaller or larger drive hammer is used, we recommend that Terracon be consulted to determine the minimum drive time based on the actual equipment to be used.

For Allowable Stress Design (ASD), we recommend the allowable skin friction and end bearing values be determined by applying a factor of safety of at least 1.5 to the ultimate value.

Piles should have a minimum center-to-center spacing of at least 3 times their largest crosssectional dimension to prevent reduction in the axial capacities due to group effects.

Final pile design to be completed by an engineering licensed in the State of Ohio based upon information contained in this geotechnical report and independent pile load testing.

Lateral Capacity Recommendations

Lateral load response of pile foundations was calculated using the computer program *L-Pile 2019*, by Ensoft, Inc. The stiffness of the pile and the stress-strain properties of the surrounding soils determine the lateral resistance of the foundation. We modeled the lateral response of the tested piles to evaluate L-Pile input parameters that can be used for design of the production piles. Recommended L-Pile input parameters for lateral load analysis for driven pile foundations are shown in the following table:

Depth Interval of	(P-y) Curve Type	Effective Unit	Cohesion (psf)	Friction Angle
Layer (feet)	Model	Weight (pcf)		(deg)
0 to 2	Sand (Reese) ¹	115	-	29

Solar Pile Load Testing Report

Kensington Solar Facility Summitville, Columbiana County, Ohio Revised April 21, 2022 Terracon Project No. N6225000



Depth Interval of Layer (feet)	(P-y) Curve Type Model	Effective Unit Weight (pcf)	Cohesion (psf)	Friction Angle (deg)
2 to 5	Stiff Clay w/o Free Water (Reese) ¹	120	1,000	-
5 to 20	Sand (Reese) ¹	120	-	36
1. Use default	value for Strain Factor, 50			

Based on the results of the pile load testing program, the lateral load test results were varied between the different embedment depths and test locations. We have divided the site into two Lateral Zones as shown on Exhibit A-2. The following table of p-multiplier values should be used for the corresponding embedment depths and Lateral Zones:

Embedment	P-Multiplier ^{1, 2, 3}								
Depth (feet bgs)	Lateral Zone A (Direct Embedment) (PLT-1, 3, 5, 6 and 12)	Lateral Zone B (Pre-drilled Condition) (PLT-2, 4, 7, 8, 9, 10 and 11)							
6	6.0	-							
7	4.0	2.0							
8	3.3	1.8							
9	5.5	1.7							
10	2.7	0.8							
11	-	0.9							

1. The P-multiplier should be reduced by 30% in the upper 2.0 feet to account for seasonal freeze/thaw effects.

2. Linearly interpolate between values for piles embedded to depths between those provided in the table.

3. For piles embedded below depths of 10 feet in Zones A and B use the p-multiplier given for the 10-foot embedment depth piles in Zone A and B.

L-PILE analyses were performed by applying the field test load that resulted in approximately ¹/₂inch deflection at a point about six inches above the ground surface. The shear load was applied at approximately 3 feet above the ground surface. The effective unit weight and cohesion were based on the results of the borings. The p- multiplier was then adjusted (by trial and error method) such that the applied load resulted in a deflection value that matched the load test results. Please note that this procedure was based on only one discrete set of data determined at about six inches from the ground surface during the field load testing. These results should be used for L-PILE analysis only using the 2019 version of L-Pile. These parameters are only applicable to piles embedded between six and eleven feet below grade. In our evaluation, the piles were modeled as a Steel AISC Section Strong Axis.



The structural engineer should evaluate the moment capacity of the pile as part of their structural evaluation. Piles should have a minimum center-to-center spacing of at least five times their largest cross-sectional dimension in the direction of the lateral loads, or the lateral capacities should be reduced due to group effects. If piles will be spaced closer than five times their largest cross-sectional dimension, we should be notified to provide supplemental recommendations regarding resistance to lateral loads.

DRIVEN PILE CONSTRUCTION CONSIDERATIONS

Based on the field exploration and laboratory testing, it is our opinion that the soils on the site are suitable for pile installation. However, pre-drilling may be required for piles that are designed to be driven 6 feet or deeper, due to presence of shallow bedrock at certain locations of the site.

A representative of the geotechnical engineer should observe pile driving operations. Each pile should be observed and checked for buckling, crimping and alignment in addition to recording penetration resistance, depth of embedment, and general pile driving operations.

GENERAL COMMENTS

Our analysis and opinions are based upon our understanding of the project, the geotechnical conditions in the area, and the data obtained from our site exploration. Natural variations will occur between exploration point locations or due to the modifying effects of construction or weather. The nature and extent of such variations may not become evident until during or after construction. Terracon should be retained as the Geotechnical Engineer, where noted in this report, to provide observation and testing services during pertinent construction phases. If variations appear, we can provide further evaluation and supplemental recommendations. If variations are noted in the absence of our observation and testing services on-site, we should be immediately notified so that we can provide evaluation and supplemental recommendations.

Our Scope of Services does not include either specifically or by implication any environmental or biological (e.g., mold, fungi, bacteria) assessment of the site or identification or prevention of pollutants, hazardous materials or conditions. If the owner is concerned about the potential for such contamination or pollution, other studies should be undertaken.

Our services and any correspondence or collaboration through this system are intended for the sole benefit and exclusive use of our client for specific application to the project discussed and are accomplished in accordance with generally accepted geotechnical engineering practices with no third-party beneficiaries intended. Any third-party access to services or correspondence is solely for information purposes to support the services provided by Terracon to our client. Reliance upon the services and any work product is limited to our client, and is not intended for

Solar Pile Load Testing Report

Kensington Solar Facility Summitville, Columbiana County, Ohio Revised April 21, 2022 Terracon Project No. N6225000



third parties. Any use or reliance of the provided information by third parties is done solely at their own risk. No warranties, either express or implied, are intended or made.

Site characteristics as provided are for design purposes and not to estimate excavation cost. Any use of our report in that regard is done at the sole risk of the excavating cost estimator as there may be variations on the site that are not apparent in the data that could significantly impact excavation cost. Any parties charged with estimating excavation costs should seek their own site characterization for specific purposes to obtain the specific level of detail necessary for costing. Site safety, and cost estimating including, excavation support, and dewatering requirements/design are the responsibility of others. If changes in the nature, design, or location of the project are planned, our conclusions and recommendations shall not be considered valid unless we review the changes and either verify or modify our conclusions in writing.

ATTACHMENTS

Responsive Resourceful Reliable

APPENDIX A – FIELD EXPLORATION

Contents:

Exhibit A-1	Site Location Plan
Exhibit A-2	Pile Load Test Location Plan

Note: All attachments are one page unless noted above.

SITE LOCATION

Kensington Solar Facility Summitville, Columbiana County, Ohio Revised April 21, 2022 Terracon Project No. N6225000



DIAGRAM IS FOR GENERAL LOCATION ONLY, AND IS NOT INTENDED FOR CONSTRUCTION PURPOSES

MAP PROVIDED BY MICROSOFT BING MAPS

lerracon

GeoReport

PILE LOAD TEST LOCATION PLAN

Kensington Solar Facility Summitville, Columbiana County, Ohio Revised April 21, 2022 Terracon Project No. N6225000



DIAGRAM IS FOR GENERAL LOCATION ONLY, AND IS NOT INTENDED FOR CONSTRUCTION PURPOSES

MAP PROVIDED BY MICROSOFT BING MAPS

Terracon

GeoReport

APPENDIX B – TEST PILE DRIVING DATA

Contents:

Exhibit B-1 to B-4

Test Pile Driving Records (4 pages)

Note: All attachments are one page unless noted above.



Donth (foot)			Cı	umulative	Driving Ti	me, secon	ds		
Depth (leet)	PLT-1A	PLT-1B	PLT-1C	PLT-2A	PLT-2B	PLT-2C	PLT-3A	PLT-3B	PLT-3C
1	0.0	0.0		0.0	0.0		0.0	0.0	
2	0.0	0.0		0.0	0.0		0.0	0.0	
3	3.0	5.0		1.0	1.0		2.0	3.0	
4	17.0	20.0		4.0	6.0		11.0	12.0	
5	38.0	43.0		5.0	8.0		24.0	26.0	
6	57.0	66.0		8.0	12.0		48.0	47.0	
7	81.0	90.0		10.0	18.0		68.0	73.0	
8		138.0		130.0	26.0			107.0	
9		258.0			146.0			139.0	
10								175.0	
11									
12									
Embedment Depth, ft	7.0	8.6	0.0	7.9	8.6	0.0	7.0	10.0	0.0
Total Drive Time, sec	81.0	258.0	0.0	130.0	146.0	0.0	68.0	175.0	0.0
Average, sec/ft	16.2	39.1	0.0	22.0	22.1	0.0	13.6	21.9	0.0

NOTES:

Piles advanced with Vermeer PD-10 hydraulic ram.

Installation depth started at 24 inches below ground surface





Donth (foot)			Cı	umulative	Driving Ti	ne, secon	ds		
Deptil (leet)	PLT-4A	PLT-4B	PLT-4C	PLT-5A	PLT-5B	PLT-5C	PLT-6A	PLT-6B	PLT-6C
1	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
2	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
3	0.0	0.0	0.0	7.0	3.0	7.0	5.0	4.0	
4	1.0	0.0	0.0	34.0	20.0	15.0	15.0	13.0	
5	2.0	0.0	0.0	77.0	84.0	58.0	27.0	28.0	
6	6.0	0.0	3.0	197.0	204.0	178.0	45.0	50.0	
7	10.0	2.0	7.0				67.0	80.0	
8	130.0	7.0	19.0				97.0	120.0	
9		11.0						161.0	
10		18.0						203.0	
11		38.0						249.0	
12									
Embedment Depth, ft	7.1	11.0	8.0	5.6	5.6	5.8	8.0	11.0	0.0
Total Drive Time, sec	130.0	38.0	19.0	197.0	204.0	178.0	97.0	249.0	0.0
Average, sec/ft	25.5	4.2	3.2	54.7	56.7	46.8	16.2	27.7	0.0
NOTES:							-		

Piles advanced with Vermeer PD-10 hydraulic ram. Installation depth started at 24 inches below ground surface





Donth (foot)			Cı	imulative	Driving Ti	ne, secon	ds		
Deptil (leet)	PLT-7A	PLT-7B	PLT-7C	PLT-8A	PLT-8B	PLT-8C	PLT-9A	PLT-9B	PLT-9C
1	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
2	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
3	0.0	0.0	1.0	0.0	1.0	1.0	0.0	0.0	0.0
4	0.0	5.0	3.0	1.0	2.0	3.0	2.0	0.0	0.0
5	0.0	11.0	7.0	2.0	5.0	5.0	7.0	1.0	1.0
6	0.0	16.0	12.0	4.0	8.0	9.0	13.0	2.0	3.0
7	1.0	19.0	16.0	12.0	12.0	19.0	22.0	5.0	8.0
8	6.0	25.0	23.0		17.0			8.0	
9		32.0			23.0			11.0	
10		44.0			143.0			19.0	
11		94.0							
12									
Embedment Depth, ft	8.0	11.0	8.0	7.0	9.1	7.0	7.0	10.0	7.0
Total Drive Time, sec	6.0	94.0	23.0	12.0	143.0	19.0	22.0	19.0	8.0
Average, sec/ft	1.0	10.4	3.8	2.4	20.1	3.8	4.4	2.4	1.6

NOTES:

Piles advanced with Vermeer PD-10 hydraulic ram.

Installation depth started at 24 inches below ground surface





Donth (foot)			Cı	imulative	Driving Ti	ne, secon	ds		
Deptil (leet)	PLT-10A	PLT-10B	PLT-10C	PLT-11A	PLT-11B	PLT-11C	PLT-12A	PLT-12B	PLT-12C
1	0.0	0.0		0.0	0.0		0.0	0.0	0.0
2	0.0	0.0		0.0	0.0		0.0	0.0	0.0
3	2.0	0.0		2.0	0.0		2.0	0.0	0.0
4	4.0	0.0		8.0	1.0		9.0	7.0	4.0
5	11.0	0.0		13.0	2.0		20.0	18.0	17.0
6	31.0	0.0		25.0	5.0		49.0	41.0	41.0
7	67.0	0.0		49.0	9.0		122.0	98.0	92.0
8	112.0	3.0			13.0		187.0	218.0	212.0
9		9.0			18.0				
10		14.0			138.0				
11		24.0							
12									
Embedment Depth, ft	8.0	11.0	0.0	7.0	9.1	0.0	8.0	7.1	7.6
Total Drive Time, sec	112.0	24.0	0.0	49.0	138.0	0.0	187.0	218.0	212.0
Average, sec/ft	18.7	2.7	0.0	9.8	19.4	0.0	31.2	42.7	37.9

NOTES:

Piles advanced with Vermeer PD-10 hydraulic

ram. Installation depth started at 24 inches

below ground surface



APPENDIX C – PILE LOAD TEST RESULTS – AXIAL TENSION LOAD

Contents:

Exhibit C-1 to C-24 Tension Load Test Results (24 pages)

Note: All attachments are one page unless noted above.

Tension Load Test Result for PLT-1A

Project Information

Project Name: Kensington Solar Р

Project Name: Kensington Solar		Tension Te	st Results	Davisson Offset Limit Lines		
Project Location: Summitville, Ohio	% of	Axial		Elastic	Davisson Offset	
Project Number: N6225000	Design	Load	Deflection Δ (in.)	Data (in)	Limit (in)	Comments
	Load	[lbs]	Gauges #1 & #2	(PL/AE)	(0.15+D/120+(PL/AE))	
	0%	0	0.000	0.000	0.199	
Axial Load Test Set Up	5%	500	0.003	0.000	0.199	
Number of Gauges: 2	10%	1000	0.004	0.000	0.199	
Height of Gauges [in]: 6	15%	1500	0.006	0.000	0.199	
Load Cell: 25k Ed Jr.	20%	2000	0.007	0.000	0.199	
·	25%	2500	0.008	0.000	0.199	
	30%	3000	0.011	0.000	0.199	
Test Date and Representative	35%	3500	0.014	0.000	0.199	
Tested By Terracon Rep: I. McGougan	40%	4000	0.018	0.000	0.199	
Date Tested:	45%	4500	0.022	0.000	0.200	
·	50%	5000	0.027	0.000	0.200	
	55%	5500	0.036	0.000	0.200	
Pile Information	60%	6000	0.052	0.000	0.200	
Pile ID: PLT-1A	65%	6500	0.070	0.001	0.200	
Latitude: 40.70095	70%	7000	0.159	0.001	0.200	
Longitude: -80.88916	75%	7500	0.288	0.001	0.200	
Pile Type: W6x9	80%	8000	0.772	0.001	0.200	
Pile Embedment Depth [in]: 7	85%	8500		0.001	0.200	
Pile Diameter [in]: 5.9	90%	9000		0.001	0.200	
Pile Stick-Up [in]: 36	95%	9500		0.001	0.200	
Axial Design Load [lbs]: 10000	100%	10000		0.001	0.200	
Pile Area [sq. in]: 2.96	0%	0	0.726	0.000	0.199	
Elastic Modulus [ksi]: 29,000						
Drive Time [sec]: 81						



Davisson Offset Limit Lines

Tension Load Test Result for PLT-1B

Project Information

Project Name: Kensington Solar Project Location: Summitville, Ohio Project Number: N6225000

Project Number: N6225000	Design	Load	Deflection Δ (in.)	Data (in)	Limit (in)	Comments
	Load	[adi]	Gauges #1 & #2	(PL/AE)	(0.15+D/120+(PL/AE))	
	0%	0	0.000	0.000	0.199	
Axial Load Test Set Up	5%	500	0.002	0.000	0.199	
Number of Gauges: 2	10%	1000	0.003	0.000	0.199	
Height of Gauges [in]: 6	15%	1500	0.004	0.000	0.199	
Load Cell: 25k Ed Jr.	20%	2000	0.006	0.000	0.199	
·	25%	2500	0.002	0.000	0.199	
	30%	3000	0.007	0.000	0.199	
Test Date and Representative	35%	3500	0.008	0.000	0.200	
Tested By Terracon Rep: I. McGougan	40%	4000	0.008	0.000	0.200	
Date Tested:	45%	4500	0.008	0.000	0.200	
'	50%	5000	0.008	0.001	0.200	
	54%	5400	0.009	0.001	0.200	
Pile Information	60%	6000	0.009	0.001	0.200	
Pile ID: PLT-1B	65%	6500	0.010	0.001	0.200	
Latitude: 40.70095	70%	7000	0.011	0.001	0.200	
Longitude: -80.88916	75%	7500	0.012	0.001	0.200	
Pile Type: W6x9	80%	8000	0.012	0.001	0.200	
Pile Embedment Depth [in]: 8.6	85%	8500	0.012	0.001	0.200	
Pile Diameter [in]: 5.9	90%	9000	0.012	0.001	0.200	
Pile Stick-Up [in]: 36	95%	9500	0.012	0.001	0.200	
Axial Design Load [lbs]: 10000	100%	10000	0.013	0.001	0.200	
Pile Area [sq. in]: 2.96	0%	0	0.005	0.000	0.199	

Tension Test Results



Davisson Offset Limit Lines

Tension Load Test Result for PLT-2A

Project Information

Project Name: Kensington Solar Project Location: Summitville, Ohio Project Number: N6225000

Project Number: N6225000	Design Load	Load [lbs]	Deflection ∆ (in.) Gauges #1 & #2	Data (in) (PL/AE)	Limit (in) (0.15+D/120+(PL/AE))	Comments
	0%	0	0.000	0.000	0.199	
Axial Load Test Set Up	5%	500	0.001	0.000	0.199	
Number of Gauges: 2	10%	1000	0.005	0.000	0.199	
Height of Gauges [in]: 6	15%	1500	0.018	0.000	0.199	
Load Cell: 25k Ed Jr.	20%	2000	0.067	0.000	0.199	
·	25%	2500	0.760	0.000	0.199	
	30%	3000		0.000	0.199	
Test Date and Representative	35%	3500		0.000	0.199	
Tested By Terracon Rep: I. McGougan	40%	4000		0.000	0.200	
Date Tested:	45%	4500		0.000	0.200	
·	50%	5000		0.000	0.200	
	55%	5500		0.001	0.200	
Pile Information	60%	6000		0.001	0.200	
Pile ID: PLT-2A	65%	6500		0.001	0.200	
Latitude: 40.69342	70%	7000		0.001	0.200	
Longitude: -80.88928	75%	7500		0.001	0.200	
Pile Type: W6x9	80%	8000		0.001	0.200	
Pile Embedment Depth [in]: 7.9	85%	8500		0.001	0.200	
Pile Diameter [in]: 5.9	90%	9000		0.001	0.200	
Pile Stick-Up [in]: 36	95%	9500		0.001	0.200	
Axial Design Load [lbs]: 10000	100%	10000		0.001	0.200	
Pile Area [sq. in]: 2.96	0%	0	0.743	0.000	0.199	

Tension Test Results



Davisson Offset Limit Lines

Tension Load Test Result for PLT-2B

Project Information

Project Name: Kensington Solar Project Location: Summitville, Ohio Project Number: N6225000

Project Number: N6225000	Design Load	Load [lbs]	Deflection ∆ (in.) Gauges #1 & #2	Data (in) (PL/AE)	Limit (in) (0.15+D/120+(PL/AE))	Comments
	0%	0	0.000	0.000	0.199	
Axial Load Test Set Up	5%	500	0.002	0.000	0.199	
Number of Gauges: 2	10%	1000	0.003	0.000	0.199	
Height of Gauges [in]: 6	15%	1500	0.004	0.000	0.199	
Load Cell: 25k Ed Jr.	20%	2000	0.004	0.000	0.199	
·	25%	2500	0.005	0.000	0.199	
	30%	3000	0.005	0.000	0.199	
Test Date and Representative	35%	3500	0.022	0.000	0.200	
Tested By Terracon Rep: I. McGougan	40%	4000	0.007	0.000	0.200	
Date Tested:	45%	4500	0.009	0.000	0.200	
·	50%	5000	0.010	0.001	0.200	
	55%	5500	0.014	0.001	0.200	
Pile Information	60%	6000	0.022	0.001	0.200	
Pile ID: PLT-2B	65%	6500	0.037	0.001	0.200	
Latitude: 40.69342	70%	7000	0.101	0.001	0.200	
Longitude: -80.88928	75%	7500	0.784	0.001	0.200	
Pile Type: W6x9	80%	8000		0.001	0.200	
Pile Embedment Depth [in]: 8.6	85%	8500		0.001	0.200	
Pile Diameter [in]: 5.9	90%	9000		0.001	0.200	
Pile Stick-Up [in]: 36	95%	9500		0.001	0.200	
Axial Design Load [lbs]: 10000	100%	10000		0.001	0.200	
Pile Area [sq. in]: 2.96	0%	0	0.766	0.000	0.199	

Tension Test Results



Davisson Offset Limit Lines

Tension Load Test Result for PLT-3A

Project Information

Project Name: Kensington Solar Project Location: Summitville, Ohio Project Number: N6225000

Project Number: N6225000	Design	Load	Deflection ∆ (in.)	Data (in)	Limit (in)	Comments
	0%	[IDS]	0 000	(PL/AE)	(0.15+D/120+(PL/AE)) 0.199	
Axial Load Test Set Up	5%	500	0.002	0.000	0.199	
Number of Gauges: 2	10%	1000	0.003	0.000	0.199	
Height of Gauges [in]: 6	15%	1500	0.005	0.000	0.199	
Load Cell: 25k Ed Jr.	20%	2000	0.009	0.000	0.199	
•	25%	2500	0.018	0.000	0.199	
	30%	3000	0.058	0.000	0.199	
Test Date and Representative	35%	3500	0.130	0.000	0.199	
Tested By Terracon Rep: I. McGougan	40%	4000	0.210	0.000	0.199	
Date Tested:	45%	4500	0.500	0.000	0.200	
•	50%	5000	0.753	0.000	0.200	
	55%	5500		0.000	0.200	
Pile Information	60%	6000		0.000	0.200	
Pile ID: PLT-3A	65%	6500		0.001	0.200	
Latitude: 40.69014	70%	7000		0.001	0.200	
Longitude: -80.88951	75%	7500		0.001	0.200	
Pile Type: W6x9	80%	8000		0.001	0.200	
Pile Embedment Depth [in]: 7	85%	8500		0.001	0.200	
Pile Diameter [in]: 5.9	90%	9000		0.001	0.200	
Pile Stick-Up [in]: 36	95%	9500		0.001	0.200	
Axial Design Load [lbs]: 10000	100%	10000		0.001	0.200	
Pile Area [sq. in]: 2.96	0%	0	0.725	0.000	0.199	

Tension Test Results



Davisson Offset Limit Lines

Tension Load Test Result for PLT-3B

Project Information

Project Name: Kensington Solar Project Location: Summitville, Ohio Project Number: N6225000

Project Number: N6225000	Design Load	Load [lbs]	Deflection ∆ (in.) Gauges #1 & #2	Data (in) (PL/AE)	Limit (in) (0.15+D/120+(PL/AE))	Comments
	0%	0	0.000	0.000	0.199	
Axial Load Test Set Up	5%	500	0.003	0.000	0.199	
Number of Gauges: 2	10%	1000	0.003	0.000	0.199	
Height of Gauges [in]: 6	15%	1500	0.003	0.000	0.199	
Load Cell: 25k Ed Jr.	20%	2000	0.003	0.000	0.199	
	25%	2500	0.003	0.000	0.199	
	30%	3000	0.005	0.000	0.200	
Test Date and Representative	35%	3500	0.006	0.000	0.200	
Tested By Terracon Rep: I. McGougan	40%	4000	0.010	0.000	0.200	
Date Tested:	45%	4500	0.014	0.001	0.200	
·	50%	5000	0.020	0.001	0.200	
	55%	5500	0.034	0.001	0.200	
Pile Information	60%	6000	0.060	0.001	0.200	
Pile ID: PLT-3B	65%	6500	0.088	0.001	0.200	
Latitude: 40.69014	70%	7000	0.122	0.001	0.200	
Longitude: -80.88951	75%	7500	0.184	0.001	0.200	
Pile Type: W6x9	80%	8000	0.320	0.001	0.200	
Pile Embedment Depth [in]: 10	85%	8500	0.397	0.001	0.200	
Pile Diameter [in]: 5.9	90%	9000	0.513	0.001	0.200	
Pile Stick-Up [in]: 36	95%	9500	0.761	0.001	0.200	
Axial Design Load [lbs]: 10000	96%	9600		0.001	0.200	
Pile Area [sq. in]: 2.96	0%	0	0.732	0.000	0.199	

Tension Test Results



Davisson Offset Limit Lines

Tension Load Test Result for PLT-4A

Project Information

Project Name: Kensington Solar Project Location: Summitville, Ohio Project Number: N6225000

Project Number: N6225000	Design	Load	Deflection Δ (in.)	Data (in)	Limit (in)	Comments
	Load	[lbs]	Gauges #1 & #2	(PL/AE)	(0.15+D/120+(PL/AE))	
	0%	0	0.000	0.000	0.199	
Axial Load Test Set Up	5%	500	-0.001	0.000	0.199	
Number of Gauges: 2	10%	1000	0.006	0.000	0.199	
Height of Gauges [in]: 6	15%	1500	0.007	0.000	0.199	
Load Cell: 25k Ed Jr.	20%	2000	0.008	0.000	0.199	
'	25%	2500	0.005	0.000	0.199	
	30%	3000	0.013	0.000	0.199	
Test Date and Representative	35%	3500	0.015	0.000	0.199	
Tested By Terracon Rep: I. McGougan	40%	4000	0.017	0.000	0.199	
Date Tested:	45%	4500	0.020	0.000	0.200	
· ·	50%	5000	0.023	0.000	0.200	
	55%	5500	0.026	0.000	0.200	
Pile Information	60%	6000	0.028	0.000	0.200	
Pile ID: PLT-4A	65%	6500	0.033	0.001	0.200	
Latitude: 40.68967	70%	7000	0.036	0.001	0.200	
Longitude: -80.89409	75%	7500	0.039	0.001	0.200	
Pile Type: W6x9	80%	8000	0.043	0.001	0.200	
Pile Embedment Depth [in]: 7.1	85%	8500	0.048	0.001	0.200	
Pile Diameter [in]: 5.9	90%	9000	0.051	0.001	0.200	
Pile Stick-Up [in]: 36	95%	9500	0.055	0.001	0.200	
Axial Design Load [lbs]: 10000	100%	10000	0.060	0.001	0.200	
Pile Area [sq. in]: 2.96	0%	0	0.016	0.000	0.199	

Tension Test Results



Davisson Offset Limit Lines

Tension Load Test Result for PLT-4B

Project Information

Project Name: Kensington Solar Project Location: Summitville, Ohio Project Number: N6225000

Project Location: 06225000	Design Load	Load [lbs]	Deflection ∆ (in.) Gauges #1 & #2	Data (in) (PL/AE)	Limit (in) (0.15+D/120+(PL/AE))	Comments
	0%	0	0.000	0.000	0.199	
Axial Load Test Set Up	5%	500	0.001	0.000	0.199	
Number of Gauges: 2	10%	1000	0.002	0.000	0.199	
Height of Gauges [in]: 6	15%	1500	0.002	0.000	0.199	
Load Cell: 25k Ed Jr.	20%	2000	0.004	0.000	0.199	
	25%	2500	0.007	0.000	0.199	
	30%	3000	0.009	0.000	0.200	
Test Date and Representative	35%	3500	0.013	0.000	0.200	
Tested By Terracon Rep: I. McGougan	40%	4000	0.017	0.001	0.200	
Date Tested:	45%	4500	0.019	0.001	0.200	
·	50%	5000	0.021	0.001	0.200	
	55%	5500	0.026	0.001	0.200	
Pile Information	60%	6000	0.031	0.001	0.200	
Pile ID: PLT-4B	65%	6500	0.038	0.001	0.200	
Latitude: 40.68967	70%	7000	0.048	0.001	0.200	
Longitude: -80.89409	75%	7500	0.058	0.001	0.200	
Pile Type: W6x9	80%	8000		0.001	0.200	
Pile Embedment Depth [in]: 11	85%	8500		0.001	0.200	
Pile Diameter [in]: 5.9	90%	9000		0.001	0.200	
Pile Stick-Up [in]: 36	95%	9500		0.001	0.200	
Axial Design Load [lbs]: 10000	100%	10000		0.001	0.200	
Pile Area [sq. in]: 2.96	0%	0	0.725	0.000	0.199	

Tension Test Results



Davisson Offset Limit Lines

Tension Load Test Result for PLT-5A

Project Information

Project Name: Kensington Solar Project Location: Summitville, Ohio Project Number: N6225000

Project Number: N6225000	Design Load	Load [lbs]	Deflection ∆ (in.) Gauges #1 & #2	Data (in) (PL/AF)	Limit (in) (0 15+D/120+(PI /AE))	Comments
	0%	0	0.000	0.000	0.199	
Axial Load Test Set Up	5%	500	0.002	0.000	0.199	
Number of Gauges: 2	10%	1000	0.003	0.000	0.199	
Height of Gauges [in]: 6	15%	1500	0.004	0.000	0.199	
Load Cell: 25k Ed Jr.	20%	2000	0.005	0.000	0.199	
	24%	2400	0.006	0.000	0.199	
	30%	3000	0.006	0.000	0.199	
Test Date and Representative	35%	3500	0.004	0.000	0.199	
Tested By Terracon Rep: I. McGougan	40%	4000	0.007	0.000	0.199	
Date Tested:	45%	4500	0.007	0.000	0.199	
·	50%	5000	0.007	0.000	0.199	
	55%	5500	0.008	0.000	0.200	
Pile Information	60%	6000	0.009	0.000	0.200	
Pile ID: PLT-5A	65%	6500	0.010	0.000	0.200	
Latitude: 40.67996	70%	7000	0.011	0.000	0.200	
Longitude: -80.89076	75%	7500	0.011	0.000	0.200	
Pile Type: W6x9	80%	8000	0.012	0.001	0.200	
Pile Embedment Depth [in]: 5.6	85%	8500	0.013	0.001	0.200	
Pile Diameter [in]: 5.9	90%	9000	0.015	0.001	0.200	
Pile Stick-Up [in]: 36	95%	9500	0.016	0.001	0.200	
Axial Design Load [lbs]: 10000	100%	10000	0.017	0.001	0.200	
Pile Area [sq. in]: 2.96	0%	0	0.009	0.000	0.199	

Tension Test Results



Davisson Offset Limit Lines

Tension Load Test Result for PLT-5B

Project Information

Project Name: Kensington Solar Project Location: Summitville, Ohio Project Number: N6225000

Project Number: N6225000	Design Load	Load [lbs]	Deflection ∆ (in.) Gauges #1 & #2	Data (in) (PL/AE)	Limit (in) (0.15+D/120+(PL/AE))	Comments
	0%	0	0.000	0.000	0.199	
Axial Load Test Set Up	5%	500	0.001	0.000	0.199	
Number of Gauges: 2	10%	1000	0.002	0.000	0.199	
Height of Gauges [in]: 6	15%	1500	0.003	0.000	0.199	
Load Cell: 25k Ed Jr.	20%	2000	0.004	0.000	0.199	
·	25%	2500	0.005	0.000	0.199	
	30%	3000	0.006	0.000	0.199	
Test Date and Representative	35%	3500	0.007	0.000	0.199	
Tested By Terracon Rep: I. McGougan	40%	4000	0.008	0.000	0.199	
Date Tested:	45%	4500	0.009	0.000	0.199	
· ·	50%	5000	0.009	0.000	0.199	
	55%	5500	0.010	0.000	0.200	
Pile Information	60%	6000	0.012	0.000	0.200	
Pile ID: PLT-5B	65%	6500	0.013	0.000	0.200	
Latitude: 40.67996	70%	7000	0.015	0.000	0.200	
Longitude: -80.89076	75%	7500	0.016	0.000	0.200	
Pile Type: W6x9	80%	8000	0.017	0.001	0.200	
Pile Embedment Depth [in]: 5.6	85%	8500	0.018	0.001	0.200	
Pile Diameter [in]: 5.9	90%	9000	0.020	0.001	0.200	
Pile Stick-Up [in]: 36	95%	9500	0.021	0.001	0.200	
Axial Design Load [lbs]: 10000	100%	10000	0.023	0.001	0.200	
Pile Area [sq. in]: 2.96	0%	0	0.012	0.000	0.199	

Tension Test Results


Davisson Offset Limit Lines

Tension Load Test Result for PLT-6A

Project Information

Project Name: Kensington Solar Project Location: Summitville, Ohio Project Number: N6225000

Project Number: N6225000	Design Load	Load [lbs]	Deflection ∆ (in.) Gauges #1 & #2	Data (in) (PL/AE)	Limit (in) (0.15+D/120+(PL/AE))	Comments
	0%	0	0.000	0.000	0.199	
Axial Load Test Set Up	5%	500	0.003	0.000	0.199	
Number of Gauges: 2	10%	1000	0.004	0.000	0.199	
Height of Gauges [in]: 6	15%	1500	0.006	0.000	0.199	
Load Cell: 25k Ed Jr.	20%	2000	0.008	0.000	0.199	
	25%	2500	0.011	0.000	0.199	
	30%	3000	0.014	0.000	0.199	
Test Date and Representative	35%	3500	0.023	0.000	0.199	
Tested By Terracon Rep: I. McGougan	40%	4000	0.033	0.000	0.200	
Date Tested:	45%	4500	0.044	0.000	0.200	
	50%	5000	0.069	0.000	0.200	
	55%	5500	0.104	0.001	0.200	
Pile Information	60%	6000	0.154	0.001	0.200	
Pile ID: PLT-6A	65%	6500	0.231	0.001	0.200	
Latitude: 40.68609	70%	7000	0.301	0.001	0.200	
Longitude: -80.90316	75%	7500	0.835	0.001	0.200	
Pile Type: W6x9	80%	8000		0.001	0.200	
Pile Embedment Depth [in]: 8	85%	8500		0.001	0.200	
Pile Diameter [in]: 5.9	90%	9000		0.001	0.200	
Pile Stick-Up [in]: 36	95%	9500		0.001	0.200	
Axial Design Load [lbs]: 10000	100%	10000		0.001	0.200	
Pile Area [sq. in]: 2.96	0%	0	0.818	0.000	0.199	

Tension Test Results



Davisson Offset Limit Lines

Tension Load Test Result for PLT-6B

Project Information

Project Name: Kensington Solar Project Location: Summitville, Ohio Project Number: N6225000

Project Number: N6225000	Design Load	Load [lbs]	Deflection ∆ (in.) Gauges #1 & #2	Data (in) (PL/AE)	Limit (in) (0.15+D/120+(PL/AE))	Comments
	0%	0	0.000	0.000	0.199	
Axial Load Test Set Up	5%	500	0.003	0.000	0.199	
Number of Gauges: 2	10%	1000	0.003	0.000	0.199	
Height of Gauges [in]: 6	15%	1500	0.001	0.000	0.199	
Load Cell: 25k Ed Jr.	20%	2000	0.002	0.000	0.199	
·	25%	2500	0.002	0.000	0.199	
	30%	3000	0.004	0.000	0.199	
Test Date and Representative	35%	3500	0.004	0.000	0.199	
Tested By Terracon Rep: I. McGougan	40%	4000	0.005	0.000	0.200	
Date Tested:	45%	4500	0.007	0.000	0.200	
'	50%	5000	0.008	0.000	0.200	
	55%	5500	0.009	0.001	0.200	
Pile Information	60%	6000	0.012	0.001	0.200	
Pile ID: PLT-6B	65%	6500	0.015	0.001	0.200	
Latitude: 40.68609	70%	7000	0.018	0.001	0.200	
Longitude: -80.90316	75%	7500	0.022	0.001	0.200	
Pile Type: W6x9	80%	8000	0.027	0.001	0.200	
Pile Embedment Depth [in]: 8	85%	8500	0.035	0.001	0.200	
Pile Diameter [in]: 5.9	90%	9000	0.045	0.001	0.200	
Pile Stick-Up [in]: 36	95%	9500	0.055	0.001	0.200	
Axial Design Load [lbs]: 10000	100%	10000	0.068	0.001	0.200	
Pile Area [sq. in]: 2.96	0%	0	0.069	0.000	0.199	

Tension Test Results



Davisson Offset Limit Lines

Tension Load Test Result for PLT-7A

Project Information

Project Name: Kensington Solar Project Location: Summitville, Ohio Project Number: N6225000

Project Location: Johnmitvine, Onio Project Number: N6225000	% of Design Load	Load [lbs]	Deflection ∆ (in.) Gauges #1 & #2	Data (in) (PL/AE)	Limit (in) (0.15+D/120+(PL/AE))	Comments
	0%	0	0.000	0.000	0.199	
Axial Load Test Set Up	5%	500	0.007	0.000	0.199	
Number of Gauges: 2	10%	1000	0.064	0.000	0.199	
Height of Gauges [in]: 6	15%	1500	0.782	0.000	0.199	
Load Cell: 25k Ed Jr.	20%	2000		0.000	0.199	
•	25%	2500		0.000	0.199	
	30%	3000		0.000	0.199	
Test Date and Representative	35%	3500		0.000	0.199	
Tested By Terracon Rep: I. McGougan	40%	4000		0.000	0.200	
Date Tested:	45%	4500		0.000	0.200	
•	50%	5000		0.000	0.200	
	55%	5500		0.001	0.200	
Pile Information	60%	6000		0.001	0.200	
Pile ID: PLT-7A	65%	6500		0.001	0.200	
Latitude: 40.70103	70%	7000		0.001	0.200	
Longitude: -80.89688	75%	7500		0.001	0.200	
Pile Type: W6x9	80%	8000		0.001	0.200	
Pile Embedment Depth [in]: 8	85%	8500		0.001	0.200	
Pile Diameter [in]: 5.9	90%	9000		0.001	0.200	
Pile Stick-Up [in]: 36	95%	9500		0.001	0.200	
Axial Design Load [lbs]: 10000	100%	10000		0.001	0.200	
Pile Area [sq. in]: 2.96	0%	0	0.760	0.000	0.199	

Tension Test Results



Davisson Offset Limit Lines

Tension Load Test Result for PLT-7B

Project Information

Project Name: Kensington Solar Project Location: Summitville, Ohio Project Number: N6225000

Project Number: N6225000	Design Load	Load [lbs]	Deflection ∆ (in.) Gauges #1 & #2	Data (in) (PL/AE)	Limit (in) (0.15+D/120+(PL/AE))	Comments
	0%	0	0.000	0.000	0.199	
Axial Load Test Set Up	5%	500	0.002	0.000	0.199	
Number of Gauges: 2	10%	1000	0.003	0.000	0.199	
Height of Gauges [in]: 6	15%	1500	0.004	0.000	0.199	
Load Cell: 25k Ed Jr.	20%	2000	0.006	0.000	0.199	
·	25%	2500	0.009	0.000	0.199	
	30%	3000	0.011	0.000	0.200	
Test Date and Representative	35%	3500	0.014	0.000	0.200	
Tested By Terracon Rep: I. McGougan	40%	4000	0.020	0.001	0.200	
Date Tested:	45%	4500	0.025	0.001	0.200	
· ·	50%	5000	0.030	0.001	0.200	
	55%	5500	0.040	0.001	0.200	
Pile Information	60%	6000	0.056	0.001	0.200	
Pile ID: PLT-7B	65%	6500	0.070	0.001	0.200	
Latitude: 40.70103	70%	7000	0.155	0.001	0.200	
Longitude: -80.89688	75%	7500	0.750	0.001	0.200	
Pile Type: W6x9	80%	8000		0.001	0.200	
Pile Embedment Depth [in]: 11	85%	8500		0.001	0.200	
Pile Diameter [in]: 5.9	90%	9000		0.001	0.200	
Pile Stick-Up [in]: 36	95%	9500		0.001	0.200	
Axial Design Load [lbs]: 10000	100%	10000		0.001	0.200	
Pile Area [sq. in]: 2.96	0%	0	0.720	0.000	0.199	

Tension Test Results



Davisson Offset Limit Lines

Tension Load Test Result for PLT-8A

Project Information

Project Name: Kensington Solar Project Location: Summitville, Ohio Project Number: N6225000

Project Number: N62	25000	Design Load	Load [lbs]	Deflection ∆ (in.) Gauges #1 & #2	Data (in) (PL/AE)	Limit (in) (0.15+D/120+(PL/AE))	Comments
		0%	0	0.000	0.000	0.199	
Axial Load Test Set Up		5%	500	0.002	0.000	0.199	
Number of Gauges: 2		10%	1000	0.005	0.000	0.199	
Height of Gauges [in]: 6		15%	1500	0.008	0.000	0.199	
Load Cell: 25k	Ed Jr.	20%	2000	0.012	0.000	0.199	
·		25%	2500	0.021	0.000	0.199	
		30%	3000	0.042	0.000	0.199	
Test Date and Representative		35%	3500	0.085	0.000	0.199	
Tested By Terracon Rep: I. Mo	cGougan	40%	4000	0.170	0.000	0.199	
Date Tested:		45%	4500	0.250	0.000	0.200	
•		50%	5000	0.810	0.000	0.200	
		55%	5500		0.000	0.200	
Pile Information		60%	6000		0.000	0.200	
Pile ID: PLT	-8A	65%	6500		0.001	0.200	
Latitude: 40.6	9661	70%	7000		0.001	0.200	
Longitude: -80.8	88478	75%	7500		0.001	0.200	
Pile Type: W6>	(9	80%	8000		0.001	0.200	
Pile Embedment Depth [in]: 7		85%	8500		0.001	0.200	
Pile Diameter [in]: 5.9		90%	9000		0.001	0.200	
Pile Stick-Up [in]: 36		95%	9500		0.001	0.200	
Axial Design Load [lbs]: 100	00	100%	10000		0.001	0.200	
Pile Area [sq. in]: 2.96		0%	0	0.780	0.000	0.199	

Tension Test Results



Davisson Offset Limit Lines

Tension Load Test Result for PLT-8B

Project Information

Project Name: Kensington Solar Project Location: Summitville, Ohio Project Number: N6225000

Project Number: N6225000	Design Load	Load [lbs]	Deflection ∆ (in.) Gauges #1 & #2	Data (in) (PL/AE)	Limit (in) (0.15+D/120+(PL/AE))	Comments
	0%	0	0.000	0.000	0.199	
Axial Load Test Set Up	5%	500	0.004	0.000	0.199	
Number of Gauges: 2	10%	1000	0.010	0.000	0.199	
Height of Gauges [in]: 6	15%	1500	0.015	0.000	0.199	
Load Cell: 25k Ed Jr.	20%	2000	0.018	0.000	0.199	
	25%	2500	0.026	0.000	0.199	
	30%	3000	0.031	0.000	0.199	
Test Date and Representative	35%	3500	0.036	0.000	0.200	
Tested By Terracon Rep: I. McGougan	38%	3800	0.042	0.000	0.200	
Date Tested:	45%	4500	0.048	0.000	0.200	
•	50%	5000	0.054	0.001	0.200	
	55%	5500	0.065	0.001	0.200	
Pile Information	60%	6000	0.788	0.001	0.200	
Pile ID: PLT-8B	65%	6500		0.001	0.200	
Latitude: 40.69661	70%	7000		0.001	0.200	
Longitude: -80.88478	75%	7500		0.001	0.200	
Pile Type: W6x9	80%	8000		0.001	0.200	
Pile Embedment Depth [in]: 9.1	85%	8500		0.001	0.200	
Pile Diameter [in]: 5.9	90%	9000		0.001	0.200	
Pile Stick-Up [in]: 36	95%	9500		0.001	0.200	
Axial Design Load [lbs]: 10000	100%	10000		0.001	0.200	
Pile Area [sq. in]: 2.96	0%	0	0.750	0.000	0.199	

Tension Test Results



Davisson Offset Limit Lines

Tension Load Test Result for PLT-9A

Project Information

Project Name: Kensington Solar Project Location: Summitville, Ohio Project Number: N6225000

Project Number: N622500() Des Lo	ign ad	Load [lbs]	Deflection ∆ (in.) Gauges #1 & #2	Data (in) (PL/AE)	Limit (in) (0.15+D/120+(PL/AE))	Comments
	09	%	0	0.000	0.000	0.199	
Axial Load Test Set Up	59	%	500	0.002	0.000	0.199	
Number of Gauges: 2	10	%	1000	0.004	0.000	0.199	
Height of Gauges [in]: 6	15	%	1500	0.008	0.000	0.199	
Load Cell: 25k Ed Jr.	. 20	%	2000	0.012	0.000	0.199	
	25	%	2500	0.019	0.000	0.199	
	30	%	3000	0.027	0.000	0.199	
Test Date and Representative	35	%	3500	0.039	0.000	0.199	
Tested By Terracon Rep: I. McGoug	jan 40	%	4000	0.059	0.000	0.199	
Date Tested:	45	%	4500	0.085	0.000	0.200	
	50	%	5000	0.129	0.000	0.200	
	55	%	5500	0.800	0.000	0.200	
Pile Information	59	%	5900		0.000	0.200	
Pile ID: PLT-9A	65	%	6500		0.001	0.200	
Latitude: 40.66744	70	%	7000		0.001	0.200	
Longitude: -80.89583	3 75	%	7500		0.001	0.200	
Pile Type: W6x9	80	%	8000		0.001	0.200	
Pile Embedment Depth [in]: 7	85	%	8500		0.001	0.200	
Pile Diameter [in]: 5.9	90	%	9000		0.001	0.200	
Pile Stick-Up [in]: 36	95	%	9500		0.001	0.200	
Axial Design Load [lbs]: 10000	100)%	10000		0.001	0.200	
Pile Area [sq. in]: 2.96	00	%	0	0.759	0.000	0.199	

Tension Test Results



Davisson Offset Limit Lines

Tension Load Test Result for PLT-9B

Project Information

Project Name: Kensington Solar Project Location: Summitville, Ohio Project Number: N6225000

Project Number: N6225000	Design Load	Load [lbs]	Deflection ∆ (in.) Gauges #1 & #2	Data (in) (PL/AE)	Limit (in) (0.15+D/120+(PL/AE))	Comments
	0%	0	0.000	0.000	0.199	
Axial Load Test Set Up	5%	500	0.003	0.000	0.199	
Number of Gauges: 2	10%	1000	0.004	0.000	0.199	
Height of Gauges [in]: 6	15%	1500	0.005	0.000	0.199	
Load Cell: 25k Ed Jr.	20%	2000	0.008	0.000	0.199	
·	25%	2500	0.011	0.000	0.199	
	30%	3000	0.014	0.000	0.200	
Test Date and Representative	35%	3500	0.019	0.000	0.200	
Tested By Terracon Rep: I. McGougan	40%	4000	0.027	0.000	0.200	
Date Tested:	45%	4500	0.052	0.001	0.200	
•	50%	5000	0.755	0.001	0.200	
	55%	5500		0.001	0.200	
Pile Information	60%	6000		0.001	0.200	
Pile ID: PLT-9B	65%	6500		0.001	0.200	
Latitude: 40.66744	70%	7000		0.001	0.200	
Longitude: -80.89583	75%	7500		0.001	0.200	
Pile Type: W6x9	80%	8000		0.001	0.200	
Pile Embedment Depth [in]: 10	85%	8500		0.001	0.200	
Pile Diameter [in]: 5.9	90%	9000		0.001	0.200	
Pile Stick-Up [in]: 36	95%	9500		0.001	0.200	
Axial Design Load [lbs]: 10000	100%	10000		0.001	0.200	
Pile Area [sq. in]: 2.96	0%	0	0.745	0.000	0.199	

Tension Test Results



Davisson Offset Limit Lines

Tension Load Test Result for PLT-10A

Project Information

Project Name: Kensington Solar Project Location: Summitville, Ohio Project Number: N6225000

Project Number: N6225000	Design Load	Load [lbs]	Deflection ∆ (in.) Gauges #1 & #2	Data (in) (PL/AE)	Limit (in) (0.15+D/120+(PL/AE))	Comments
	0%	0	0.000	0.000	0.199	
Axial Load Test Set Up	5%	500	0.003	0.000	0.199	
Number of Gauges: 2	10%	1000	0.006	0.000	0.199	
Height of Gauges [in]: 6	15%	1500	0.006	0.000	0.199	
Load Cell: 25k Ed Jr.	20%	2000	0.008	0.000	0.199	
·	25%	2500	0.009	0.000	0.199	
	30%	3000	0.011	0.000	0.199	
Test Date and Representative	35%	3500	0.013	0.000	0.199	
Tested By Terracon Rep: I. McGougan	40%	4000	0.014	0.000	0.200	
Date Tested:	45%	4500	0.016	0.000	0.200	
·	50%	5000	0.018	0.000	0.200	
	55%	5500	0.021	0.001	0.200	
Pile Information	60%	6000	0.022	0.001	0.200	
Pile ID: PLT-10A	65%	6500	0.024	0.001	0.200	
Latitude: 40.67042	70%	7000	0.027	0.001	0.200	
Longitude: -80.88382	75%	7500	0.028	0.001	0.200	
Pile Type: W6x9	80%	8000	0.030	0.001	0.200	
Pile Embedment Depth [in]: 8	85%	8500	0.032	0.001	0.200	
Pile Diameter [in]: 5.9	90%	9000	0.033	0.001	0.200	
Pile Stick-Up [in]: 36	95%	9500	0.035	0.001	0.200	
Axial Design Load [lbs]: 10000	100%	10000	0.040	0.001	0.200	
Pile Area [sq. in]: 2.96	0%	0	0.010	0.000	0.199	

Tension Test Results



Davisson Offset Limit Lines

Tension Load Test Result for PLT-10B

Project Information

Project Name: Kensington Solar Project Location: Summitville, Ohio Project Number: N6225000

Project Number: N6225000	Design	Load	Deflection Δ (in.)	Data (in)	Limit (in)	Comments
	Load	[Ibs]	Gauges #1 & #2	(PL/AE)	(0.15+D/120+(PL/AE))	
	0%	0	0.000	0.000	0.199	
Axial Load Test Set Up	5%	500	0.006	0.000	0.199	
Number of Gauges: 2	10%	1000	0.006	0.000	0.199	
Height of Gauges [in]: 6	15%	1500	0.009	0.000	0.199	
Load Cell: 25k Ed Jr.	20%	2000	0.013	0.000	0.199	
ľ	25%	2500	0.112	0.000	0.199	
	30%	3000	0.144	0.000	0.200	
Test Date and Representative	35%	3500	0.240	0.000	0.200	
Tested By Terracon Rep: I. McGougan	40%	4000	0.797	0.001	0.200	
Date Tested:	45%	4500		0.001	0.200	
·	50%	5000		0.001	0.200	
	55%	5500		0.001	0.200	
Pile Information	60%	6000		0.001	0.200	
Pile ID: PLT-10B	65%	6500		0.001	0.200	
Latitude: 40.67042	70%	7000		0.001	0.200	
Longitude: -80.88382	75%	7500		0.001	0.200	
Pile Type: W6x9	80%	8000		0.001	0.200	
Pile Embedment Depth [in]: 11	85%	8500		0.001	0.200	
Pile Diameter [in]: 5.9	90%	9000		0.001	0.200	
Pile Stick-Up [in]: 36	95%	9500		0.001	0.200	
Axial Design Load [lbs]: 10000	100%	10000		0.001	0.200	
Pile Area [sq. in]: 2.96	0%	0	0.784	0.000	0.199	

Tension Test Results



Davisson Offset Limit Lines

Tension Load Test Result for PLT-11A

Project Information

Project Name: Kensington Solar Project Location: Summitville, Ohio Project Number: N6225000

Project Number: N6225000	Design	Load	Deflection Δ (in.)	Data (in)	Limit (in)	Comments
	Load	[lbs]	Gauges #1 & #2	(PL/AE)	(0.15+D/120+(PL/AE))	
	0%	0	0.000	0.000	0.199	
Axial Load Test Set Up	5%	500	0.004	0.000	0.199	
Number of Gauges: 2	10%	1000	0.008	0.000	0.199	
Height of Gauges [in]: 6	15%	1500	0.011	0.000	0.199	
Load Cell: 25k Ed Jr.	20%	2000	0.015	0.000	0.199	
·	25%	2500	0.020	0.000	0.199	
	30%	3000	0.022	0.000	0.199	
Test Date and Representative	35%	3500	0.026	0.000	0.199	
Tested By Terracon Rep: I. McGougan	40%	4000	0.031	0.000	0.199	
Date Tested:	45%	4500	0.038	0.000	0.200	
·	50%	5000	0.044	0.000	0.200	
	55%	5500	0.056	0.000	0.200	
Pile Information	60%	6000	0.385	0.000	0.200	
Pile ID: PLT-11A	65%	6500	0.820	0.001	0.200	
Latitude: 40.66277	69%	6900		0.001	0.200	
Longitude: -80.90087	75%	7500		0.001	0.200	
Pile Type: W6x9	80%	8000		0.001	0.200	
Pile Embedment Depth [in]: 7	85%	8500		0.001	0.200	
Pile Diameter [in]: 5.9	90%	9000		0.001	0.200	
Pile Stick-Up [in]: 36	95%	9500		0.001	0.200	
Axial Design Load [lbs]: 10000	100%	10000		0.001	0.200	
Pile Area [sq. in]: 2.96	0%	0	0.801	0.000	0.199	

Tension Test Results



Davisson Offset Limit Lines

Tension Load Test Result for PLT-11B

Project Information

Project Name: Kensington Solar Project Location: Summitville, Ohio Project Number: N6225000

Project Number: N6225000	Design Load	Load [lbs]	Deflection ∆ (in.) Gauges #1 & #2	Data (in) (PL/AE)	Limit (in) (0.15+D/120+(PL/AE))	Comments
	0%	0	0.000	0.000	0.199	
Axial Load Test Set Up	5%	500	0.001	0.000	0.199	
Number of Gauges: 2	10%	1000	0.003	0.000	0.199	
Height of Gauges [in]: 6	15%	1500	0.003	0.000	0.199	
Load Cell: 25k Ed Jr.	20%	2000	0.005	0.000	0.199	
·	25%	2500	0.006	0.000	0.199	
	30%	3000	0.006	0.000	0.199	
Test Date and Representative	35%	3500	0.006	0.000	0.200	
Tested By Terracon Rep: I. McGougan	40%	4000	0.008	0.000	0.200	
Date Tested:	45%	4500	0.008	0.000	0.200	
·	50%	5000	0.009	0.001	0.200	
	55%	5500	0.010	0.001	0.200	
Pile Information	60%	6000	0.011	0.001	0.200	
Pile ID: PLT-11B	65%	6500	0.012	0.001	0.200	
Latitude: 40.66277	70%	7000	0.013	0.001	0.200	
Longitude: -80.90087	75%	7500	0.015	0.001	0.200	
Pile Type: W6x9	80%	8000	0.015	0.001	0.200	
Pile Embedment Depth [in]: 9.1	85%	8500	0.016	0.001	0.200	
Pile Diameter [in]: 5.9	90%	9000	0.018	0.001	0.200	
Pile Stick-Up [in]: 36	95%	9500	0.020	0.001	0.200	
Axial Design Load [lbs]: 10000	100%	10000	0.021	0.001	0.200	
Pile Area [sq. in]: 2.96	0%	0	0.010	0.000	0.199	

Tension Test Results



Davisson Offset Limit Lines

Tension Load Test Result for PLT-12A

Project Information

Project Name: Kensington Solar Project Location: Summitville, Ohio Project Number: N6225000

Project Number: N6225000	Design Load	Load [lbs]	Deflection ∆ (in.) Gauges #1 & #2	Data (in) (PL/AE)	Limit (in) (0.15+D/120+(PL/AE))	Comments
	0%	0	0.000	0.000	0.199	
Axial Load Test Set Up	5%	500	0.002	0.000	0.199	
Number of Gauges: 2	10%	1000	0.004	0.000	0.199	
Height of Gauges [in]: 6	15%	1500	0.005	0.000	0.199	
Load Cell: 25k Ed Jr.	20%	2000	0.006	0.000	0.199	
·	25%	2500	0.007	0.000	0.199	
	30%	3000	0.008	0.000	0.199	
Test Date and Representative	35%	3500	0.010	0.000	0.199	
Tested By Terracon Rep: I. McGougan	40%	4000	0.011	0.000	0.200	
Date Tested:	45%	4500	0.012	0.000	0.200	
ľ	50%	5000	0.013	0.000	0.200	
	55%	5500	0.015	0.001	0.200	
Pile Information	60%	6000	0.015	0.001	0.200	
Pile ID: PLT-12A	65%	6500	0.016	0.001	0.200	
Latitude: 40.65468	70%	7000	0.018	0.001	0.200	
Longitude: -80.90659	75%	7500	0.019	0.001	0.200	
Pile Type: W6x9	80%	8000	0.021	0.001	0.200	
Pile Embedment Depth [in]: 8	85%	8500	0.022	0.001	0.200	
Pile Diameter [in]: 5.9	90%	9000	0.023	0.001	0.200	
Pile Stick-Up [in]: 36	95%	9500	0.025	0.001	0.200	
Axial Design Load [lbs]: 10000	100%	10000	0.027	0.001	0.200	
Pile Area [sq. in]: 2.96	0%	0	0.006	0.000	0.199	

Tension Test Results



Davisson Offset Limit Lines

Tension Load Test Result for PLT-12B

Project Information

Project Name: Kensington Solar Project Location: Summitville, Ohio Project Number: N6225000

Project Number: N6225000	Design Load	Load [lbs]	Deflection ∆ (in.) Gauges #1 & #2	Data (in) (PL/AE)	Limit (in) (0.15+D/120+(PL/AE))	Comments
	0%	0	0.000	0.000	0.199	
Axial Load Test Set Up	5%	500	0.003	0.000	0.199	
Number of Gauges: 2	10%	1000	0.003	0.000	0.199	
Height of Gauges [in]: 6	15%	1500	0.003	0.000	0.199	
Load Cell: 25k Ed Jr.	20%	2000	0.004	0.000	0.199	
·	25%	2500	0.005	0.000	0.199	
	30%	3000	0.006	0.000	0.199	
Test Date and Representative	35%	3500	0.006	0.000	0.199	
Tested By Terracon Rep: I. McGougan	40%	4000	0.007	0.000	0.199	
Date Tested:	45%	4500	0.008	0.000	0.200	
·	50%	5000	0.008	0.000	0.200	
	55%	5500	0.009	0.000	0.200	
Pile Information	60%	6000	0.009	0.000	0.200	
Pile ID: PLT-12B	65%	6500	0.010	0.001	0.200	
Latitude: 40.65468	70%	7000	0.013	0.001	0.200	
Longitude: -80.90659	75%	7500	0.015	0.001	0.200	
Pile Type: W6x9	80%	8000	0.016	0.001	0.200	
Pile Embedment Depth [in]: 7.1	85%	8500	0.017	0.001	0.200	
Pile Diameter [in]: 5.9	90%	9000	0.019	0.001	0.200	
Pile Stick-Up [in]: 36	95%	9500	0.020	0.001	0.200	
Axial Design Load [lbs]: 10000	100%	10000	0.022	0.001	0.200	
Pile Area [sq. in]: 2.96	0%	0	0.007	0.000	0.199	

Tension Test Results



APPENDIX D – PILE LOAD TEST RESULTS – LATERAL LOAD

Contents:

Exhibit D-1 to D-10	Lateral Load Test Summary Graphs (10 pages)
Exhibit D-11 to D-34	Lateral Load Test Results (24 pages)

Note: All attachments are one page unless noted above.

PLT Lateral Graphs - 6 Feet Embedment Zone A





PLT Lateral Graphs - 7 Feet Embedment Zone A

PLT Lateral Graphs - 7 Feet Embedment Zone B









PLT Lateral Graphs - 8 Feet Embedment Zone B







PLT Lateral Graphs - 9 Feet Embedment Zone B







PLT Lateral Graphs - 10 Feet Embedment Zone B

9000 –
–
– P-Multiplier (Zone B)= 0.9 PLT-4B 8000 7000 6000 5000 Load (lb.) 4000 3000 2000 1000 1.1 0 0 0.25 1.75 -2.25 0.75 1.5 6 0.5 -1.25 Deflection (in)



Lateral Load Test Result for PLT-1A

Project Information	Design
Project Name: Kensington Solar	Load
Project Location: Summitville, Ohio	0%
Project Number: N6225000	7%
	14%
	21%
Lateral Load Test Set Up	0%
Number of Top Gauges: 0	21%
Number of Bottom Gauges: 2	29%
Height of Top Gauges [in]: -	36%
Height of Bottom Gauges [in]: 6	0%
Height of Applied Load [in]: 36	36%
Load Cell: 25k Ed Jr.	43%
	50%
	0%
Test Date and Representative	50%
Tested By Terracon Rep: I. McGougan	57%
Date Tested:	64%
·	71%
	0%
Pile Information	57%
Pile ID: PLT-1A	71%
Latitude: 40.70095	79%
Longitude: -80.88916	86%
Pile Type: W6x9	93%
Pile Embedment Depth [in]: 7	100%

Pile Stick-Up [in]: 36

Lateral Design Load [lbs]: 7000 Drive Time [sec]: 81

4% 1000 0.163 1% 1500 0.235)% 0.054 0 21% 1500 0.236 29% 2000 0.303 36% 2500 0.393 0% 0 0.068 6% 2500 0.403 13% 50% 3000 0.484 3500 0.595)% 0.104 0 50% 3500 0.631 57% 54% 4000 0.708 4500 0.813 '1% 5000 0.928)% 0 0.146 57% 4000 0.818 ′1% 5000 0.998 ′9% 5500 1.098 36% 6000 1.235 93% 6500 1.374 100% 7000 1.762 0% 0 0.526

Deflection Δ (in.)

Gauges #1 & #2

0.000

0.116

Comments

Lateral

Load

[lbs]

0 500

% of



nents

Lateral Load Test Result for PLT-1B

Project Information		% of Design	Lateral Load	Deflection Δ (in.)	Com
Project Name:	Kensington Solar	Load	[lbs]	Gauges #1 & #2	
Project Location:	Summitville, Ohio	0%	0	0.000	
Project Number:	N6225000	7%	500	0.060	
		14%	1000	0.139	
		21%	1500	0.221	
Lateral Load Test Set Up		0%	0	0.028	
Number of Top Gauges:	0	21%	1500	0.238	
Number of Bottom Gauges:	2	29%	2000	0.303	
Height of Top Gauges [in]:	-	36%	2500	0.346	
Height of Bottom Gauges [in]:	6	0%	0	0.044	
Height of Applied Load [in]:	36	36%	2500	0.353	
Load Cell:	25k Ed Jr.	43%	3000	0.425	
		50%	3500	0.503	
		0%	0	0.061	
Test Date and Representati	ve	50%	3500	0.537	
Tested By Terracon Rep:	I. McGougan	57%	4000	0.602	
Date Tested:		64%	4500	0.662	
		71%	5000	0.783	
		0%	0	0.088	
Pile Information		57%	4000	0.702	
Pile ID:	PLT-1B	71%	5000	0.807	
Latitude:	40.70095	79%	5500	0.891	
Longitude:	-80.88916	86%	6000	0.971	
Pile Type:	W6x9	93%	6500	1.087	
Pile Embedment Depth [in]:	8.6	100%	7000	1.199	
Pile Stick-Up [in]:	36	0%	0	0.128	
Lateral Design Load [lbs]:	7000		•		
Drive Time [sec]:	219				



llerracon

nts

Lateral Load Test Result for PLT-2A

Project Information		Design	Load	Deflection Δ (in.)	Comme
Project Name: K	ensington Solar	Load	[lbs]	Gauges #1 & #2	
Project Location: S	ummitville, Ohio	0%	0	0.000	
Project Number: N	6225000	7%	500	0.147	
		14%	1000	0.256	
		21%	1500	0.304	
Lateral Load Test Set Up		0%	0	0.047	
Number of Top Gauges: 0		21%	1500	0.355	
Number of Bottom Gauges: 2		29%	2000	0.435	
Height of Top Gauges [in]: -		36%	2500	0.519	
Height of Bottom Gauges [in]: 6		0%	0	0.088	
Height of Applied Load [in]: 3	6	36%	2500	0.560	
Load Cell: 2	5k Ed Jr.	43%	3000	0.680	
		50%	3500	0.774	
		0%	0	0.072	
Test Date and Representative	3	50%	3500	0.823	
Tested By Terracon Rep: I.	McGougan	57%	4000	0.871	
Date Tested:		61%	4300	1.007	
		71%	5000	1.131	
		0%	0	0.104	
Pile Information		57%	4000	0.998	
Pile ID: F	'LT-2A	71%	5000	1.163	
Latitude: 4	0.69342	79%	5500	1.261	
Longitude: -	30.88928	86%	6000	1.390	
Pile Type: V	V6x9	93%	6500	1.534	
Pile Embedment Depth [in]: 7	.9	100%	7000	2.029	
Pile Stick-Up [in]: 3	6	0%	0	0.610	
Lateral Design Load [lbs]: 7	000	-			
Drive Time [sec]: 9	1				

% of

Lateral



Lateral Load Test Result for PLT-2B

Project Information		Design	Load	Deflection Δ (in.)	Commen
Project Name:	Kensington Solar	Load	[lbs]	Gauges #1 & #2	
Project Location:	Summitville, Ohio	0%	0	0.000	
Project Number:	N6225000	7%	500	0.181	
		14%	1000	0.307	
		21%	1500	0.422	
Lateral Load Test Set Up		0%	0	0.146	
Number of Top Gauges:	0	21%	1500	0.487	
Number of Bottom Gauges:	2	29%	2000	0.585	
Height of Top Gauges [in]:	-	36%	2500	0.766	
Height of Bottom Gauges [in]:	6	0%	0	0.230	
Height of Applied Load [in]:	36	36%	2500	0.824	
Load Cell:	25k Ed Jr.	43%	3000	0.957	
	-	50%	3500	1.165	
		0%	0	0.336	
Test Date and Representati	ive	50%	3500	1.255	
Tested By Terracon Rep:	I. McGougan	57%	4000	1.415	
Date Tested:		64%	4500	1.659	
		71%	5000	2.000	
		0%	0		
Pile Information		57%	4000		
Pile ID:	PLT-2B	71%	5000		
Latitude:	40.69342	79%	5500		
Longitude:	-80.88928	86%	6000		
Pile Type:	W6x9	93%	6500		
Pile Embedment Depth [in]:	8.6	100%	7000		
Pile Stick-Up [in]:	36	0%	0	0.687	
Lateral Design Load [lbs]:	7000	. <u></u>			
Drive Time [sec]:	107				

Lateral



nts

Lateral Load Test Result for PLT-3A

Project Information		Design	Load	Deflection Δ (in.)	Comme
Project Name:	Kensington Solar	Load	[lbs]	Gauges #1 & #2	
Project Location:	Summitville, Ohio	0%	0	0.000	
Project Number:	N6225000	7%	500	0.124	
-	•	14%	1000	0.258	
		21%	1500	0.317	
Lateral Load Test Set Up		0%	0	0.099	
Number of Top Gauges:	0	21%	1500	0.418	
Number of Bottom Gauges:	2	29%	2000	0.471	
Height of Top Gauges [in]:	-	36%	2500	0.581	
Height of Bottom Gauges [in]:	6	0%	0	0.150	
Height of Applied Load [in]:	36	36%	2500	0.590	
Load Cell:	25k Ed Jr.	39%	2700	0.745	
		50%	3500	0.854	
		0%	0	0.200	
Test Date and Representati	ve	50%	3500	0.915	
Tested By Terracon Rep:	I. McGougan	57%	4000	1.037	
Date Tested:		64%	4500	1.147	
		71%	5000	1.307	
		0%	0	0.290	
Pile Information		57%	4000	1.189	
Pile ID:	PLT-3A	71%	5000	1.390	
Latitude:	40.69014	79%	5500	1.724	
Longitude:	-80.88951	86%	6000	1.652	
Pile Type:	W6x9	93%	6500	1.824	
Pile Embedment Depth [in]:	7	100%	7000	2.004	
Pile Stick-Up [in]:	36	0%	0	0.517	
Lateral Design Load [lbs]:	7000		•		
Drive Time [sec]:	68				

% of

Lateral



mments

Lateral Load Test Result for PLT-3B

Project Information		% of Design	Lateral Load	Deflection Δ (in.)	Co
Project Name:	Kensington Solar	Load	[lbs]	Gauges #1 & #2	
Project Location:	Summitville, Ohio	0%	0	0.000	
Project Number:	N6225000	7%	500	0.203	
		14%	1000	0.342	
		21%	1500	0.424	
Lateral Load Test Set Up		0%	0	0.101	
Number of Top Gauges:	0	21%	1500	0.437	
Number of Bottom Gauges:	2	29%	2000	0.527	
Height of Top Gauges [in]:	-	36%	2500	0.607	
Height of Bottom Gauges [in]:	6	0%	0	0.116	
Height of Applied Load [in]:	36	36%	2500	0.651	
Load Cell:	25k Ed Jr.	43%	3000	0.731	
		50%	3500	0.858	
		0%	0	0.129	
Test Date and Representati	ve	50%	3500	0.907	
Tested By Terracon Rep:	I. McGougan	57%	4000	0.981	
Date Tested:		64%	4500	1.162	
		71%	5000	1.253	
		0%	0	0.173	
Pile Information		57%	4000	1.157	
Pile ID:	PLT-3B	71%	5000	1.319	
Latitude:	40.69014	79%	5500	1.410	
Longitude:	-80.88951	86%	6000	1.529	
Pile Type:	W6x9	93%	6500	1.666	
Pile Embedment Depth [in]:	10	100%	7000	1.840	
Pile Stick-Up [in]:	36	0%	0	0.212	
Lateral Design Load [lbs]:	7000				
Drive Time [sec]:	175				



nts

Lateral Load Test Result for PLT-4A

Project Information		Design	Load	Deflection Δ (in.)	Comme
Project Name:	Kensington Solar	Load	[lbs]	Gauges #1 & #2	
Project Location:	Summitville, Ohio	0%	0	0.000	
Project Number:	N6225000	7%	500	0.135	
-	•	14%	1000	0.216	
		21%	1500	0.235	
Lateral Load Test Set Up		0%	0	0.086	
Number of Top Gauges:	0	21%	1500	0.305	
Number of Bottom Gauges:	2	29%	2000	0.392	
Height of Top Gauges [in]:	-	31%	2200	0.465	
Height of Bottom Gauges [in]:	6	0%	0	0.150	
Height of Applied Load [in]:	36	36%	2500	0.494	
Load Cell:	25k Ed Jr.	43%	3000	0.541	
		50%	3500	0.637	
		0%	0	0.165	
Test Date and Representati	ve	50%	3500	0.721	
Tested By Terracon Rep:	I. McGougan	57%	4000	0.833	
Date Tested:		64%	4500	0.931	
		71%	5000	1.069	
		0%	0	0.216	
Pile Information		57%	4000	0.972	
Pile ID:	PLT-4A	71%	5000	1.115	
Latitude:	40.68967	79%	5500	1.214	
Longitude:	-80.89409	86%	6000	1.340	
Pile Type:	W6x9	93%	6500	1.463	
Pile Embedment Depth [in]:	7.1	100%	7000	1.538	
Pile Stick-Up [in]:	36	0%	0	0.285	
Lateral Design Load [lbs]:	7000				
Drive Time [sec]:	91				

Lateral



ents

Lateral Load Test Result for PLT-4B

Project Information		Design	Load	Deflection Δ (in.)	Comn
Project Name:	Kensington Solar	Load	[lbs]	Gauges #1 & #2	
Project Location:	Summitville, Ohio	0%	0	0.000	
Project Number:	N6225000	7%	500	0.257	
	•	14%	1000	0.727	
		21%	1500	0.917	
Lateral Load Test Set Up		0%	0	0.169	
Number of Top Gauges:	0	21%	1500	0.917	
Number of Bottom Gauges:	2	29%	2000	1.047	
Height of Top Gauges [in]:	-	36%	2500	1.144	
Height of Bottom Gauges [in]:	6	0%	0	0.150	
Height of Applied Load [in]:	36	36%	2500	1.178	
Load Cell:	25k Ed Jr.	43%	3000	1.291	
		50%	3500	1.425	
		0%	0	0.202	
Test Date and Representati	ve	50%	3500	1.500	
Tested By Terracon Rep:	I. McGougan	57%	4000	1.610	
Date Tested:		64%	4500	1.745	
		71%	5000	2.000	
		0%	0		
Pile Information		57%	4000		
Pile ID:	PLT-4B	71%	5000		
Latitude:	40.68967	79%	5500		
Longitude:	-80.89409	86%	6000		
Pile Type:	W6x9	93%	6500		
Pile Embedment Depth [in]:	11	100%	7000		
Pile Stick-Up [in]:	36	0%	0	0.318	
Lateral Design Load [lbs]:	7000				
Drive Time [sec]:	38				

Lateral



its

Lateral Load Test Result for PLT-5A

Project Information		Design	Load	Deflection Δ (in.)	Comme
Project Name: k	Censington Solar	Load	[lbs]	Gauges #1 & #2	
Project Location: S	Summitville, Ohio	0%	0	0.000	
Project Number: N	16225000	7%	500	0.099	
-		14%	1000	0.166	
		21%	1500	0.255	
Lateral Load Test Set Up		0%	0	0.050	
Number of Top Gauges: 0)	21%	1500	0.279	
Number of Bottom Gauges: 2		26%	1800	0.298	
Height of Top Gauges [in]: -		36%	2500	0.377	
Height of Bottom Gauges [in]: 6	j	0%	0	0.030	
Height of Applied Load [in]: 3	6	36%	2500	0.405	
Load Cell: 2	25k Ed Jr.	43%	3000	0.456	
		50%	3500	0.523	
		0%	0	0.074	
Test Date and Representative	е	50%	3500	0.543	
Tested By Terracon Rep: I.	. McGougan	57%	4000	0.632	
Date Tested:		64%	4500	0.698	
		71%	5000	0.825	
		0%	0	0.105	
Pile Information		57%	4000	0.466	
Pile ID: F	'LT-5A	71%	5000	0.827	
Latitude: 4	0.67996	79%	5500	0.885	
Longitude:	80.89076	86%	6000	1.023	
Pile Type: V	V6x9	93%	6500	1.150	
Pile Embedment Depth [in]: 5	i.6	100%	7000	1.405	
Pile Stick-Up [in]: 3	6	0%	0	0.403	
Lateral Design Load [lbs]: 7 Drive Time [sec]: 1	'000 77				

% of

Lateral



nments

Lateral Load Test Result for PLT-5B

Project Information		% of Design	Lateral Load	Deflection Δ (in.)	Con
Project Name:	Kensington Solar	Load	[lbs]	Gauges #1 & #2	
Project Location:	Summitville, Ohio	0%	0	0.000	
Project Number:	N6225000	7%	500	0.130	
		14%	1000	0.210	
		21%	1500	0.283	
Lateral Load Test Set Up		0%	0	0.067	
Number of Top Gauges:	0	21%	1500	0.291	
Number of Bottom Gauges:	2	29%	2000	0.346	
Height of Top Gauges [in]:	-	36%	2500	0.423	
Height of Bottom Gauges [in]:	6	0%	0	0.075	
Height of Applied Load [in]:	36	36%	2500	0.437	
Load Cell:	25k Ed Jr.	43%	3000	0.527	
	•	50%	3500	0.603	
		0%	0	0.103	
Test Date and Representative		50%	3500	0.629	
Tested By Terracon Rep:	I. McGougan	57%	4000	0.703	
Date Tested:		64%	4500	0.803	
	•	71%	5000	0.908	
		0%	0	0.125	
Pile Information		57%	4000	0.786	
Pile ID:	PLT-5B	71%	5000	0.929	
Latitude:	40.67996	79%	5500	1.012	
Longitude:	-80.89076	86%	6000	1.117	
Pile Type:	W6x9	93%	6500	1.222	
Pile Embedment Depth [in]:	5.6	100%	7000	1.402	
Pile Stick-Up [in]:	36	0%	0	0.271	
Lateral Design Load [lbs]:	7000		•		
Drive Time [sec]:	170				



nts

Lateral Load Test Result for PLT-6A

Project Information		Design	Load	Deflection Δ (in.)	Comme
Project Name:	Kensington Solar	Load	[lbs]	Gauges #1 & #2	
Project Location:	Summitville, Ohio	0%	0	0.000	
Project Number:	N6225000	7%	500	0.107	
-	•	14%	1000	0.218	
		21%	1500	0.291	
Lateral Load Test Set Up		0%	0	0.075	
Number of Top Gauges:	0	21%	1500	0.310	
Number of Bottom Gauges:	2	29%	2000	0.405	
Height of Top Gauges [in]:	-	36%	2500	0.510	
Height of Bottom Gauges [in]:	6	0%	0	0.141	
Height of Applied Load [in]:	36	36%	2500	0.546	
Load Cell:	25k Ed Jr.	43%	3000	0.620	
		50%	3500	0.745	
		0%	0	0.151	
Test Date and Representative		50%	3500	0.765	
Tested By Terracon Rep: I. McGougan		57%	4000	0.881	
Date Tested:		64%	4500	0.994	
	•	71%	5000	1.149	
		0%	0	-0.004	
Pile Information		57%	4000	1.016	
Pile ID:	PLT-6A	71%	5000	1.233	
Latitude:	40.68609	79%	5500	1.337	
Longitude:	-80.90316	86%	6000	1.455	
Pile Type:	W6x9	93%	6500	1.617	
Pile Embedment Depth [in]:	8	100%	7000	1.796	
Pile Stick-Up [in]:	36	0%	0	0.418	
Lateral Design Load [lbs]:	7000				
Drive Time [sec]:	97				

Lateral

of


nments

Lateral Load Test Result for PLT-6B

Project Information		% of Design	Lateral Load	Deflection Δ (in.)	Con
Project Name:	Kensington Solar	Load	[lbs]	Gauges #1 & #2	
Project Location:	Summitville, Ohio	0%	0	0.000	
Project Number:	N6225000	7%	500	0.117	
		14%	1000	0.199	
		21%	1500	0.279	
Lateral Load Test Set Up		0%	0	0.076	
Number of Top Gauges:	0	21%	1500	0.288	
Number of Bottom Gauges:	2	29%	2000	0.384	
Height of Top Gauges [in]:	-	36%	2500	0.491	
Height of Bottom Gauges [in]:	6	0%	0	0.099	
Height of Applied Load [in]:	36	36%	2500	0.556	
Load Cell:	25k Ed Jr.	43%	3000	0.594	
		50%	3500	0.704	
		0%	0	0.128	
Test Date and Representati	ve	50%	3500	0.750	
Tested By Terracon Rep:	I. McGougan	57%	4000	0.845	
Date Tested:		64%	4500	0.953	
		71%	5000	1.083	
		0%	0	0.172	
Pile Information		57%	4000	1.009	
Pile ID:	PLT-6B	71%	5000	1.181	
Latitude:	40.68609	79%	5500	1.251	
Longitude:	-80.90316	86%	6000	1.379	
Pile Type:	W6x9	93%	6500	1.565	
Pile Embedment Depth [in]:	8	100%	7000	2.000	
Pile Stick-Up [in]:	36	0%	0	0.677	
Lateral Design Load [lbs]:	7000				
Drive Time [sec]:	120				



Lateral Load Test Result for PLT-7A

Project Information		Design	Load	Deflection Δ (in.)	Comments
Project Name:	Kensington Solar	Load	[lbs]	Gauges #1 & #2	
Project Location:	Summitville, Ohio	0%	0	0.000	
Project Number:	N6225000	7%	500	0.244	
-	•	14%	1000	0.401	
		21%	1500	0.588	
Lateral Load Test Set Up		0%	0	0.220	
Number of Top Gauges:	0	21%	1500	0.587	
Number of Bottom Gauges:	2	29%	2000	0.782	
Height of Top Gauges [in]:	-	36%	2500	0.981	
Height of Bottom Gauges [in]:	6	0%	0	0.341	
Height of Applied Load [in]:	36	36%	2500	1.054	
Load Cell:	25k Ed Jr.	43%	3000	1.214	
		50%	3500	1.384	
		0%	0	0.479	
Test Date and Representati	ve	50%	3500	1.487	
Tested By Terracon Rep:	I. McGougan	57%	4000	1.635	
Date Tested:		64%	4500	1.789	
		71%	5000	2.034	
		0%	0		
Pile Information		57%	4000		
Pile ID:	PLT-7A	71%	5000		
Latitude:	40.70103	79%	5500		
Longitude:	-80.89688	86%	6000		
Pile Type:	W6x9	93%	6500		
Pile Embedment Depth [in]:	8	100%	7000		
Pile Stick-Up [in]:	36	0%	0	0.746	
Lateral Design Load [lbs]:	7000				
Drive Time [sec]:	6				

% of

Lateral



nts

Lateral Load Test Result for PLT-7B

Project Name: Kensington Solar Project Location: Summitville, Ohio Project Number: N6225000 Image: Additional systems New Systems Image: Additional systems New Systems Number of Top Gauges: 0 Number of Bottom Gauges: 2 Height of Top Gauges [in]: - Height of Applied Load [in]: 36 Load Cell: 25k Ed Jr. Test Date and Representative 50% Tested By Terracon Rep: 1. McGougan Date Tested: 57% Pile Information 57% Pile Information 57%		Comm
Project Location: Summitville, Ohio Project Number: N6225000 Itateral Load Test Set Up 7% Number of Top Gauges: 0 Number of Bottom Gauges: 2 Height of Top Gauges [in]: - Height of Bottom Gauges [in]: - Height of Applied Load [in]: 36 Load Cell: 25k Ed Jr. Test Date and Representative 50% 3500 Tested By Terracon Rep: I. McGougan Date Tested: 57% 4000 Pile Information 57% 4000	Solar Load [lbs] Gauges #1 & #2	
Project Number: N6225000 Image: Project Number: N622500 Image: Project Number: N622500 Number of Top Gauges: 0 Number of Bottom Gauges: 2 Height of Top Gauges [in]: - Height of Bottom Gauges [in]: 6 Height of Applied Load [in]: 36 Load Cell: 25k Ed Jr. Test Date and Representative 50% Tested By Terracon Rep: I. McGougan Date Tested: 57% Pile Information 57% Pile Information 57%	, Ohio 0% 0 0.000	
Lateral Load Test Set Up 14% 1000 0.294 Number of Top Gauges: 0 0.474 Number of Bottom Gauges: 2 Height of Top Gauges [in]: - Height of Bottom Gauges [in]: - Height of Bottom Gauges [in]: - Height of Applied Load [in]: 36 Load Cell: 25k Ed Jr. Test Date and Representative 50% Tested By Terracon Rep: I. McGougan Date Tested: 57% Pile Information 0% O% 0 0% 0 0% 0 0% 0 0% 0 0% 0 0% 0 0% 0 0% 0 1.104 50% 57% 4000 1.552 64% 64% 4500 0% 0 0% 0 0% 0 0% 0 0% 0 0% 0 <tr< td=""><td>7% 500 0.127</td><td></td></tr<>	7% 500 0.127	
Lateral Load Test Set Up 21% 1500 0.474 Number of Top Gauges: 0 0% 0 0.062 Number of Bottom Gauges: 2 21% 1500 0.523 Height of Top Gauges [in]: - 36% 2500 0.830 Height of Bottom Gauges [in]: - 36% 2500 0.905 Load Cell: 25k Ed Jr. 36% 2500 0.905 Load Cell: 25k Ed Jr. 43% 3000 1.104 50% 3500 1.278 0% 0 0.146 Test Date and Representative 50% 3500 1.392 57% 4000 1.552 G4% 4500 1.747 71% 5000 2.000 Date Tested: 0% 0 0% 0 0% 0	14% 1000 0.294	
Lateral Load Test Set Up 0% 0 0.062 Number of Top Gauges: 0 21% 1500 0.523 Number of Bottom Gauges: 2 29% 2000 0.676 Height of Top Gauges [in]: - 36% 2500 0.830 Height of Applied Load [in]: 36 36% 2500 0.905 Load Cell: 25k Ed Jr. 43% 3000 1.104 50% 3500 1.278 0% 0 0.146 Test Date and Representative 50% 3500 1.392 57% 4000 1.552 64% 4500 1.747 71% 5000 2.000 Pile Information 57% 4000 57% 4000 0	21% 1500 0.474	
Number of Top Gauges: 0 Number of Bottom Gauges: 2 Height of Top Gauges [in]: - - Height of Bottom Gauges [in]: 6 36% Height of Applied Load [in]: 36 36% Load Cell: 25k Ed Jr. 36% Test Date and Representative 50% Tested By Terracon Rep: 1. McGougan Date Tested: 57% Pile Information 57% Pile Information 57%	0% 0 0.062	
Number of Bottom Gauges: 2 Height of Top Gauges [in]: - 36% Height of Bottom Gauges [in]: 6 0% Height of Applied Load [in]: 36 36% Load Cell: 25k Ed Jr. Test Date and Representative 50% Tested By Terracon Rep: I. McGougan Date Tested: 57% Pile Information 57% Pile Information 57%	21% 1500 0.523	
Height of Top Gauges [in]: - 36% 2500 0.830 Height of Bottom Gauges [in]: 6 0% 0 0.112 Height of Applied Load [in]: 36 36% 2500 0.905 Load Cell: 25k Ed Jr. 43% 3000 1.104 50% 3500 1.278 0% 0 0.146 Test Date and Representative 50% 3500 1.392 57% 4000 1.552 Date Tested: 50% 0 0 0 0 0 Pile Information 57% 4000 1.747 71% 5000 2.000 0	29% 2000 0.676	
Height of Bottom Gauges [in]: 6 0% 0 0.112 Height of Applied Load [in]: 36 36% 2500 0.905 Load Cell: 25k Ed Jr. 43% 3000 1.104 Test Date and Representative 50% 3500 1.278 Tested By Terracon Rep: I. McGougan 57% 4000 1.552 64% 4500 1.747 71% 5000 2.000 Pile Information 57% 4000 0 0	36% 2500 0.830	
Height of Applied Load [in]: 36 Load Cell: 25k Ed Jr. Colspan="2">Colspan="2">Colspan="2">Colspan="2">Colspan="2">Colspan="2">Colspan="2">Colspan="2"Co	0% 0 0.112	
Load Cell: 25k Ed Jr. 43% 3000 1.104 50% 3500 1.278 0% 0 0.146 Test Date and Representative 50% 3500 1.392 57% 4000 1.552 Date Tested: 50% 5000 1.747 71% 5000 2.000 Pile Information 57% 4000 0 0% 0 0%	36% 2500 0.905	
Test Date and Representative 50% 3500 1.278 Tested By Terracon Rep: I. McGougan 0% 0 0.146 Date Tested: 57% 4000 1.552 64% 4500 1.747 71% 5000 2.000 0% 0 0	43% 3000 1.104	
Test Date and Representative 0% 0 0.146 Tested By Terracon Rep: I. McGougan 57% 4000 1.552 Date Tested: 64% 4500 1.747 71% 5000 2.000 0% 0% 0 0 0% 0	50% 3500 1.278	
Test Date and Representative 50% 3500 1.392 Tested By Terracon Rep: I. McGougan 57% 4000 1.552 Date Tested: 64% 4500 1.747 71% 5000 2.000 0 0% 0 0 0	0% 0 0.146	
Tested By Terracon Rep: I. McGougan Date Tested: 57% 4000 1.552 64% 4500 1.747 71% 5000 2.000 0% 0 57%	50% 3500 1.392	
Date Tested: 64% 4500 1.747 71% 5000 2.000 0% 0 Pile Information 57% 4000 0	n 57% 4000 1.552	
71% 5000 2.000 0% 0	64% 4500 1.747	
0% 0 Pile Information 57% 4000	71% 5000 2.000	
Pile Information 57% 4000	0% 0	
	57% 4000	
Pile ID: PLT-7B 71% 5000	71% 5000	
Latitude: 40.70103 79% 5500	79% 5500	
Longitude: -80.89688 86% 6000	86% 6000	
Pile Type: W6x9 93% 6500	93% 6500	
Pile Embedment Depth [in]: 11 100% 7000	100% 7000	
Pile Stick-Up [in]: 36 0% 0 0.323	0% 0 0.323	
Lateral Design Load [lbs]: 7000		

Lateral



its

Lateral Load Test Result for PLT-8A

Project Information		Design	Load	Deflection Δ (in.)	Comme
Project Name:	Kensington Solar	Load	[lbs]	Gauges #1 & #2	
Project Location:	Summitville, Ohio	0%	0	0.000	
Project Number:	N6225000	7%	500	0.133	
	•	14%	1000	0.315	
		21%	1500	0.435	
Lateral Load Test Set Up		0%	0	0.074	
Number of Top Gauges:	0	21%	1500	0.437	
Number of Bottom Gauges:	2	29%	2000	0.535	
Height of Top Gauges [in]:	-	36%	2500	0.654	
Height of Bottom Gauges [in]:	6	0%	0	0.075	
Height of Applied Load [in]:	36	36%	2500	0.655	
Load Cell:	25k Ed Jr.	43%	3000	0.825	
	-	50%	3500	0.925	
		0%	0	0.168	
Test Date and Representati	ve	50%	3500	0.815	
Tested By Terracon Rep:	I. McGougan	57%	4000	1.145	
Date Tested:		64%	4500	1.360	
	-	71%	5000	1.516	
		0%	0	0.290	
Pile Information		57%	4000	1.360	
Pile ID:	PLT-8A	71%	5000	1.611	
Latitude:	40.69661	79%	5500	1.524	
Longitude:	-80.88478	86%	6000	2.018	
Pile Type:	W6x9	93%	6500		
Pile Embedment Depth [in]:	7	100%	7000		
Pile Stick-Up [in]:	36	0%	0	0.487	
Lateral Design Load [lbs]:	7000	-			
Drive Time [sec]:	12				

Lateral



mments

Lateral Load Test Result for PLT-8B

Project Information		% of Design	Lateral Load	Deflection Δ (in.)	Co
Project Name:	Kensington Solar	Load	[lbs]	Gauges #1 & #2	
Project Location:	Summitville, Ohio	0%	0	0.000	
Project Number:	N6225000	7%	500	0.229	
		14%	1000	0.356	
		21%	1500	0.505	
Lateral Load Test Set Up		0%	0	0.201	
Number of Top Gauges:	0	21%	1500	0.537	
Number of Bottom Gauges:	2	29%	2000	0.651	
Height of Top Gauges [in]:	-	36%	2500	0.836	
Height of Bottom Gauges [in]:	6	0%	0	0.284	
Height of Applied Load [in]:	36	36%	2500	0.863	
Load Cell:	25k Ed Jr.	43%	3000	1.015	
		50%	3500	1.186	
		0%	0	0.352	
Test Date and Representati	ve	50%	3500	1.234	
Tested By Terracon Rep:	I. McGougan	57%	4000	1.358	
Date Tested:		64%	4500	1.515	
		70%	4900	1.712	
		0%	0	0.425	
Pile Information		57%	4000	1.558	
Pile ID:	PLT-8B	71%	5000	1.757	
Latitude:	40.69661	79%	5500	1.907	
Longitude:	-80.88478	86%	6000	2.000	
Pile Type:	W6x9	93%	6500		
Pile Embedment Depth [in]:	9.1	100%	7000		
Pile Stick-Up [in]:	36	0%	0	0.464	
Lateral Design Load [lbs]:	7000				
Drive Time [sec]:	104				



Lateral Load Test Result for PLT-9A

Project Information		Design	Load	Deflection Δ (in.)	Comment
Project Name:	Kensington Solar	Load	[lbs]	Gauges #1 & #2	
Project Location:	Summitville, Ohio	0%	0	0.000	
Project Number:	N6225000	7%	500	0.209	
-	•	14%	1000	0.338	
		21%	1500	0.495	
Lateral Load Test Set Up		0%	0	0.162	
Number of Top Gauges:	0	21%	1500	0.525	
Number of Bottom Gauges:	2	29%	2000	0.736	
Height of Top Gauges [in]:	-	36%	2500	2.521	
Height of Bottom Gauges [in]:	6	0%	0	0.460	
Height of Applied Load [in]:	36	36%	2500	1.166	
Load Cell:	25k Ed Jr.	43%	3000	2.524	
	-	50%	3500		
		0%	0		
Test Date and Representati	ve	50%	3500		
Tested By Terracon Rep:	I. McGougan	57%	4000		
Date Tested:		64%	4500		
		71%	5000		
		0%	0		
Pile Information		57%	4000		
Pile ID:	PLT-9A	71%	5000		
Latitude:	40.66744	79%	5500		
Longitude:	-80.89583	86%	6000		
Pile Type:	W6x9	93%	6500		
Pile Embedment Depth [in]:	7	100%	7000		
Pile Stick-Up [in]:	36	0%	0	1.527	
Lateral Design Load [lbs]:	7000				
Drive Time [sec]:	22				



ents

Lateral Load Test Result for PLT-9B

Project Information		Design	Load	Deflection Δ (in.)	Comm
Project Name:	Kensington Solar	Load	[lbs]	Gauges #1 & #2	
Project Location:	Summitville, Ohio	0%	0	0.000	
Project Number:	N6225000	7%	500	0.239	
	•	14%	1000	0.417	
		21%	1500	0.584	
Lateral Load Test Set Up		0%	0	0.203	
Number of Top Gauges:	0	21%	1500	0.757	
Number of Bottom Gauges:	2	29%	2000	0.834	
Height of Top Gauges [in]:	-	36%	2500	1.016	
Height of Bottom Gauges [in]:	6	0%	0	0.281	
Height of Applied Load [in]:	36	36%	2500	1.047	
Load Cell:	25k Ed Jr.	43%	3000	1.246	
		50%	3500	1.380	
		0%	0	0.343	
Test Date and Representati	ve	50%	3500	1.550	
Tested By Terracon Rep:	I. McGougan	57%	4000	1.774	
Date Tested:		64%	4500	2.000	
		71%	5000		
		0%	0		
Pile Information		57%	4000		
Pile ID:	PLT-9B	71%	5000		
Latitude:	40.66744	79%	5500		
Longitude:	-80.89583	86%	6000		
Pile Type:	W6x9	93%	6500		
Pile Embedment Depth [in]:	10	100%	7000		
Pile Stick-Up [in]:	36	0%	0	0.433	
Lateral Design Load [lbs]:	7000				
Drive Time [sec]:	19				

Lateral



ents

Lateral Load Test Result for PLT-10A

Project Information		Design	Load	Deflection Δ (in.)	Comn
Project Name:	Kensington Solar	Load	[lbs]	Gauges #1 & #2	
Project Location:	Summitville, Ohio	0%	0	0.000	
Project Number:	N6225000	7%	500	0.158	
	•	14%	1000	0.294	
		21%	1500	0.371	
Lateral Load Test Set Up		0%	0	0.100	
Number of Top Gauges:	0	21%	1500	0.416	
Number of Bottom Gauges:	2	29%	2000	0.520	
Height of Top Gauges [in]:	-	36%	2500	0.561	
Height of Bottom Gauges [in]:	6	0%	0	0.113	
Height of Applied Load [in]:	36	36%	2500	0.584	
Load Cell:	25k Ed Jr.	43%	3000	0.675	
		50%	3500	0.759	
		0%	0	0.140	
Test Date and Representati	ve	50%	3500	0.825	
Tested By Terracon Rep:	I. McGougan	57%	4000	0.856	
Date Tested:		64%	4500	0.967	
		71%	5000	1.055	
		0%	0	0.152	
Pile Information		57%	4000	0.981	
Pile ID:	PLT-10A	71%	5000	1.084	
Latitude:	40.67042	79%	5500	1.158	
Longitude:	-80.88382	86%	6000	1.246	
Pile Type:	W6x9	93%	6500	1.393	
Pile Embedment Depth [in]:	8	100%	7000	1.590	
Pile Stick-Up [in]:	36	0%	0	0.324	
Lateral Design Load [lbs]:	7000	-			
Drive Time [sec]:	112				

Lateral



Lateral Load Test Result for PLT-10B

Project Information		Design	Load	Deflection Δ (in.)	Comment
Project Name:	Kensington Solar	Load	[lbs]	Gauges #1 & #2	
Project Location:	Summitville, Ohio	0%	0	0.000	
Project Number:	N6225000	7%	500	0.186	
	•	14%	1000	0.420	
		21%	1500	0.560	
Lateral Load Test Set Up		0%	0	0.083	
Number of Top Gauges:	0	21%	1500	0.577	
Number of Bottom Gauges:	2	29%	2000	0.693	
Height of Top Gauges [in]:	-	36%	2500	0.850	
Height of Bottom Gauges [in]:	6	0%	0	0.099	
Height of Applied Load [in]:	36	36%	2500	0.868	
Load Cell:	25k Ed Jr.	43%	3000	0.993	
	•	50%	3500	1.190	
		0%	0	0.118	
Test Date and Representati	ive	50%	3500	1.198	
Tested By Terracon Rep:	I. McGougan	57%	4000	1.289	
Date Tested:		61%	4300	1.464	
	•	71%	5000	1.531	
		0%	0	0.130	
Pile Information		57%	4000	1.368	
Pile ID:	PLT-10B	71%	5000	1.617	
Latitude:	40.67042	79%	5500	1.695	
Longitude:	-80.88382	86%	6000	2.000	
Pile Type:	W6x9	93%	6500		
Pile Embedment Depth [in]:	11	100%	7000		
Pile Stick-Up [in]:	36	0%	0	0.314	
Lateral Design Load [lbs]:	7000	-	•	· ·	
Drive Time [sec]:	24				



Lateral Load Test Result for PLT-11A

Project Information		Design	Load	Deflection Δ (in.)	Commen
Project Name:	Kensington Solar	Load	[lbs]	Gauges #1 & #2	
Project Location:	Summitville, Ohio	0%	0	0.000	
Project Number:	N6225000	7%	500	0.145	
-		14%	1000	0.233	
		21%	1500	0.327	
Lateral Load Test Set Up		0%	0	0.097	
Number of Top Gauges:	0	21%	1500	0.375	
Number of Bottom Gauges:	2	29%	2000	0.423	
Height of Top Gauges [in]:	-	36%	2500	0.440	
Height of Bottom Gauges [in]:	6	0%	0	0.109	
Height of Applied Load [in]:	36	36%	2500	0.446	
Load Cell:	25k Ed Jr.	43%	3000	0.520	
		50%	3500	0.560	
		0%	0	0.125	
Test Date and Representati	ve	50%	3500	0.620	
Tested By Terracon Rep:	I. McGougan	57%	4000	0.655	
Date Tested:		64%	4500	0.716	
		71%	5000	0.801	
		0%	0	0.140	
Pile Information		57%	4000	0.680	
Pile ID:	PLT-11A	71%	5000	0.820	
Latitude:	40.66277	79%	5500	0.865	
Longitude:	-80.90087	86%	6000	1.160	
Pile Type:	W6x9	93%	6500	2.025	
Pile Embedment Depth [in]:	7	100%	7000		
Pile Stick-Up [in]:	36	0%	0	1.148	
Lateral Design Load [lbs]:	7000				
Drive Time [sec]:	49				

Lateral



Lateral Load Test Result for PLT-11B

Project Information		Design	Load	Deflection Δ (in.)	Comment
Project Name:	Kensington Solar	Load	[lbs]	Gauges #1 & #2	
Project Location:	Summitville, Ohio	0%	0	0.000	
Project Number:	N6225000	7%	500	0.137	
	•	14%	1000	0.266	
		21%	1500	0.356	
Lateral Load Test Set Up		0%	0	0.113	
Number of Top Gauges:	0	21%	1500	0.403	
Number of Bottom Gauges:	2	29%	2000	0.439	
Height of Top Gauges [in]:	-	36%	2500	0.530	
Height of Bottom Gauges [in]:	6	0%	0	0.147	
Height of Applied Load [in]:	36	36%	2500	0.568	
Load Cell:	25k Ed Jr.	43%	3000	0.632	
		50%	3500	0.706	
		0%	0	0.157	
Test Date and Representati	ive	50%	3500	0.712	
Tested By Terracon Rep:	I. McGougan	57%	4000	0.774	
Date Tested:		64%	4500	0.848	
		69%	4800	0.927	
		0%	0		
Pile Information		57%	4000		
Pile ID:	PLT-11B	71%	5000		
Latitude:	40.66277	79%	5500		
Longitude:	-80.90087	86%	6000		
Pile Type:	W6x9	93%	6500		
Pile Embedment Depth [in]:	9.1	100%	7000		
Pile Stick-Up [in]:	36	0%	0	1.269	
Lateral Design Load [lbs]:	7000				
Drive Time [sec]:	98				



ents

Lateral Load Test Result for PLT-12A

Project Information		Design	Load	Deflection Δ (in.)	Comm
Project Name:	Kensington Solar	Load	[lbs]	Gauges #1 & #2	
Project Location:	Summitville, Ohio	0%	0	0.000	-
Project Number:	N6225000	7%	500	0.150	
-	•	14%	1000	0.290	
		21%	1500	0.393	
Lateral Load Test Set Up		0%	0	0.056	
Number of Top Gauges:	0	21%	1500	0.428	
Number of Bottom Gauges:	2	29%	2000	0.530	
Height of Top Gauges [in]:	-	36%	2500	0.633	
Height of Bottom Gauges [in]:	6	0%	0	0.079	
Height of Applied Load [in]:	36	36%	2500	0.692	
Load Cell:	25k Ed Jr.	43%	3000	0.814	
	•	50%	3500	0.935	
		0%	0	0.096	
Test Date and Representati	ve	50%	3500	0.975	
Tested By Terracon Rep:	I. McGougan	57%	4000	1.095	
Date Tested:		64%	4500	1.193	
		71%	5000	1.325	
		0%	0	0.150	
Pile Information		57%	4000	1.177	
Pile ID:	PLT-12A	71%	5000	1.380	
Latitude:	40.65468	79%	5500	1.473	
Longitude:	-80.90659	86%	6000	1.688	
Pile Type:	W6x9	93%	6500	2.027	
Pile Embedment Depth [in]:	8	100%	7000		
Pile Stick-Up [in]:	36	0%	0	0.495	
Lateral Design Load [lbs]:	7000	H	•	· ·	
Drive Time [sec]:	187				

Lateral



ents

Lateral Load Test Result for PLT-12B

Project Information		Design	Load	Deflection Δ (in.)	Comm
Project Name:	Kensington Solar	Load	[lbs]	Gauges #1 & #2	
Project Location:	Summitville, Ohio	0%	0	0.000	
Project Number:	N6225000	7%	500	0.144	
-	•	14%	1000	0.260	
		21%	1500	0.378	
Lateral Load Test Set Up		0%	0	0.090	
Number of Top Gauges:	0	21%	1500	0.423	
Number of Bottom Gauges:	2	29%	2000	0.526	
Height of Top Gauges [in]:	-	36%	2500	0.632	
Height of Bottom Gauges [in]:	6	0%	0	0.114	
Height of Applied Load [in]:	36	36%	2500	0.690	
Load Cell:	25k Ed Jr.	43%	3000	0.772	
	•	50%	3500	0.900	
		0%	0	0.125	
Test Date and Representati	ve	50%	3500	0.946	
Tested By Terracon Rep:	I. McGougan	57%	4000	1.053	
Date Tested:		64%	4500	1.167	
		71%	5000	1.320	
		0%	0	0.180	
Pile Information		57%	4000	1.197	
Pile ID:	PLT-12B	71%	5000	1.365	
Latitude:	40.65468	79%	5500	1.480	
Longitude:	-80.90659	86%	6000	1.723	
Pile Type:	W6x9	93%	6500	2.000	
Pile Embedment Depth [in]:	7.1	100%	7000		
Pile Stick-Up [in]:	36	0%	0	0.481	
Lateral Design Load [lbs]:	7000				
Drive Time [sec]:	178				

Lateral



APPENDIX E – PILE LOAD TEST RESULTS – AXIAL COMPRESSION LOAD

Contents:

Exhibit E-1 to E-6

Compression Load Test Results (6 pages)

Note: All attachments are one page unless noted above.

Compression Load Test Result for PLT-4C



Compression Load Test Result for PLT-5C

Project Information Project Name: Kensington Solar **Compression Test Results** Project Location: Summitville, Ohio % of Axial Project Number: N6225000 Deflection ∆ (in.) Gauges #1 & #2 Comments Design Load Load [lbs] 0% 0 0.000 Axial Load Test Set Up 4% 500 0.002 Number of Gauges: 2 8% 1000 0.002 Height of Gauges [in]: 6 12% 1500 0.003 Load Cell: 25k Ed Jr. 15% 2000 0.003 19% 2500 0.004 23% 3000 0.005 Test Date and Representative 27% 3500 0.005 Tested By Terracon Rep: I. McGougan 31% 4000 0.005 35% Date Tested: 4500 0.006 38% 5000 0.007 42% 5500 0.008 **Pile Information** 46% 6000 0.008 Pile ID: PLT-5C 50% 6500 0.009 Latitude: 40.67996 54% 7000 0.010 Longitude: -80.89076 58% 7500 0.011 62% Pile Type: W6x9 8000 0.011 Pile Embedment Depth [in]: 5.8 65% 8500 0.012 Pile Diameter [in]: 5.9 69% 9000 0.013 Pile Stick-Up [in]: 36 73% 9500 0.013 Axial Design Load [lbs]: 13000 77% 10000 0.015 Pile Area [sq. in]: 2.96 81% 10500 0.016 Elastic Modulus [ksi]: 85% 11000 0.016 Drive Time [sec]: 139 88% 0.017 11500 92% 12000 0.018 96% 12500 0.019 100% 13000 0.020 0.00 G 0.10 0.20 0.30 Deflection (inches) 0.40 0.50 0.60 0.70 0.80 0.90 1.00 500 500 1000 1500 1500 1500 1500 1500 1500 1500 1500 1500 11000 11000 11100 11100 11100 11100 **Compression Axial Load (lbs)**

Compression Load Test Result for PLT-7C



Compression Load Test Result for PLT-8C

Project Information Project Name: Kensington Solar **Compression Test Results** Project Location: Summitville, Ohio % of Axial Project Number: N6225000 Deflection ∆ (in.) Gauges #1 & #2 Comments Design Load Load [lbs] 0% 0 0.000 Axial Load Test Set Up 4% 500 0.003 Number of Gauges: 2 8% 1000 0.005 Height of Gauges [in]: 6 12% 1500 0.009 Load Cell: 25k Ed Jr. 15% 2000 0.011 19% 2500 0.013 23% 3000 0.016 Test Date and Representative 27% 3500 0.017 Tested By Terracon Rep: I. McGougan 31% 4000 0.018 35% Date Tested: 4500 0.019 38% 5000 0.021 42% 5500 0.024 **Pile Information** 46% 6000 0.025 Pile ID: PLT-8C 50% 6500 0.028 Latitude: 40.69661 54% 7000 0.031 Longitude: -80.88478 58% 7500 0.034 62% W6x9 8000 0.037 Pile Type: Pile Embedment Depth [in]: 65% 8500 0.040 7 0.043 Pile Diameter [in]: 5.9 69% 9000 Pile Stick-Up [in]: 36 73% 9500 0.045 Axial Design Load [lbs]: 13000 77% 10000 0.047 Pile Area [sq. in]: 2.96 81% 10500 0.050 Elastic Modulus [ksi]: 85% 11000 0.053 Drive Time [sec]: 19 88% 0.055 11500 92% 12000 0.057 96% 12500 0.060 100% 13000 0.063 0.00 @ 0.10 0.20 0.30 Deflection (inches) 0.40 0.50 0.60 0.70 0.80 0.90 1.00 500 500 1000 1500 1500 1500 1500 1500 1500 1500 1500 1500 11000 11000 11100 11100 11100 11100

Compression Axial Load (lbs)

Compression Load Test Result for PLT-9C



Compression Load Test Result for PLT-12C

Project Information	(New Dr			- 0							0										ſ								
Project Project L	t Name: K	lensi	ngtoi nitvill	n Sol	lar hio			0/ /	.f	1	C0I Avial	mpre	ssio	n Tes	st Re	sults	5												
Project N	Number: N	V622	5000))	10			Desi	gn	Ĺ	oad		Def	lectio	on 🛆	(in.)		Con	nmer	nts	1								
,								Loa	d	[lbs]		Ga	uges	#1 8	#2					l								
								0%	D		0			0.0	000						ļ								
Axial Load Test Set	Up						_	4%	5		500			0.0	005						ł								
Number of C	Jauges: 2							120	0	1	000 500			0.0)01)02						ł								
Lo	ad Cell: 2	5k E	d Jr.				_	159	/0 //	2	000			0.0	002						1								
								199	6	2	500			0.0	003						i								
								23%	6	3	000			0.0)03						ļ								
Test Date and Repre	sentativ	<u>e</u>					_	27%	6	3	500			0.0	003						ł								
Tested By Terrac	on Rep: I.	McG	Gouga	an				319	<u>//</u>	4	-000 500			0.0	205						ł								
Date	resteu.						_	389	6	4	000			0.0	005						1								
								429	6	5	500			0.0	006						i								
Pile Information								46%	6	6	000			0.0	07						l								
	Pile ID: F	LT-1	2C					50%	6	6	500			0.0	008						ļ								
L	Latitude: 40.65468					54%	6	7	000	_		0.0	09						ł										
LOI Pil	e Type: W	20.90 ₩6x0	1009				⊢	58% 62°	'0 /0	/ Я	000	_		0.0)10)11						1								
Pile Embedment De	epth [in]: 8	}					⊢	65%	6	8	500	+		0.0)12		+				i								
Pile Diame	eter [in]: 5	.9						69%	6	9	000			0.0	013														
Pile Stick	-Up [in]: 3	6						73%	6	9	500			0.0)14						ļ								
Axial Design Lo	ad [lbs]: 1	3000)					779	6	10	0000	_		0.0	014						ł								
Flastic Modul	[Sq. inj: 2	90						850	/o /o	1	1000	-		0.0)15)16						1								
Drive Tim	ne [sec]: 1	73						889	%	1	1500			0.0)17						i								
								92%	6	12	2000			0.0)18						l								
								96%	6	12	2500			0.0)19						ł								
							Ļ	100	%	1.	3000	_		0.0)20						,								
	0.00		-			\bigcirc	\bigcirc	\bigcirc	\bigcirc	\bigcirc	\bigcirc	\bigcirc	\bigcirc	0	0	0	0	0	\odot	0	-0	+6)) (\bigcirc	Θ	HC	-
																		+++-										Ŭ	
	0.10																												
	0.20																												
											++-++		++-++	++-+++				+++											
	0.30	_																											
~							+++-		+++-			++-++	+++-	++-++	++++			++-+-			++++++					•			
es	0.40	-																											
с Р																						-							
<u>i</u>	0.50	-																											
Ē			++-++				++-++		++-+-	++-++	++-++	++-++	++-++	++-++	++-++			++-+	+++		+++++								
Ę	0.60	-				+++																							
ů S												++++	+++								+								
e E	0.70	-																											
Ď																													
	0.80																												
			++-++				++-+-		+++		++-+		++-++	++-+++	++++			++-+			+ + + + + +								
	0.90																												
		++		-+										+++++															-+++-
	1.00	+++	++++++			++++																					H##		
		0	ő	ő	202	ğ	00	ő	<u>S</u>	ő	00	ğ	<u>S</u>	ő	<u>S</u>	ğ	<u>S</u>	g	<u>S</u>	g	<u> </u>	g	įε	ξŞ	ž	ğ	g	0	ξ
			L)	4	4	20	25	Я	35	4	45	50	55	00	65	2	75	80	85	8	95	0	5 č	5 4	_	2	20	50	6
											Con	nnr	066	ion	۸.	ial	1.0	he	lhe	•		Υ.			-	<u>, </u>	-	~	~
											001	прI	695		. ~^	a	200	au	102	"									

-----Axial Deflection

This foregoing document was electronically filed with the Public Utilities

Commission of Ohio Docketing Information System on

5/3/2022 5:19:04 PM

in

Case No(s). 21-0764-EL-BGN

Summary: Notice Notice of Filing Data Responses electronically filed by Ms. Anna Sanyal on behalf of Kensington PV 1, LLC