

Appendix I: Geotechnical Report

Appendices

Appendix A

Site Coordinates and Investigation Summary

Appendix A
Site Coordinates and Investigation Summary

Geotech ID	Turbine ID	Structure Type	UTM NAD83 Z14N [m]		Soil Boring	Electrical Resistivity	Thermal Resistivity	Piezometer	Geophysical Testing
			Northing	Eastng					
GEO-001	2.3-1	Turbine	4546219.3	322614.9	X				
GEO-002	2.3-2	Turbine	4550155.9	328951.3	X				
GEO-003	2.3-3	Turbine	4540531.8	325135.2	X				X
GEO-004	2.3-4	Turbine	4555547.5	339126.9	X			X	
GEO-005	2.3-5	Turbine	4550876.8	330232.5	X	X	X	X	
GEO-006	2.3-6	Turbine	4551577.3	334368.0	X	X	X		
GEO-007	2.3-7	Turbine	4553214.6	334691.8	X			X	
GEO-008	2.3-8	Turbine	4547748.1	328866.6	X				
GEO-009	2.3-9	Turbine	4551544.0	338428.4	X				
GEO-010	2.3-10	Turbine	4548944.7	344822.5					
GEO-011	2.3-107	Turbine	4552418.4	336574.9					
GEO-012	2.3-12	Turbine	4549320.8	330222.6					
GEO-013	3.8-1	Turbine	4545288.4	319620.5	X				X
GEO-014	3.8-2	Turbine	4544715.0	319963.6	X				
GEO-015	3.8-3	Turbine	4546020.6	323462.6	X	X	X		
GEO-016	3.8-4	Turbine	4545670.1	323772.0	X				
GEO-017	--	Dropped	4542879.1	324854.2					
GEO-018	3.8-6	Turbine	4542650.4	325199.4	X			X	
GEO-019	3.8-7	Turbine	4541998.9	325124.8	X				
GEO-020	3.8-8	Turbine	4541256.1	324761.0	X				
GEO-021	3.8-9	Turbine	4541120.8	325225.7	X			X	
GEO-022	3.8-10	Turbine	4542520.9	326422.2	X				
GEO-023	--	Dropped	4547723.2	327081.5					
GEO-024	3.8-12	Turbine	4546802.8	326622.5	X				
GEO-025	3.8-13	Turbine	4546402.9	326953.6	X				
GEO-026	3.8-14	Turbine	4545875.5	327741.3	X				
GEO-027	--	Dropped	4546004.7	327761.1					
GEO-028	3.8-15	Turbine	4545425.5	327878.9	X				
GEO-029	--	Dropped	4547820.4	327610.2					
GEO-030	3.8-17	Turbine	4547428.1	327749.1	X				
GEO-031	3.8-18	Turbine	4545328.4	329526.3	X				
GEO-032	--	Dropped	4545165.1	329395.5					
GEO-033	3.8-19	Turbine	4551644.0	337479.1	X				X
GEO-034	3.8-20	Turbine	4551541.8	337992.8	X				
GEO-035	3.8-21	Turbine	4550629.4	338124.3	X			X	
GEO-036	3.8-22	Turbine	4550103.1	338202.1	X				
GEO-037	3.8-23	Turbine	4549927.6	338928.3	X				
GEO-038	3.8-24	Turbine	4549899.6	339979.5	X	X	X		
GEO-039	3.8-25	Turbine	4554829.0	339283.5	X				
GEO-040	3.8-26	Turbine	4551698.0	340823.8	X				
GEO-041	3.8-27	Turbine	4548942.9	341288.9	X				
GEO-042	3.8-28	Turbine	4548991.7	344231.2	X				
GEO-043	3.8-29	Turbine	4548203.7	343986.5	X				
GEO-044	3.8-29-2	Turbine	4548181.0	344107.0	X				
GEO-045	3.8-30	Turbine	4542927.6	326484.0	X				
GEO-046	--	Dropped	4543036.5	326466.9					
GEO-047	3.8-31	Turbine	4542157.5	324459.5	X				
GEO-048	3.8-31-2	Turbine	4542147.8	324163.3	X				
GEO-049	3.8-32	Turbine	4544539.9	326421.6	X				
GEO-050	3.8-33	Turbine	4544111.4	325884.5	X			X	
GEO-051	3.8-34	Turbine	4543626.0	325869.2	X				
GEO-052	3.8-35	Turbine	4548866.6	343529.1	X				
GEO-053	3.8-36	Turbine	4544801.8	319596.9	X				
GEO-054	--	Dropped	4545274.7	328724.8					
GEO-055	--	Dropped	4550584.0	338924.8					
GEO-056	3.8-39	Turbine	4550364.6	339674.3	X				
GEO-057	--	Dropped	4548099.8	345470.5					
GEO-058	3.8-41	Turbine	4547477.2	345610.7					
GEO-059	3.8-42	Turbine	4547019.2	345658.2	X				

Geotech ID	Turbine ID	Structure Type	UTM NAD83 Z14N [m]		Soil Boring	Electrical Resistivity	Thermal Resistivity	Piezometer	Geophysical Testing
			Northing	Easting					
GEO-060	3.8-43	Turbine	4543949.2	325266.8	X			X	
GEO-061	3.8-44	Turbine	4553223.6	336017.9	X				X
GEO-062	3.8-45	Turbine	4548686.1	328618.5	X				
GEO-063	3.8-46	Turbine	4552372.8	337543.9	X				
GEO-064	--	Dropped	4548400.0	343623.0					
GEO-065	--	Dropped	4548297.0	343623.0					
GEO-066	3.8-51	Turbine	4543617.4	324704.8	X				X
GEO-067	--	Dropped	4542169.5	325992.6					
GEO-068	3.8-53	Turbine	4544367.3	326870.1	X				
GEO-069	3.8-54	Turbine	4546020.7	329629.2	X				
GEO-070	3.8-55	Turbine	4547759.0	328810.0	X				
GEO-071	2.3-11/3.8-56	Turbine	4553213.4	336408.8	X				
GEO-072	3.8-57	Turbine	4553078.1	336837.0	X				
GEO-073	3.8-58	Turbine	4552429.3	336577.0					
GEO-074	3.8-59	Turbine	4552379.1	337006.9	X				
GEO-075	3.8-60	Turbine	4551391.1	338409.0	X				
GEO-076	3.8-61	Turbine	4548968.7	343381.3	X				
GEO-077	3.8-62	Turbine	4548935.4	344812.7	X				
GEO-078	3.8-63	Turbine	4545354.8	328297.0	X				
GEO-079	3.8-101	Turbine	4546147.4	325270.7	X				
GEO-080	3.8-101	Turbine	4550123.9	337534.8					
GEO-081	3.8-104	Turbine	4547664.2	326388.0	X				
GEO-082	3.8-105	Turbine	4546818.3	326235.2	X				
GEO-083	3.8-106	Turbine	4546775.0	326708.0	X				
GEO-084	3.8-5	Turbine	4542846.9	324876.3	X			X	
GEO-085	3.8-11	Turbine	4547740.3	327077.5	X				X
GEO-086	3.8-14-2	Turbine	4545922.4	327750.9	X				
GEO-087	3.8-16	Turbine	4547804.6	327652.3	X	X	X		
GEO-088	3.8-18-2	Turbine	4545185.4	329414.1	X				
GEO-089	3.8-37	Turbine	4545293.9	328728.7	X	X	X		
GEO-090	3.8-38	Turbine	4550571.2	339008.4	X				
GEO-091	3.8-40	Turbine	4548037.1	345589.1	X				
GEO-092	3.8-47	Turbine	4548374.3	343629.4	X			X	
GEO-093	3.8-47-2	Turbine	4548322.8	343634.7	X				
GEO-094	3.8-52	Turbine	4542142.6	325974.7	X				
GEO-095	3.8-48	Turbine	4543793.3	326847.9					
GEO-096	3.8-30-2	Turbine	4543010.5	326543.0	X				
Sub-Res	--	Previous Substation	4547035.0	327329.0		X	X		

Appendix B

Soil Boring Logs



Barr Engineering Company
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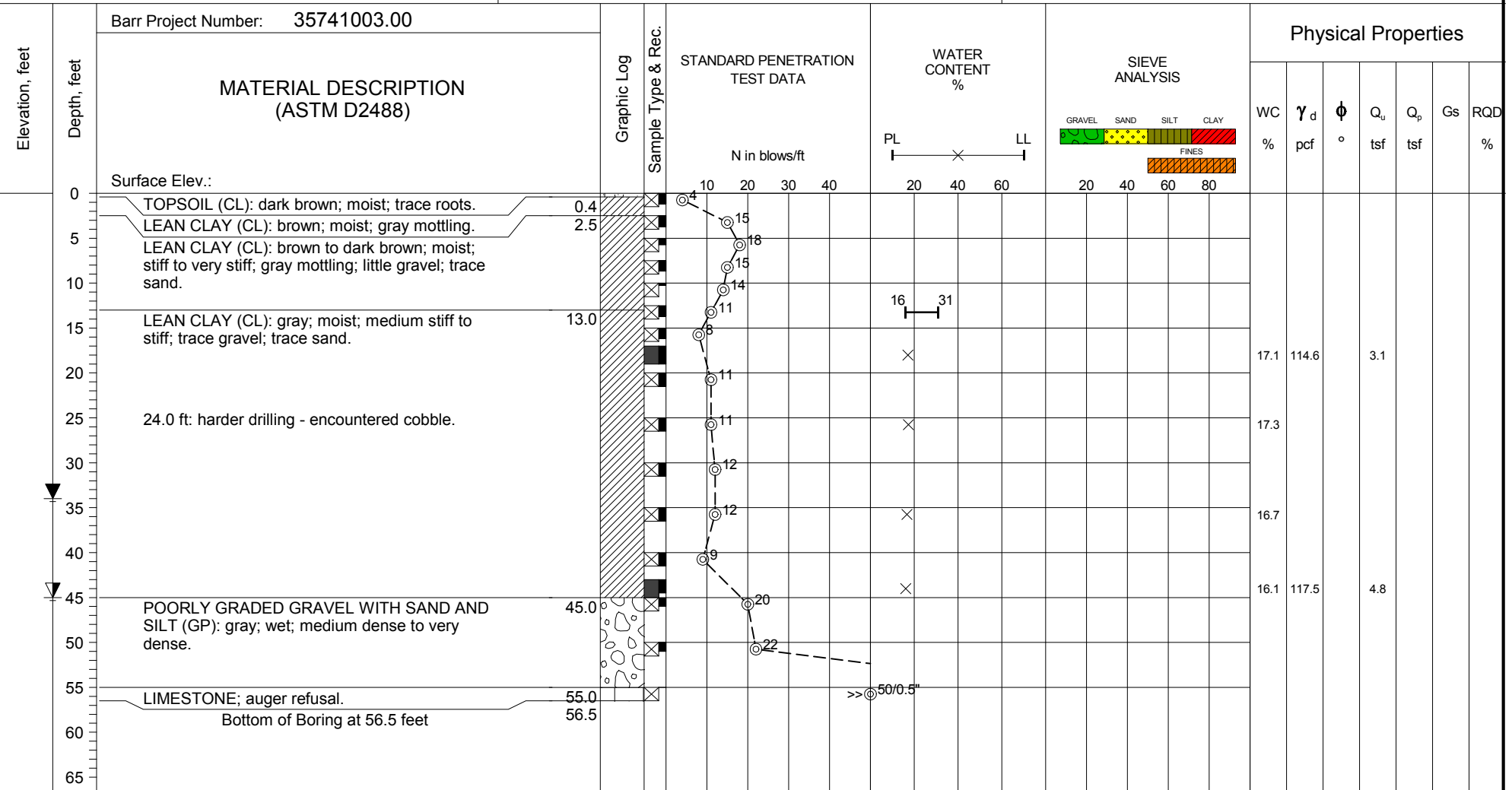
LOG OF BORING GEO-001

Sheet 1 of 1

Project: Seneca Wind Project

Location: Seneca County, Ohio

Client: sPower



Completion Depth: 56.5
Date Boring Started: 4/11/18
Date Boring Completed: 4/11/18
Logged By: BAA
Drilling Contractor: EPC Engineering and Testing
Drilling Method: HSA
Ground Surface Elevation:
Coordinates: UTM 17 N:4546219.3m, E:322614.9m
Datum: NAD83

Remarks:

SAMPLE TYPES

SPLIT SPOON 3-inch Shelby Tube

WATER LEVELS (ft)

End of Drilling 34.0
 At Time of Drilling 45.0

LEGEND

MC Moisture Content Q_u Unconfined Compression
 γ Dry Unit Weight Q_p Hand Penetrometer UC
 ϕ Friction Angle Gs Specific Gravity
RQD Rock Quality Designation

The stratification lines represent approximate boundaries. The transition may be gradual.



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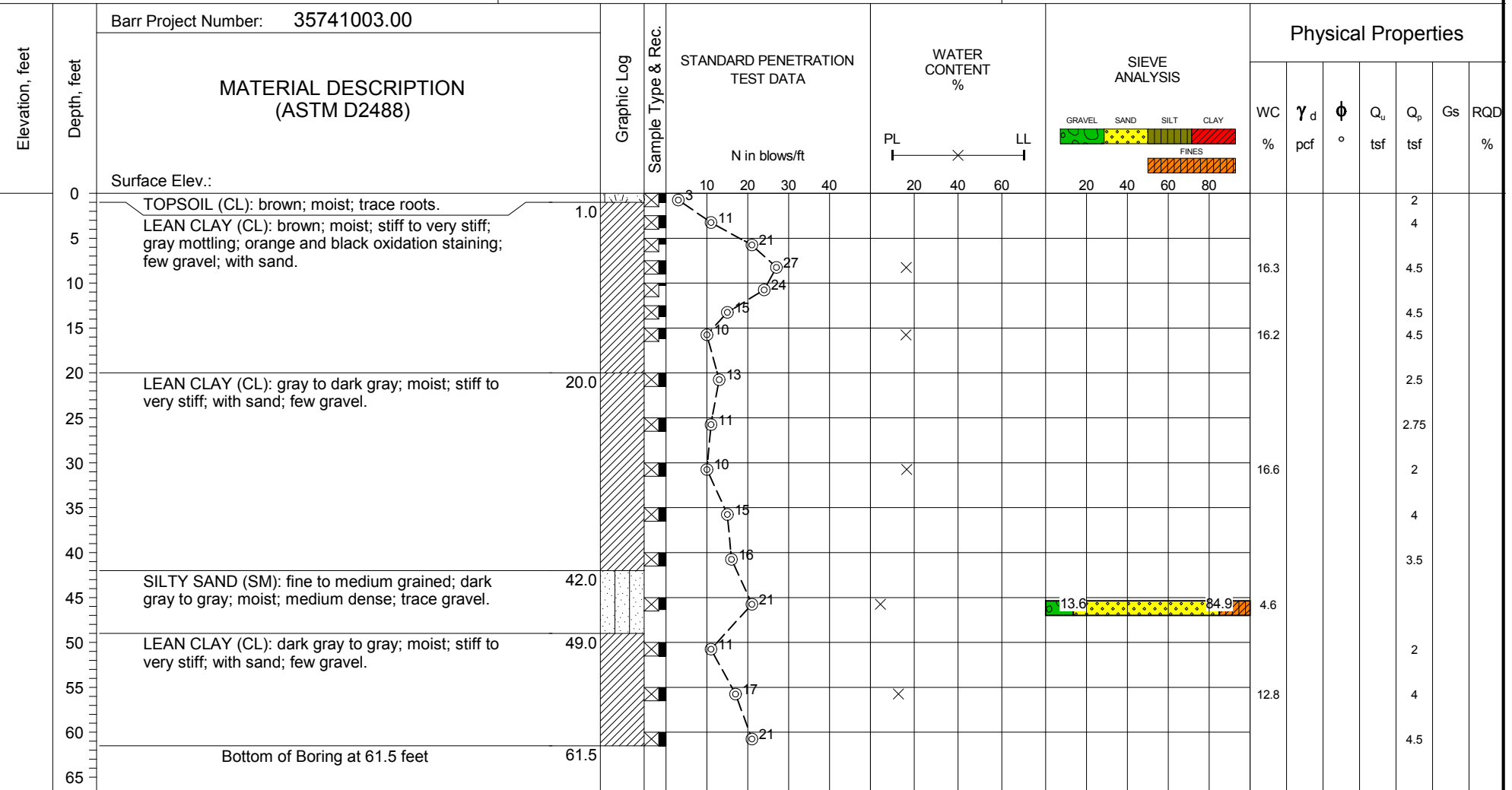
LOG OF BORING GEO-002

Sheet 1 of 1

Project: Seneca Wind Project

Location: Seneca County, Ohio

Client: sPower



Completion Depth: 61.5
Date Boring Started: 4/27/18
Date Boring Completed: 4/27/18
Logged By: DJZ
Drilling Contractor: EPC Engineering and Testing
Drilling Method: HSA
Ground Surface Elevation:
Coordinates: UTM 17 N:4550155.9m, E:328951.3m
Datum: NAD83

Remarks:

SAMPLE TYPES

⊠ SPLIT
SPOON

WATER LEVELS (ft)

▼ End of Drilling
Dry
▼ At Time of Drilling
Dry

LEGEND

MC Moisture Content
 γ Dry Unit Weight
 ϕ Friction Angle
 Q_u Unconfined Compression
 Q_p Hand Penetrometer UC
Gs Specific Gravity
RQD Rock Quality Designation

The stratification lines represent approximate boundaries. The transition may be gradual.



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LOG OF BORING GEO-003

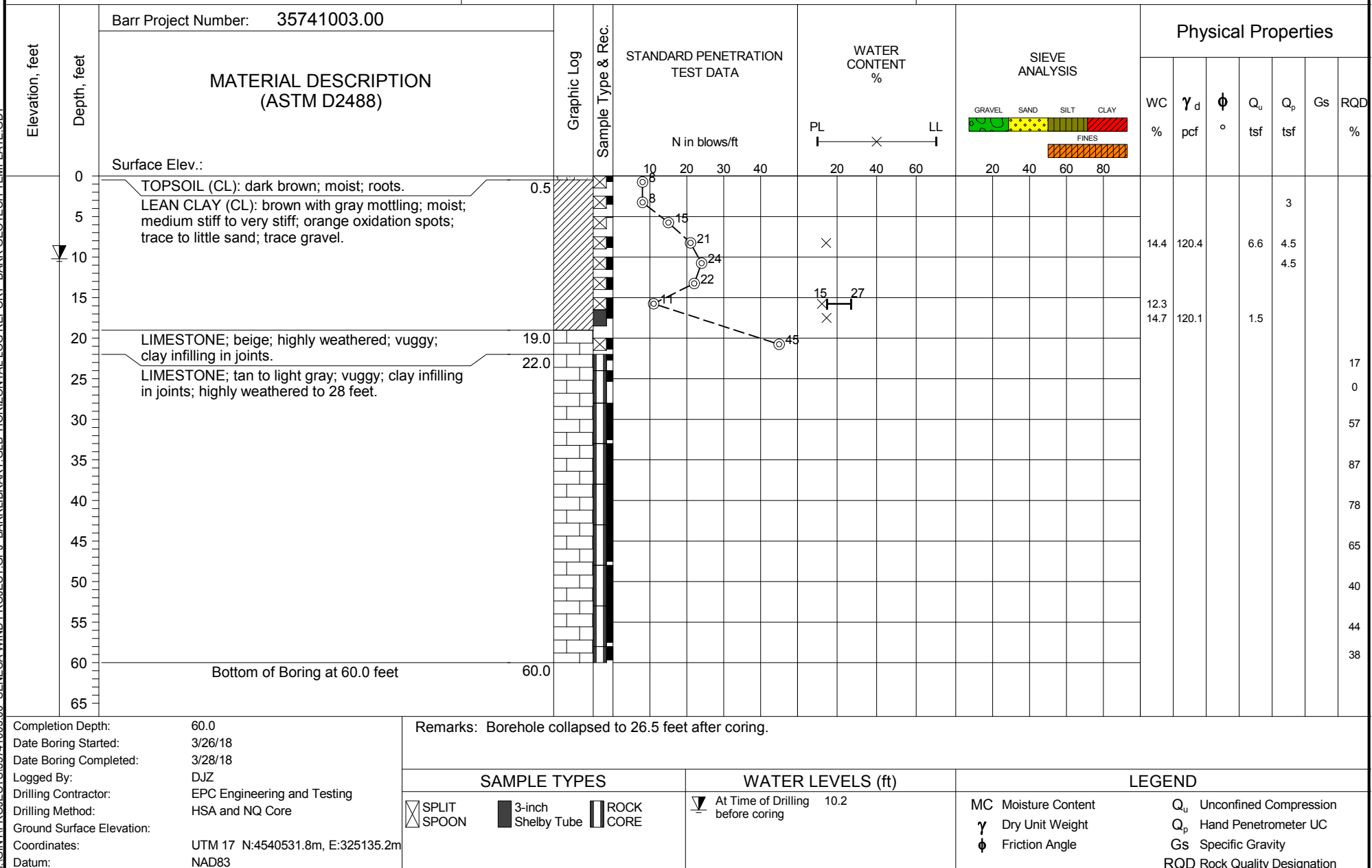
Sheet 1 of 1

Project: Seneca Wind Project

Location: Seneca County, Ohio

Client: sPower

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The stratification lines represent approximate boundaries. The transition may be gradual.



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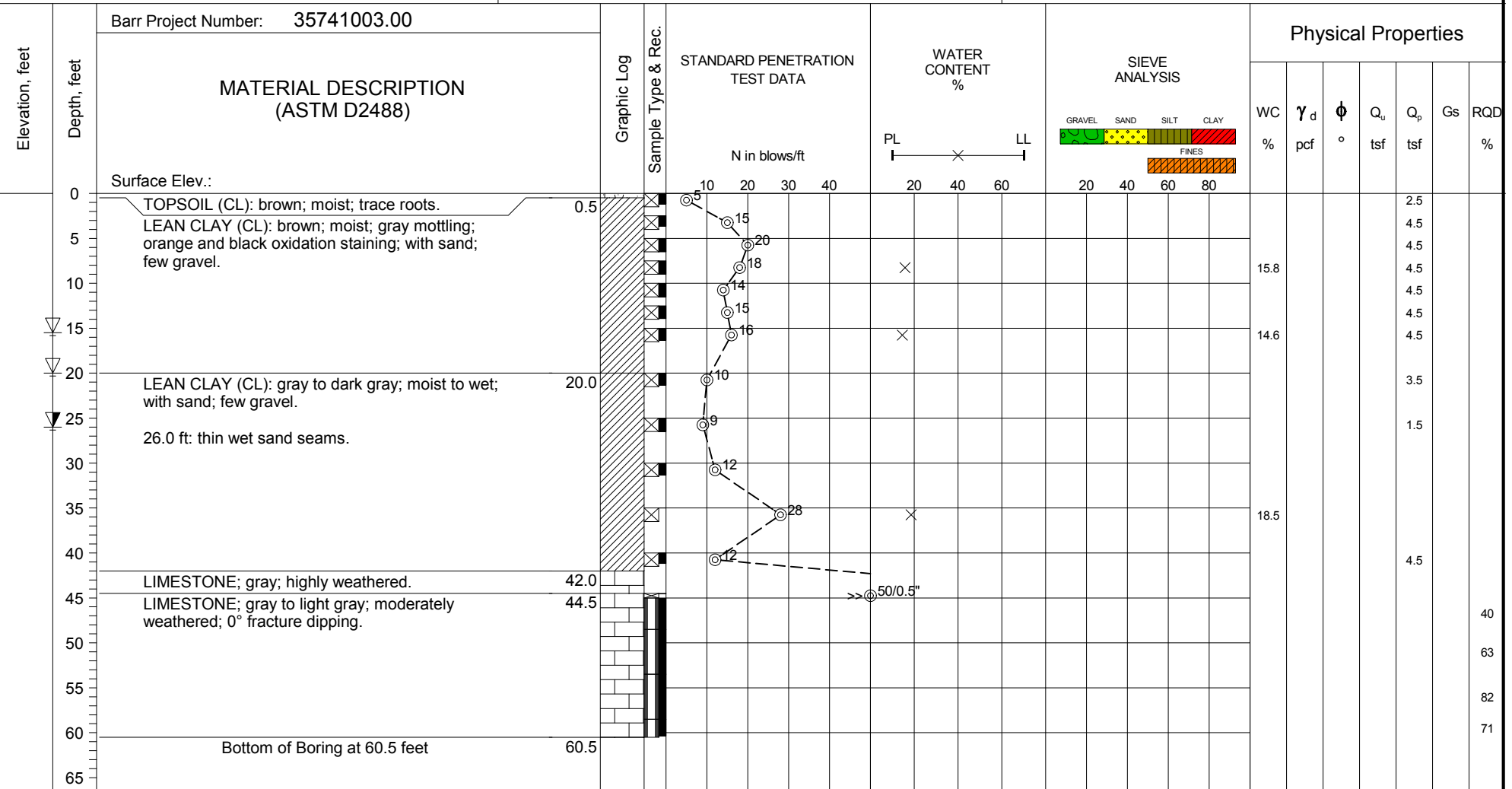
LOG OF BORING GEO-004

Sheet 1 of 1

Project: Seneca Wind Project

Location: Seneca County, Ohio

Client: sPower



Completion Depth: 60.5
Date Boring Started: 4/28/18
Date Boring Completed: 5/1/18
Logged By: DJZ
Drilling Contractor: EPC Engineering and Testing
Drilling Method: HSA and NQ Core
Ground Surface Elevation: UTM 17 N:4555547.5m, E:339126.9m
Coordinates: UTM 17 N:4555547.5m, E:339126.9m
Datum: NAD83

Remarks: Piezometer installed to a depth of 12 feet below grade.

SAMPLE TYPES

SPLIT SPOON ROCK CORE

WATER LEVELS (ft)

At Time of Drilling 26.0
 0.5 hrs After Drilling 20.0
 72 hrs After Drilling 15.5

LEGEND

MC Moisture Content Q_u Unconfined Compression
 γ Dry Unit Weight Q_p Hand Penetrometer UC
 ϕ Friction Angle Gs Specific Gravity
RQD Rock Quality Designation

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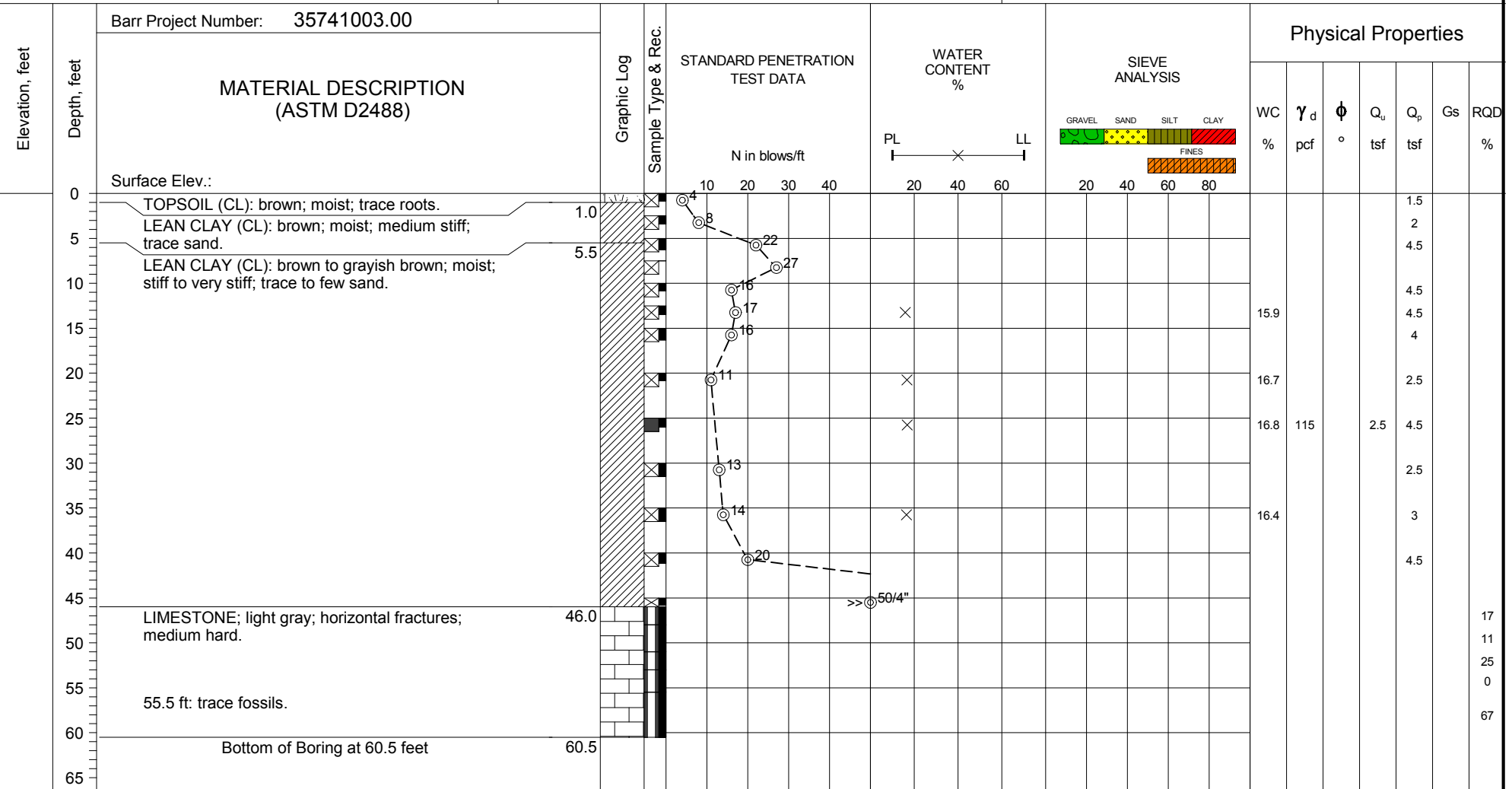
LOG OF BORING GEO-005

Sheet 1 of 1

Project: Seneca Wind Project

Location: Seneca County, Ohio

Client: sPower



Completion Depth: 60.5
Date Boring Started: 4/21/18
Date Boring Completed: 5/2/18
Logged By: IGM / DJZ
Drilling Contractor: EPC Engineering and Testing
Drilling Method: HSA
Ground Surface Elevation:
Coordinates: UTM 17 N:4550876.8m, E:330232.5m
Datum: NAD83

Remarks: Cobbles at surface.

SAMPLE TYPES

SPLIT SPOON 3-inch Shelby Tube ROCK CORE

WATER LEVELS (ft)

End of Drilling
 Dry
 At Time of Drilling
 Dry

LEGEND

MC Moisture Content Q_u Unconfined Compression
 γ Dry Unit Weight Q_p Hand Penetrometer UC
 ϕ Friction Angle Gs Specific Gravity
RQD Rock Quality Designation

The stratification lines represent approximate boundaries. The transition may be gradual.



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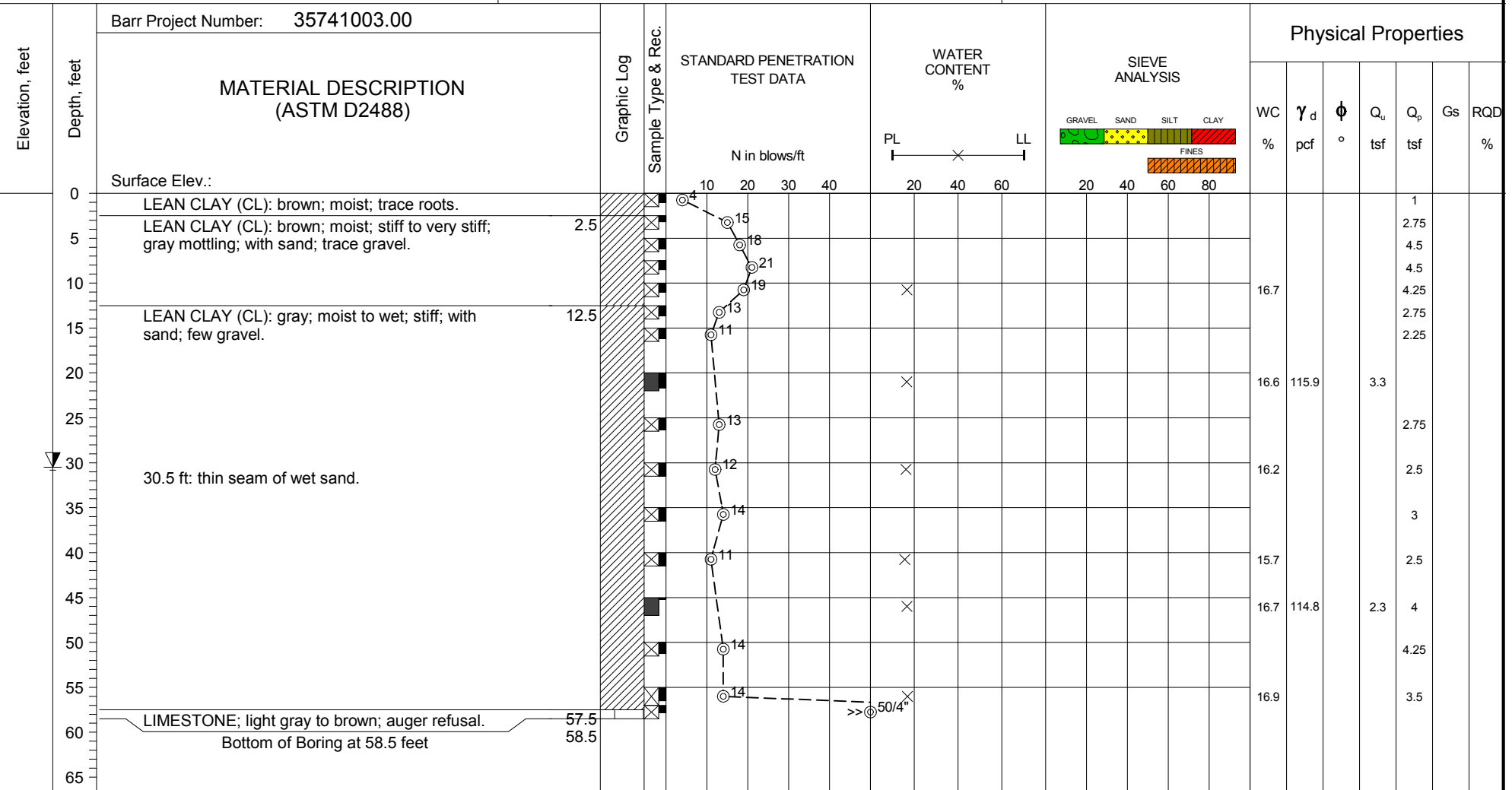
LOG OF BORING GEO-006

Sheet 1 of 1

Project: Seneca Wind Project

Location: Seneca County, Ohio

Client: sPower



Completion Depth:	58.5	Remarks:
Date Boring Started:	4/22/18	
Date Boring Completed:	4/22/18	
Logged By:	IGM	
Drilling Contractor:	EPC Engineering and Testing	
Drilling Method:	HSA	
Ground Surface Elevation:		
Coordinates:	UTM 17 N:4551577.3m, E:334368.0m	
Datum:	NAD83	

SAMPLE TYPES	WATER LEVELS (ft)	LEGEND
SPLIT SPOON	End of Drilling Dry	MC Moisture Content
3-inch Shelby Tube	At Time of Drilling 30.5	Q_u Unconfined Compression
		γ Dry Unit Weight
		ϕ Friction Angle
		Q_p Hand Penetrometer UC
		Gs Specific Gravity
		RQD Rock Quality Designation

The stratification lines represent approximate boundaries. The transition may be gradual.



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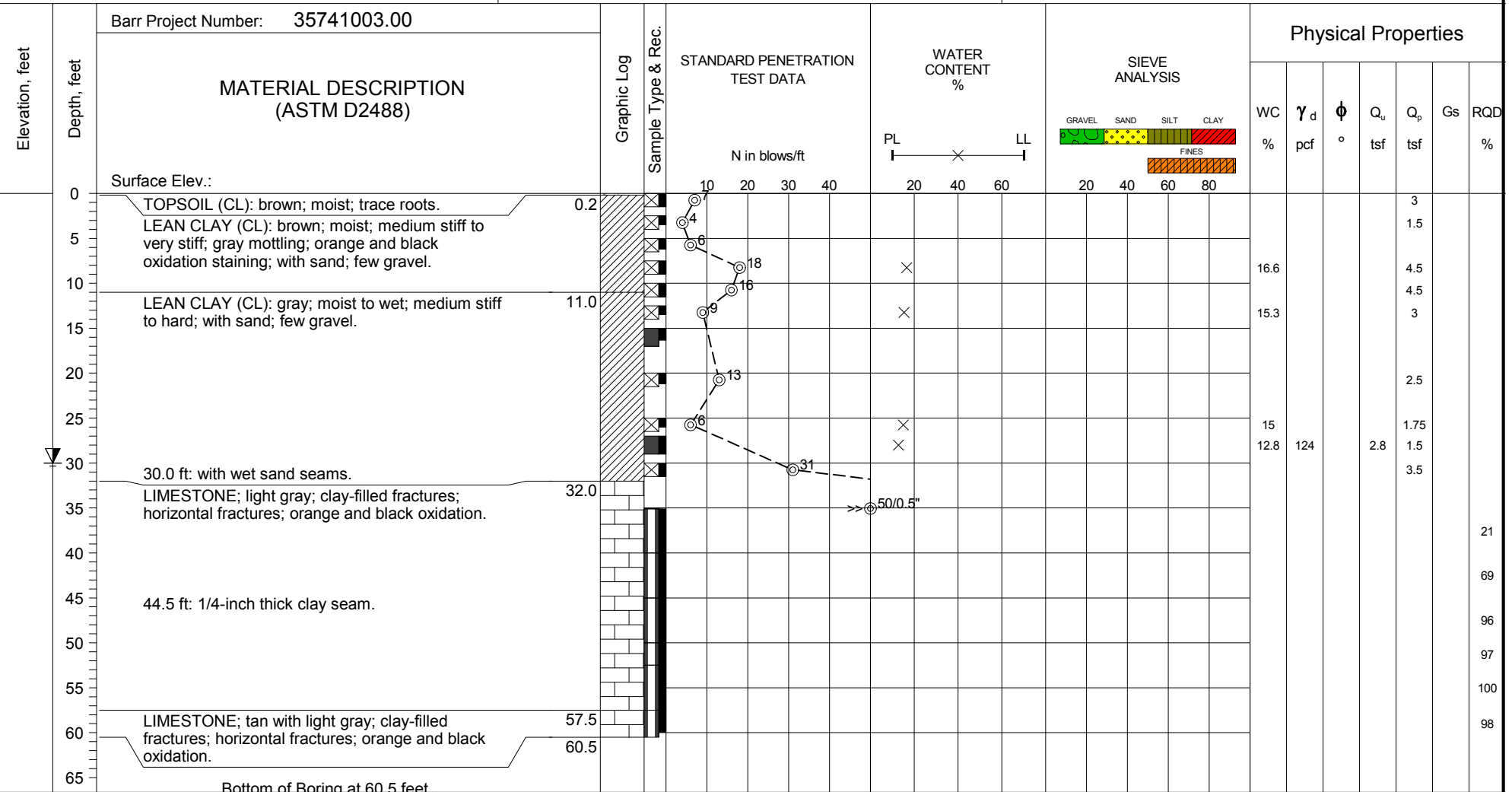
LOG OF BORING GEO-007

Sheet 1 of 1

Project: Seneca Wind Project

Location: Seneca County, Ohio

Client: sPower



Completion Depth: 60.5
Date Boring Started: 4/26/18
Date Boring Completed: 5/1/18
Logged By: DJZ
Drilling Contractor: EPC Engineering and Testing
Drilling Method: HSA and NQ Core
Ground Surface Elevation:
Coordinates: UTM 17 N:4553214.6m, E:334691.8m
Datum: NAD83

Remarks:

SAMPLE TYPES

⊠ SPLIT SPOON 3-inch Shelby Tube █ ROCK CORE

WATER LEVELS (ft)

▼ At Time of Drilling 30.0
▽ After Drilling before coring 30.0

LEGEND

MC Moisture Content Q_u Unconfined Compression
 γ Dry Unit Weight Q_p Hand Penetrometer UC
 ϕ Friction Angle Gs Specific Gravity
RQD Rock Quality Designation

The stratification lines represent approximate boundaries. The transition may be gradual.



Sheet 1 of 1

Client: sPower

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The stratification lines represent approximate boundaries. The transition may be gradual.



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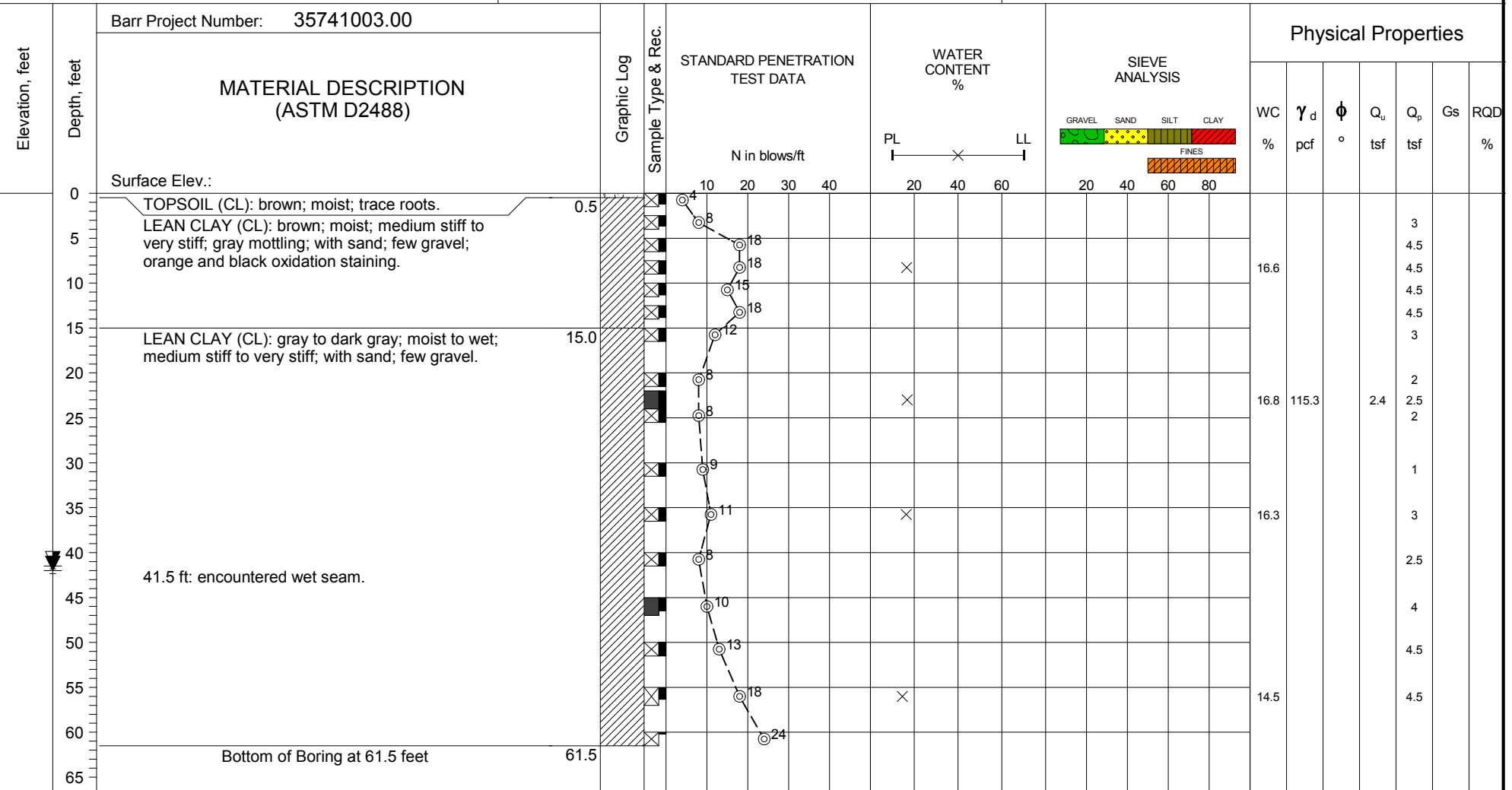
LOG OF BORING GEO-009

Sheet 1 of 1

Project: Seneca Wind Project

Location: Seneca County, Ohio

Client: sPower



Completion Depth: 61.5
Date Boring Started: 5/9/18
Date Boring Completed: 5/9/18
Logged By: DJZ
Drilling Contractor: EPC Engineering and Testing
Drilling Method: HSA
Ground Surface Elevation:
Coordinates: UTM 17 N:4551544.0m, E:338428.4m
Datum: NAD83

Remarks:

SAMPLE TYPES

SPLIT SPOON 3-inch Shelby Tube

WATER LEVELS (ft)

End of Drilling 42.0
 At Time of Drilling 41.5

LEGEND

MC Moisture Content Q_u Unconfined Compression
 γ Dry Unit Weight Q_p Hand Penetrometer UC
 ϕ Friction Angle Gs Specific Gravity
RQD Rock Quality Designation

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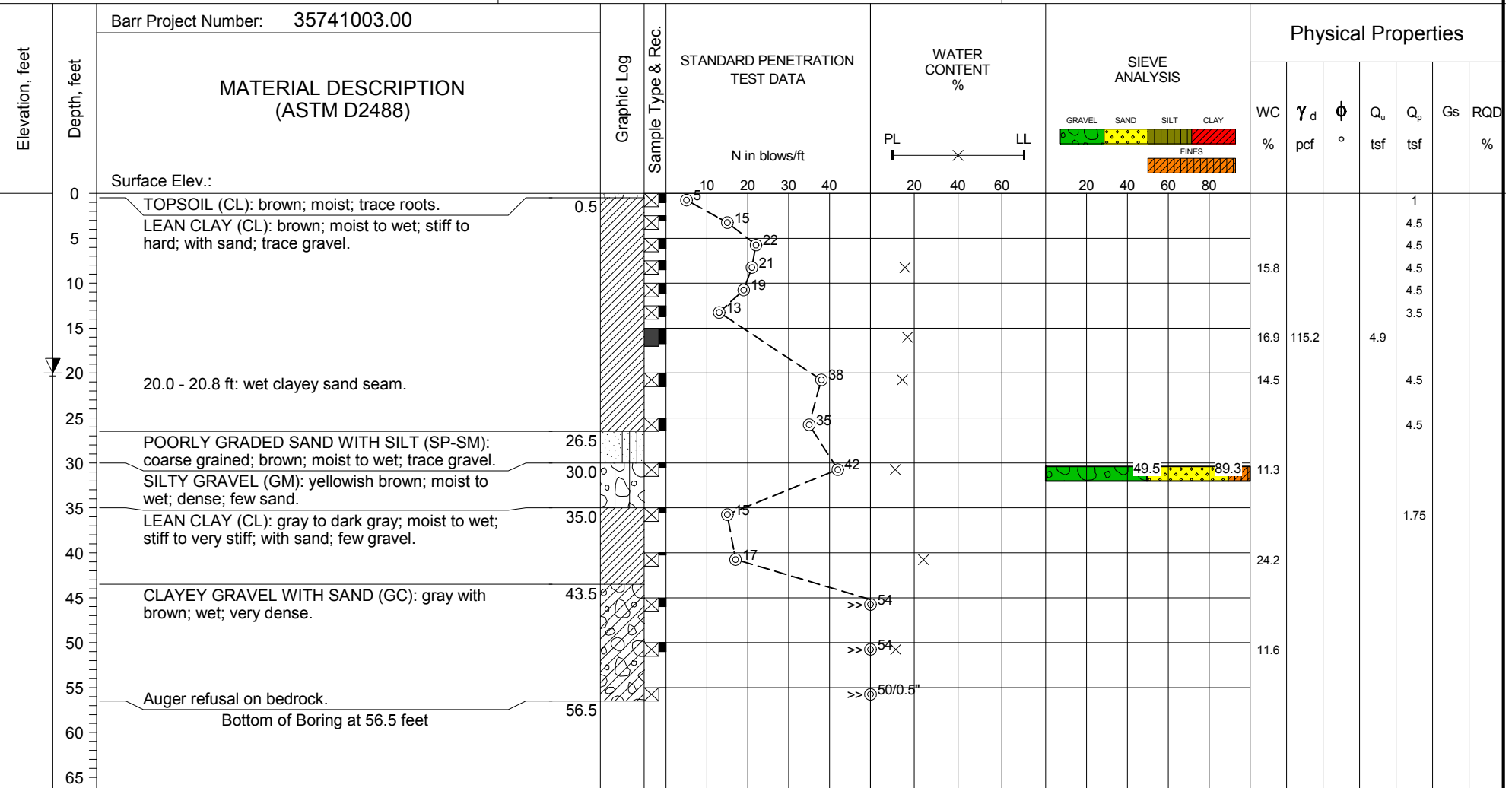
LOG OF BORING GEO-013

Sheet 1 of 1

Project: Seneca Wind Project

Location: Seneca County, Ohio

Client: sPower



Completion Depth: 56.5
Date Boring Started: 4/18/18
Date Boring Completed: 4/20/18
Logged By: IGM
Drilling Contractor: EPC Engineering and Testing
Drilling Method: HSA / Mud rotary
Ground Surface Elevation:
Coordinates: UTM 17 N:4545288.4m, E:319620.5m
Datum: NAD83

Remarks:

SAMPLE TYPES

⊗ SPLIT SPOON 3-inch Shelby Tube

WATER LEVELS (ft)

▼ At Time of Drilling 20.0

LEGEND

MC	Moisture Content	Q_u	Unconfined Compression
γ	Dry Unit Weight	Q_p	Hand Penetrometer UC
ϕ	Friction Angle	Gs	Specific Gravity
		RQD	Rock Quality Designation

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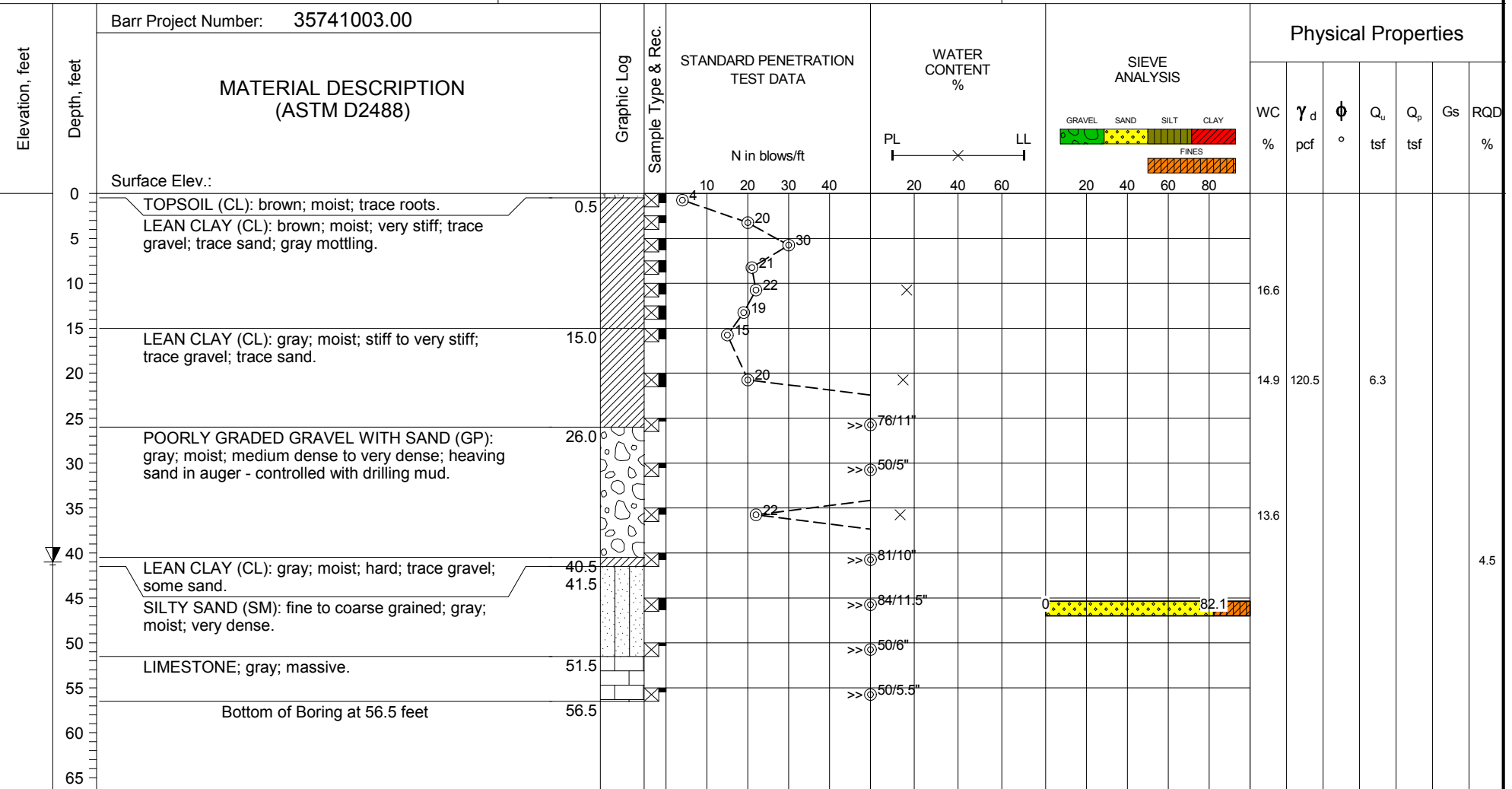
LOG OF BORING GEO-014

Sheet 1 of 1

Project: Seneca Wind Project

Location: Seneca County, Ohio

Client: sPower



Completion Depth: 56.5
Date Boring Started: 4/11/18
Date Boring Completed: 4/12/18
Logged By: BAA / BWL
Drilling Contractor: EPC Engineering and Testing
Drilling Method: HSA / Mud rotary
Ground Surface Elevation: UTM 17 N:4544715.0m, E:319963.6m
Coordinates: UTM 17 N:4544715.0m, E:319963.6m
Datum: NAD83

Remarks:

SAMPLE TYPES

⊗ SPLIT
⊗ SPOON

WATER LEVELS (ft)

▼ At Time of Drilling 41.0

LEGEND

MC	Moisture Content	Q_u	Unconfined Compression
γ	Dry Unit Weight	Q_p	Hand Penetrometer UC
ϕ	Friction Angle	Gs	Specific Gravity
		RQD	Rock Quality Designation

The stratification lines represent approximate boundaries. The transition may be gradual.



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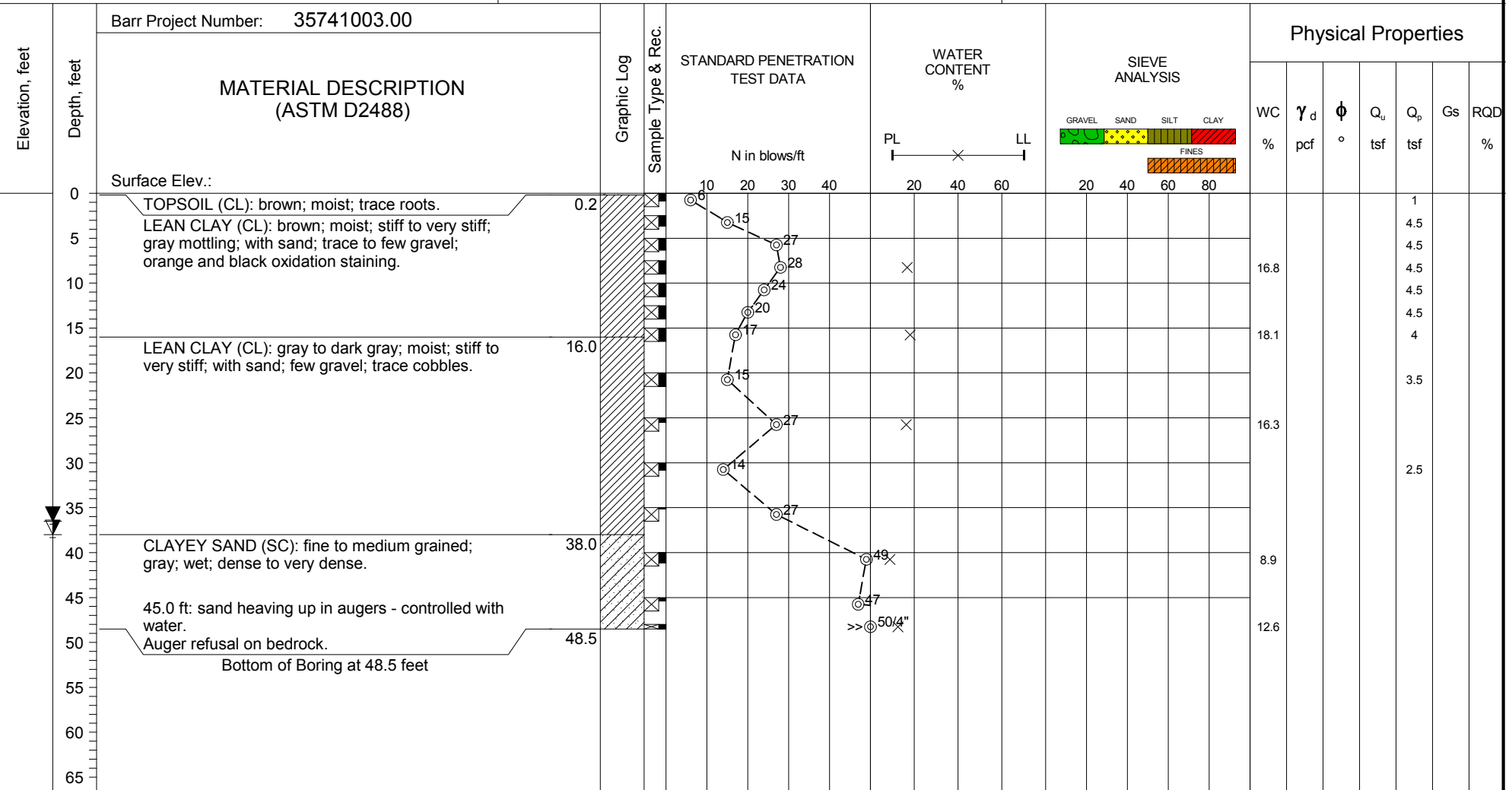
LOG OF BORING GEO-015

Sheet 1 of 1

Project: Seneca Wind Project

Location: Seneca County, Ohio

Client: sPower



Completion Depth: 48.5
Date Boring Started: 4/20/18
Date Boring Completed: 4/20/18
Logged By: DJZ
Drilling Contractor: EPC Engineering and Testing
Drilling Method: HSA
Ground Surface Elevation:
Coordinates: UTM 17 N:4546020.6m, E:323462.6m
Datum: NAD83

Remarks:

SAMPLE TYPES

⊠ SPLIT
⊠ SPOON

WATER LEVELS (ft)

▼ End of Drilling 36.5
▼ At Time of Drilling 38.0

LEGEND

MC	Moisture Content	Q_u	Unconfined Compression
γ	Dry Unit Weight	Q_p	Hand Penetrometer UC
ϕ	Friction Angle	Gs	Specific Gravity
		RQD	Rock Quality Designation

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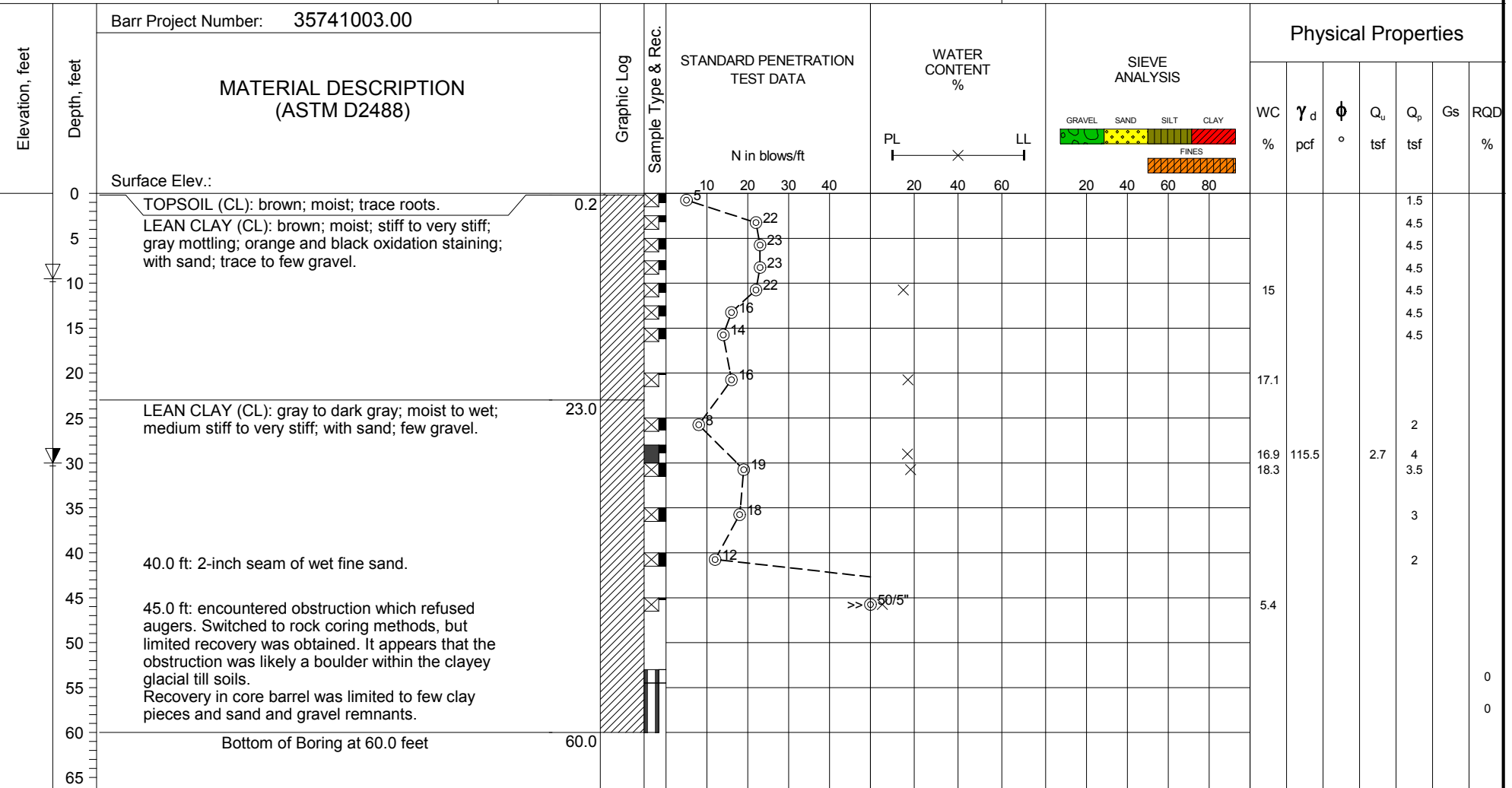
LOG OF BORING GEO-016

Sheet 1 of 1

Project: Seneca Wind Project

Location: Seneca County, Ohio

Client: sPower



Completion Depth: 60.0
Date Boring Started: 4/20/18
Date Boring Completed: 4/30/18
Logged By: DJZ
Drilling Contractor: EPC Engineering and Testing
Drilling Method: HSA and NQ Core
Ground Surface Elevation:
Coordinates: UTM 17 N:4545670.1m, E:323772.0m
Datum: NAD83

Remarks:

SAMPLE TYPES

⊠ SPLIT SPOON 3-inch Shelby Tube ROCK CORE

WATER LEVELS (ft)

▽ At Time of Drilling 30.0
▽ 100 hrs After Drilling 9.5 before coring

LEGEND

MC Moisture Content Q_u Unconfined Compression
 γ Dry Unit Weight Q_p Hand Penetrometer UC
 ϕ Friction Angle Gs Specific Gravity
RQD Rock Quality Designation

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LOG OF BORING GEO-018

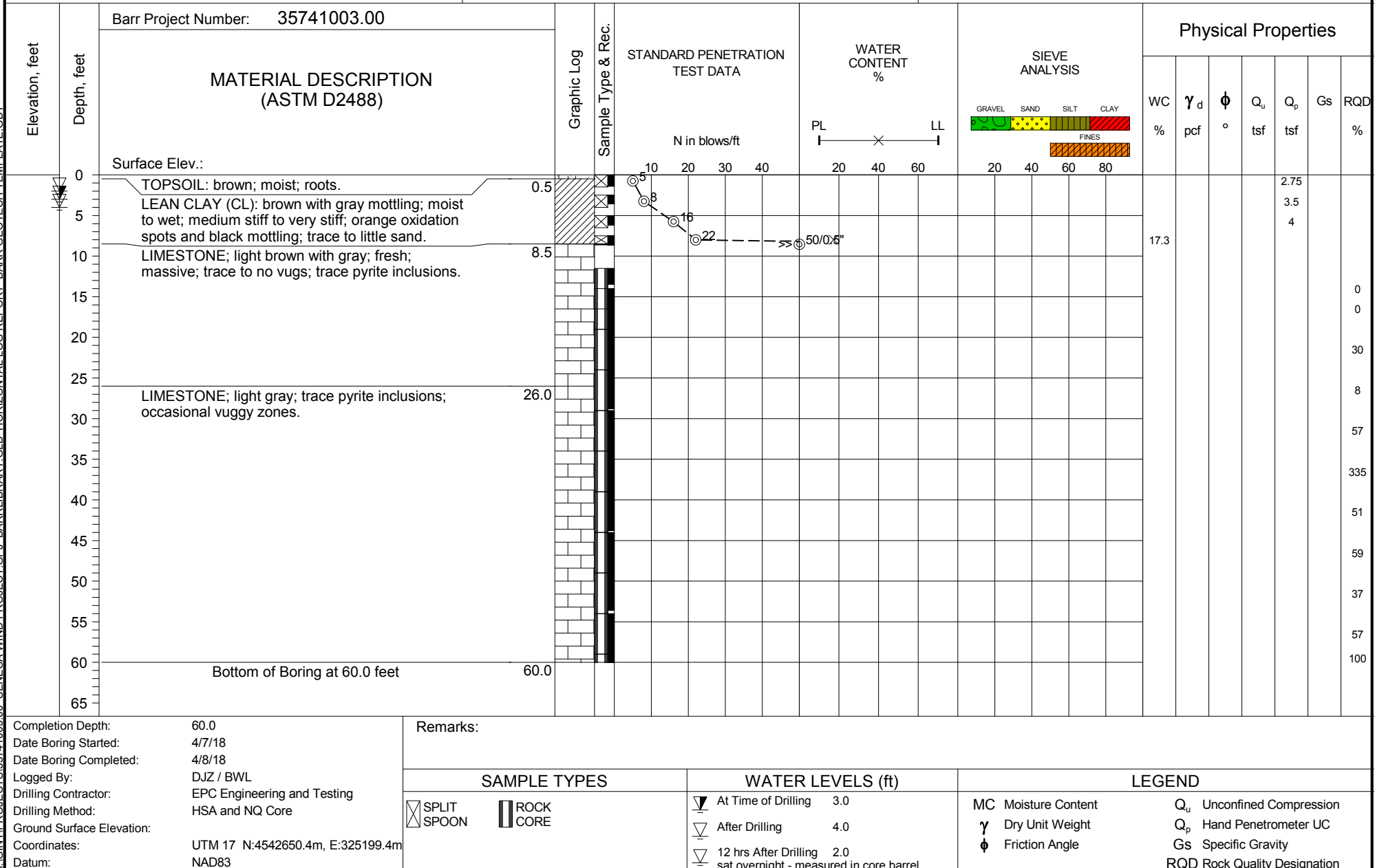
Sheet 1 of 1

Project: Seneca Wind Project

Location: Seneca County, Ohio

Client: sPower

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LOG OF BORING GEO-019

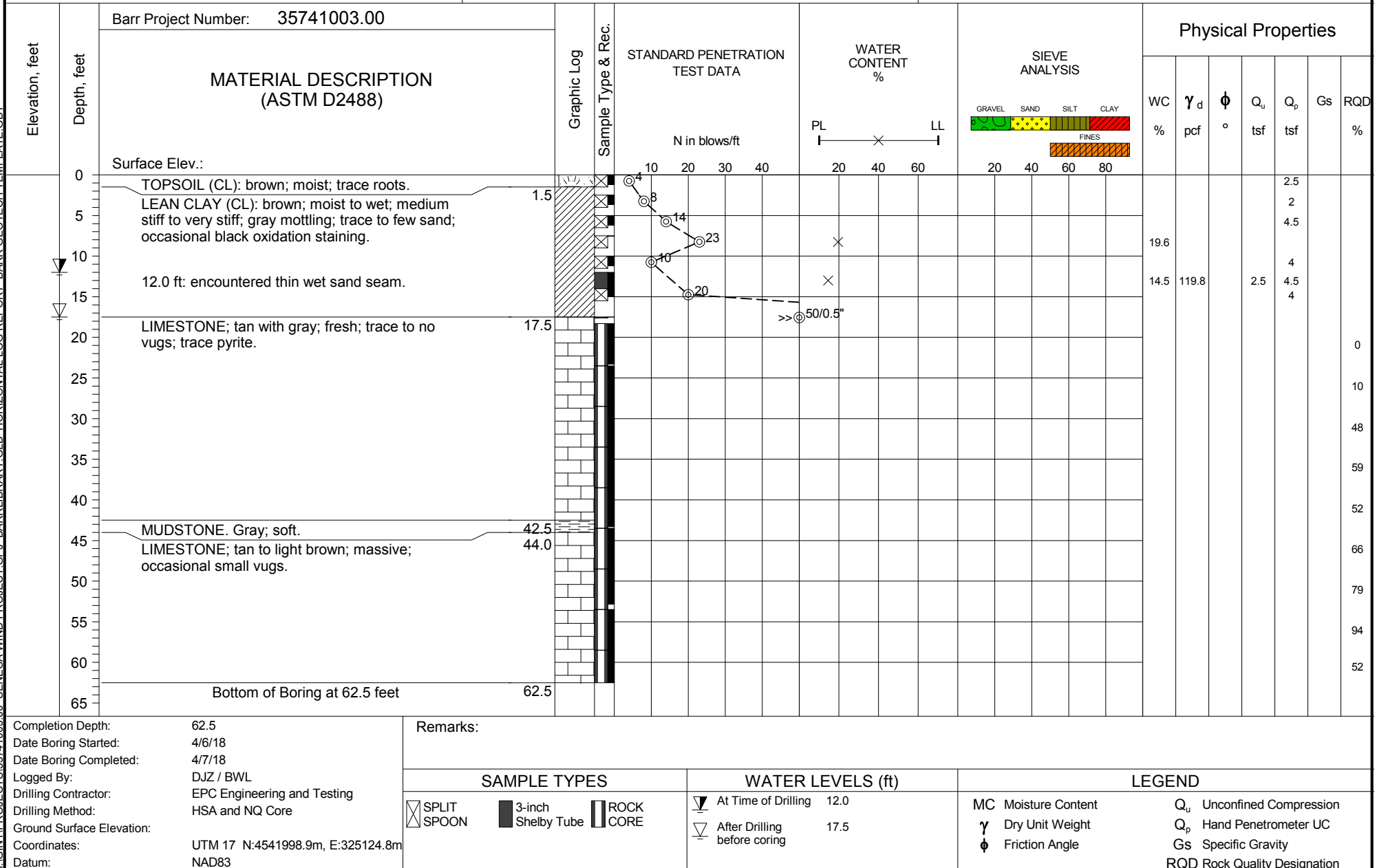
Sheet 1 of 1

Project: Seneca Wind Project

Location: Seneca County, Ohio

Client: sPower

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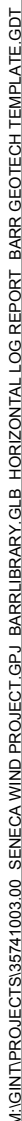


The stratification lines represent approximate boundaries. The transition may be gradual.



Sheet 1 of 1

Client: sPower



MC	Moisture Content	Q_u	Unconfined Compression
γ	Dry Unit Weight	Q_p	Hand Penetrometer UC
ϕ	Friction Angle	Gs	Specific Gravity
		RQD	Rock Quality Designation

The stratification lines represent approximate boundaries. The transition may be gradual.



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LOG OF BORING GEO-021

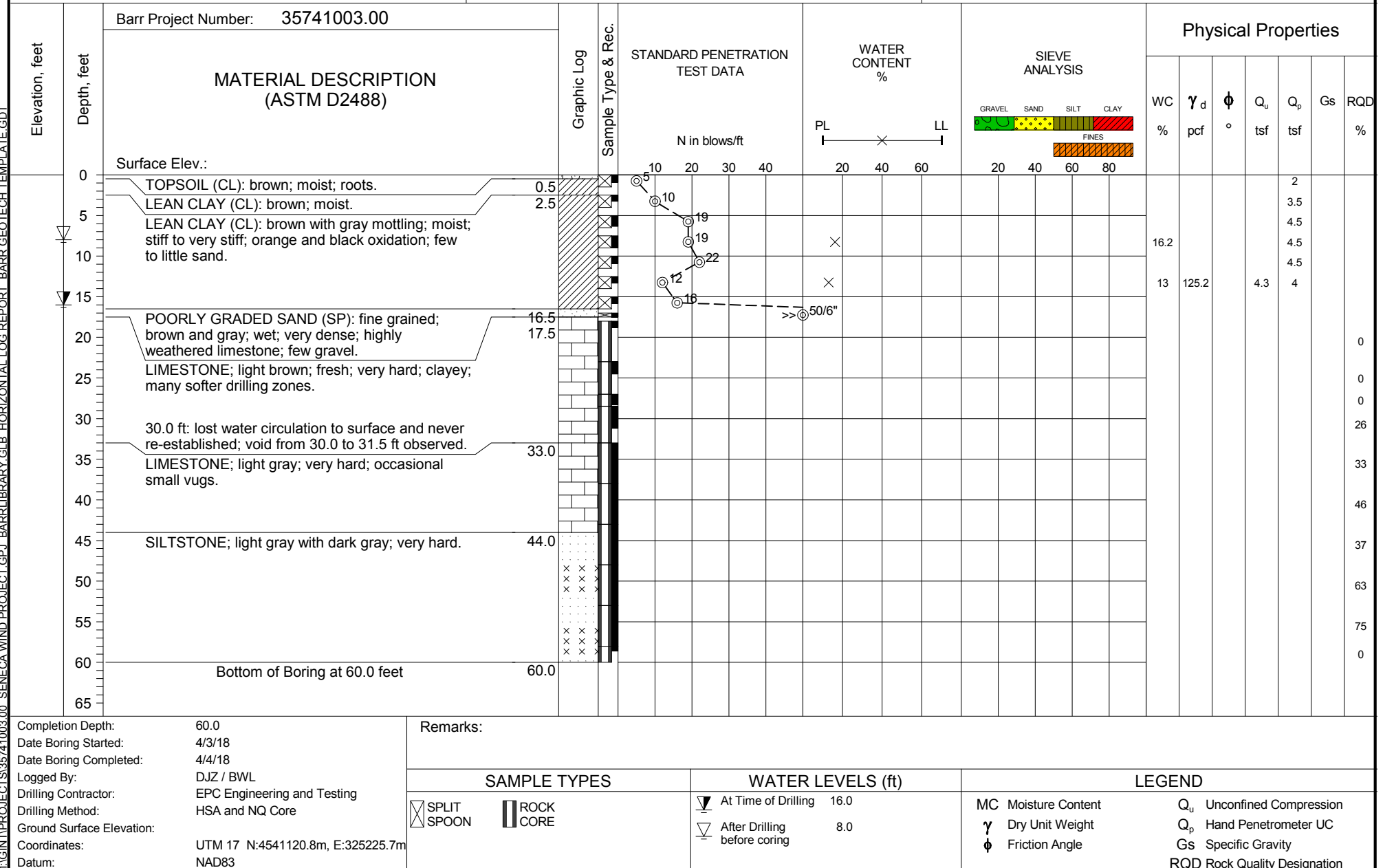
Sheet 1 of 1

Project: Seneca Wind Project

Location: Seneca County, Ohio

Client: sPower

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The stratification lines represent approximate boundaries. The transition may be gradual.



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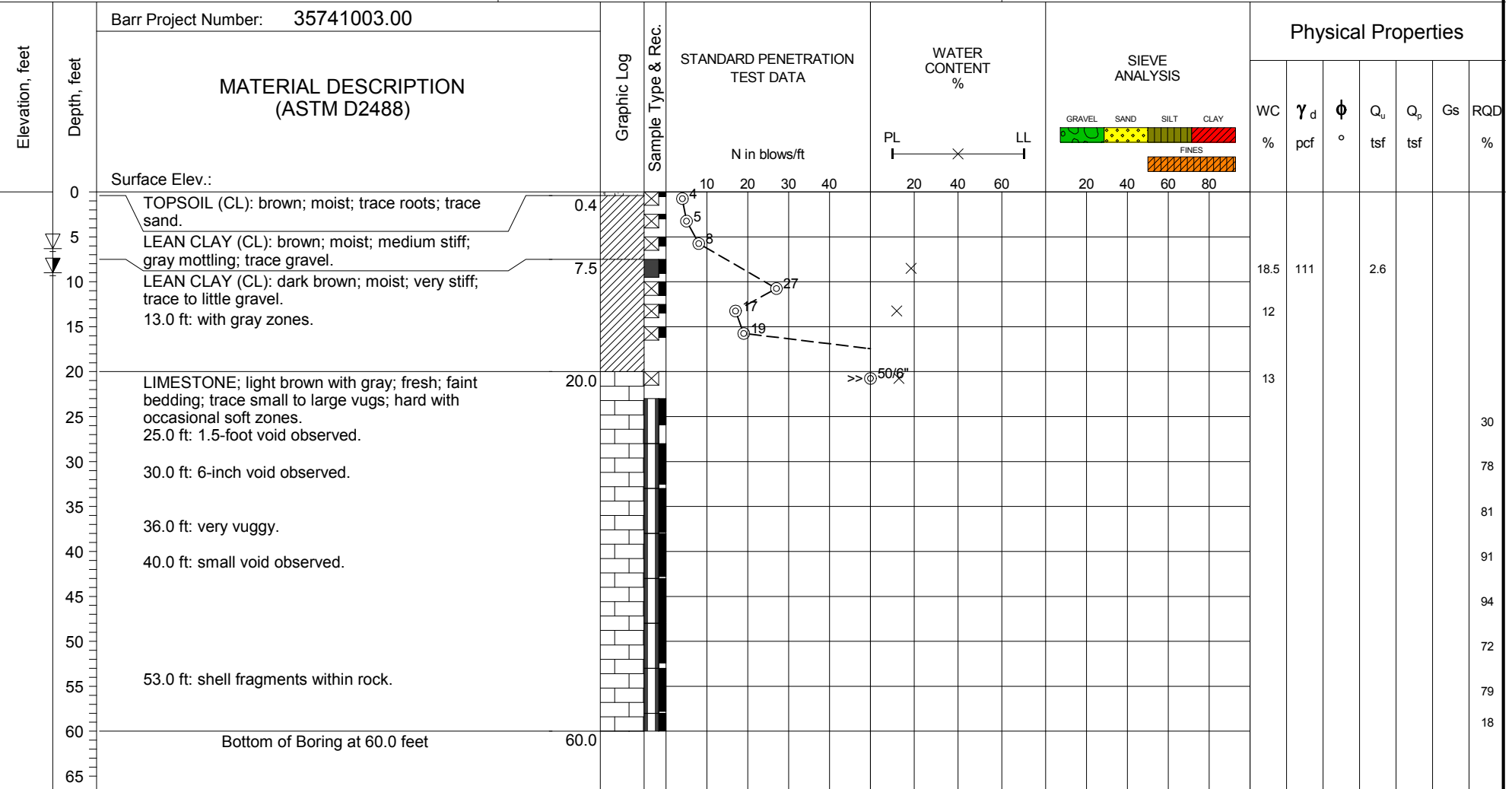
LOG OF BORING GEO-022

Sheet 1 of 1

Project: Seneca Wind Project

Location: Seneca County, Ohio

Client: sPower



Completion Depth: 60.0
Date Boring Started: 4/11/18
Date Boring Completed: 4/11/18
Logged By: BAA / BWL
Drilling Contractor: EPC Engineering and Testing
Drilling Method: HSA and NQ Core
Ground Surface Elevation:
Coordinates: UTM 17 N:4542520.9m, E:326422.2m
Datum: NAD83

Remarks:

SAMPLE TYPES

⊠ SPLIT SPOON 3-inch Shelby Tube █ ROCK CORE

WATER LEVELS (ft)

▽ At Time of Drilling 9.0
▽ 6 hrs After Drilling 6.3

LEGEND

MC Moisture Content Q_u Unconfined Compression
 γ Dry Unit Weight Q_p Hand Penetrometer UC
 ϕ Friction Angle Gs Specific Gravity
RQD Rock Quality Designation

The stratification lines represent approximate boundaries. The transition may be gradual.



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LOG OF BORING GEO-024

Sheet 1 of 1

Project: Seneca Wind Project

Location: Seneca County, Ohio

Client: sPower

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Elevation, feet	Depth, feet	Barr Project Number: 35741003.00	MATERIAL DESCRIPTION (ASTM D2488)	Graphic Log	Sample Type & Rec.	STANDARD PENETRATION TEST DATA N in blows/ft	WATER CONTENT %	SIEVE ANALYSIS GRAVEL SAND SILT CLAY FINES	Physical Properties						
									WC %	γ_d pcf	ϕ °	Q_u tsf	Q_p tsf	Gs	RQD %
	0		Surface Elev.:												
	5		LEAN CLAY (CL): brown; moist; stiff to hard; some sand; little gravel.			5									
	10					17									
	15					22									
	20					25									
	25					25									
	30					31									
	35					18									
	40					10									
	45					11									
	50					19									
	55					11									
	60					18									
	65					14									
			SILTY SAND (SM): fine to medium grained; gray; moist; medium dense.												
			LEAN CLAY (CL): brown; moist; stiff; trace gravel; with sand.												
			Auger refusal on bedrock.												
			Bottom of Boring at 47.5 feet												
Completion Depth:		47.5	Remarks:												
Date Boring Started:		4/17/18													
Date Boring Completed:		4/17/18													
Logged By:		IGM													
Drilling Contractor:		EPC Engineering and Testing													
Drilling Method:		HSA / Mud rotary													
Ground Surface Elevation:															
Coordinates:		UTM 17 N:4546802.8m, E:326622.5m													
Datum:		NAD83													
SAMPLE TYPES						WATER LEVELS (ft)			LEGEND						
SPLIT SPOON						At Time of Drilling before mud rotaryDry			MC	Moisture Content	Q_u	Unconfined Compression			
									γ	Dry Unit Weight	Q_p	Hand Penetrometer UC			
									ϕ	Friction Angle	Gs	Specific Gravity			
												RQD	Rock Quality Designation		

The stratification lines represent approximate boundaries. The transition may be gradual.



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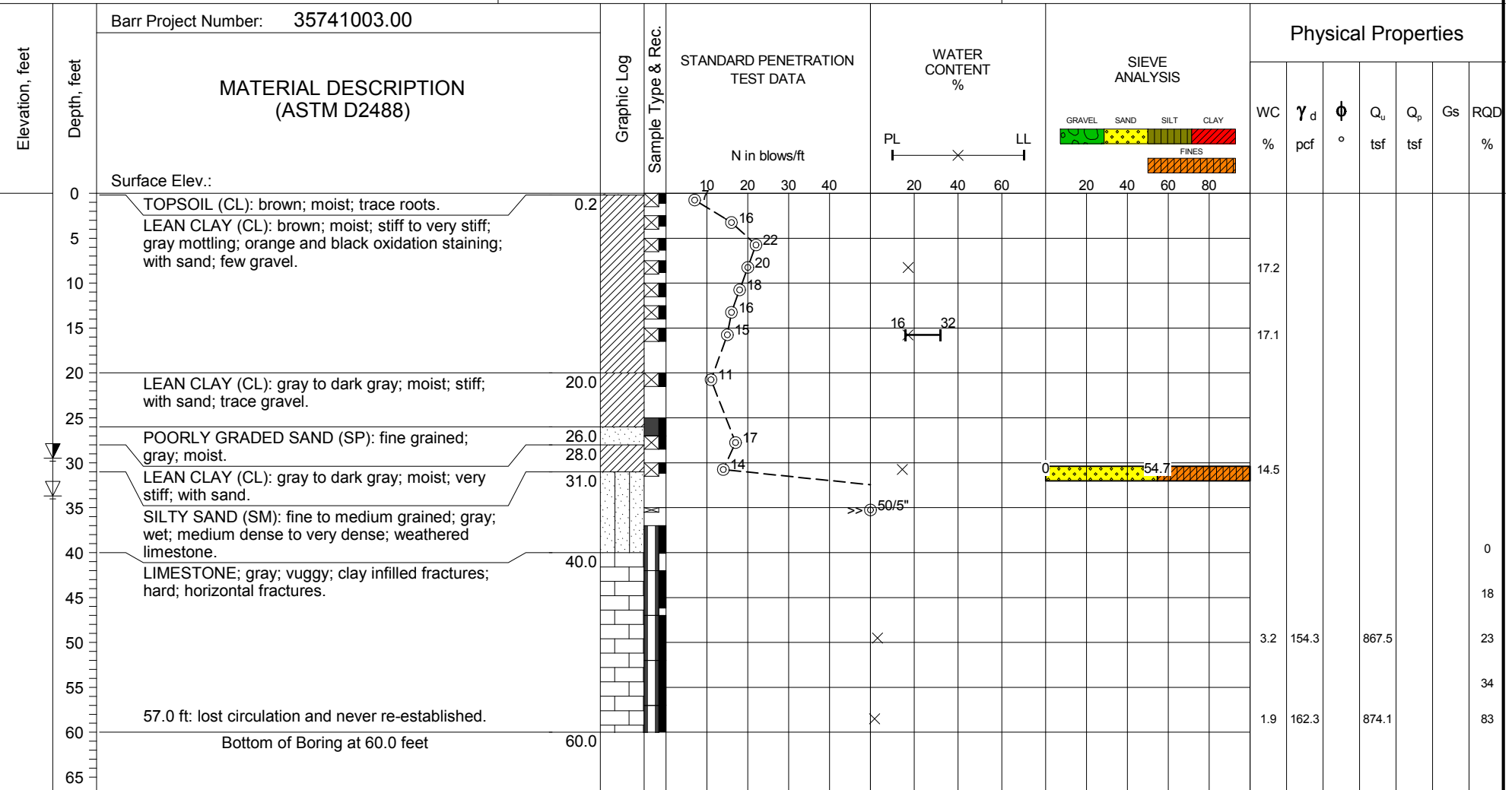
LOG OF BORING GEO-025

Sheet 1 of 1

Project: Seneca Wind Project

Location: Seneca County, Ohio

Client: sPower



Completion Depth:	60.0	Remarks:
Date Boring Started:	4/16/18	
Date Boring Completed:	4/16/18	
Logged By:	DJZ	
Drilling Contractor:	EPC Engineering and Testing	
Drilling Method:	HSA and NQ Core	
Ground Surface Elevation:		
Coordinates:	UTM 17 N:4546402.9m, E:326953.6m	
Datum:	NAD83	
SAMPLE TYPES		WATER LEVELS (ft)
LEGEND		
MC Moisture Content		Q_u Unconfined Compression
γ Dry Unit Weight		Q_p Hand Penetrometer UC
ϕ Friction Angle		Gs Specific Gravity
		RQD Rock Quality Designation

The stratification lines represent approximate boundaries. The transition may be gradual.



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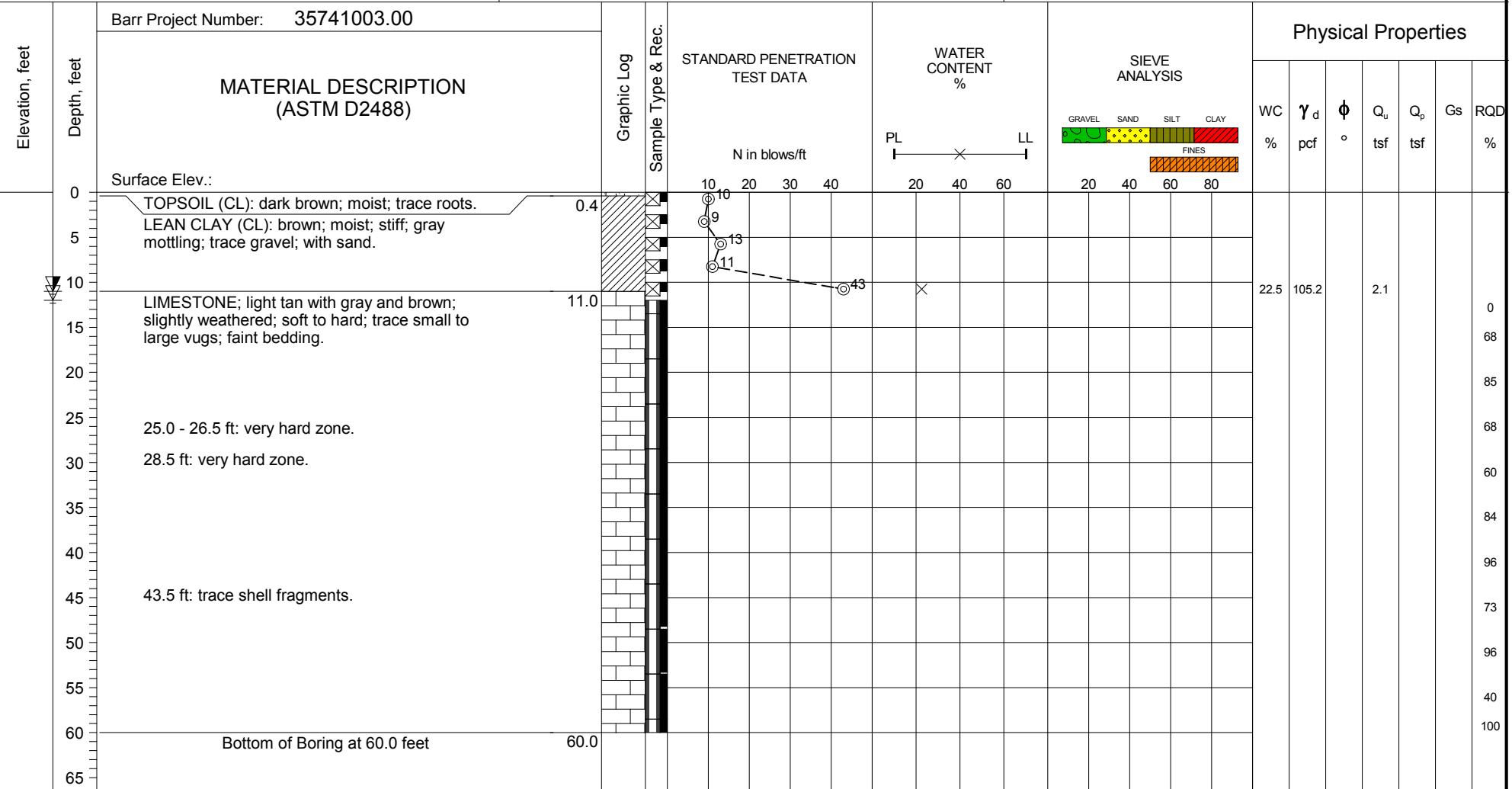
LOG OF BORING GEO-026

Sheet 1 of 1

Project: Seneca Wind Project

Location: Seneca County, Ohio

Client: sPower



Completion Depth: 60.0
Date Boring Started: 4/12/18
Date Boring Completed: 4/13/18
Logged By: BAA / BWL
Drilling Contractor: EPC Engineering and Testing
Drilling Method: HSA and NQ Core
Ground Surface Elevation:
Coordinates: UTM 17 N:4545875.5m, E:327741.3m
Datum: NAD83

Remarks:

SAMPLE TYPES



WATER LEVELS (ft)

At Time of Drilling 11.0
12 hrs After Drilling 12.0 before coring

LEGEND

MC Moisture Content Q_u Unconfined Compression
 γ Dry Unit Weight Q_p Hand Penetrometer UC
 ϕ Friction Angle Gs Specific Gravity
RQD Rock Quality Designation

The stratification lines represent approximate boundaries. The transition may be gradual.



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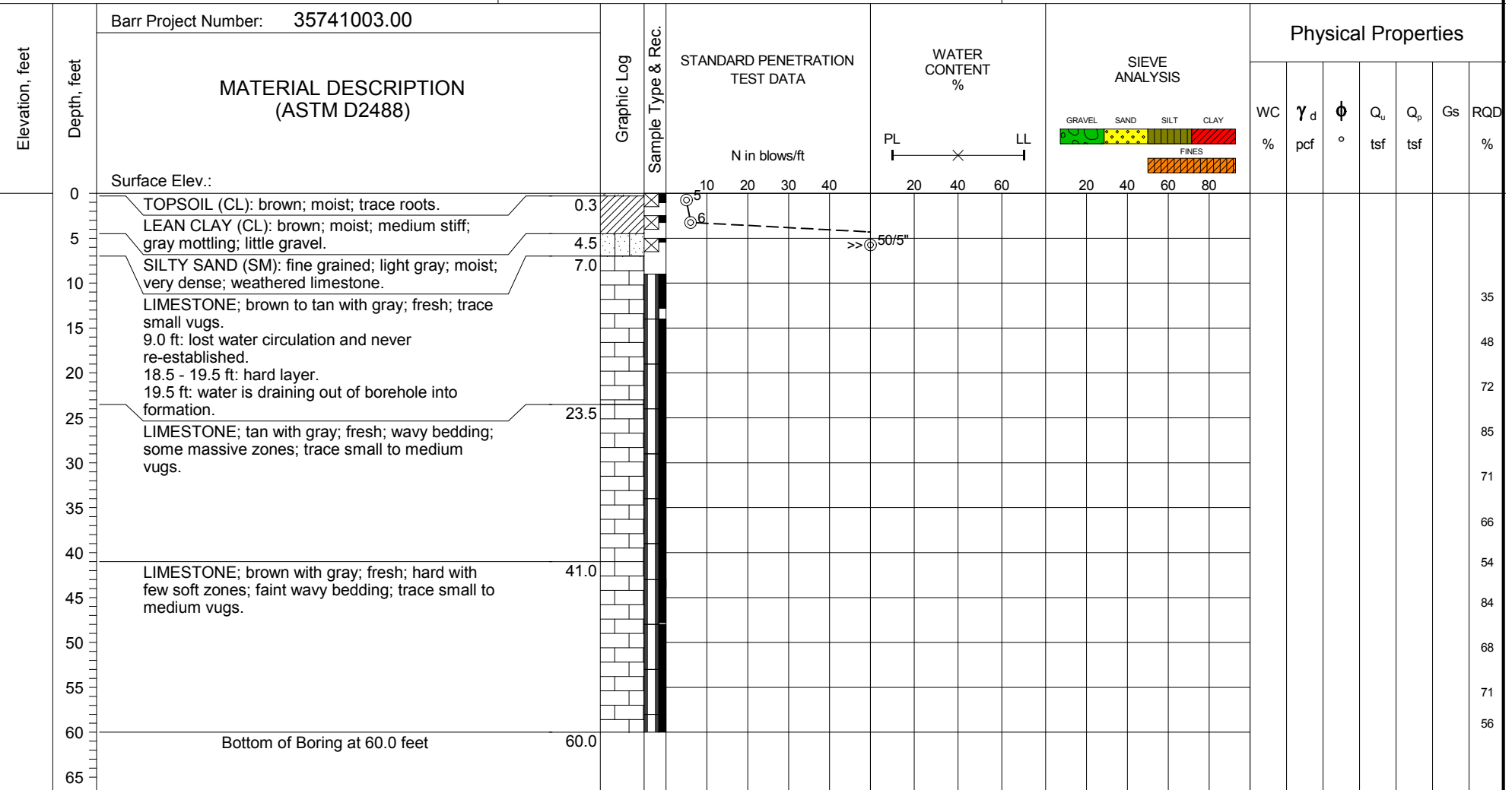
LOG OF BORING GEO-028

Sheet 1 of 1

Project: Seneca Wind Project

Location: Seneca County, Ohio

Client: sPower



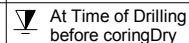
Completion Depth: 60.0
Date Boring Started: 4/12/18
Date Boring Completed: 4/13/18
Logged By: BAA / BWL
Drilling Contractor: EPC Engineering and Testing
Drilling Method: HSA and NQ Core
Ground Surface Elevation:
Coordinates: UTM 17 N:4545425.5m, E:327878.9m
Datum: NAD83

Remarks:

SAMPLE TYPES



WATER LEVELS (ft)



At Time of Drilling
before coringDry

LEGEND

MC Moisture Content
 γ Dry Unit Weight
 ϕ Friction Angle

Q_u Unconfined Compression
 Q_p Hand Penetrometer UC
Gs Specific Gravity
RQD Rock Quality Designation

The stratification lines represent approximate boundaries. The transition may be gradual.

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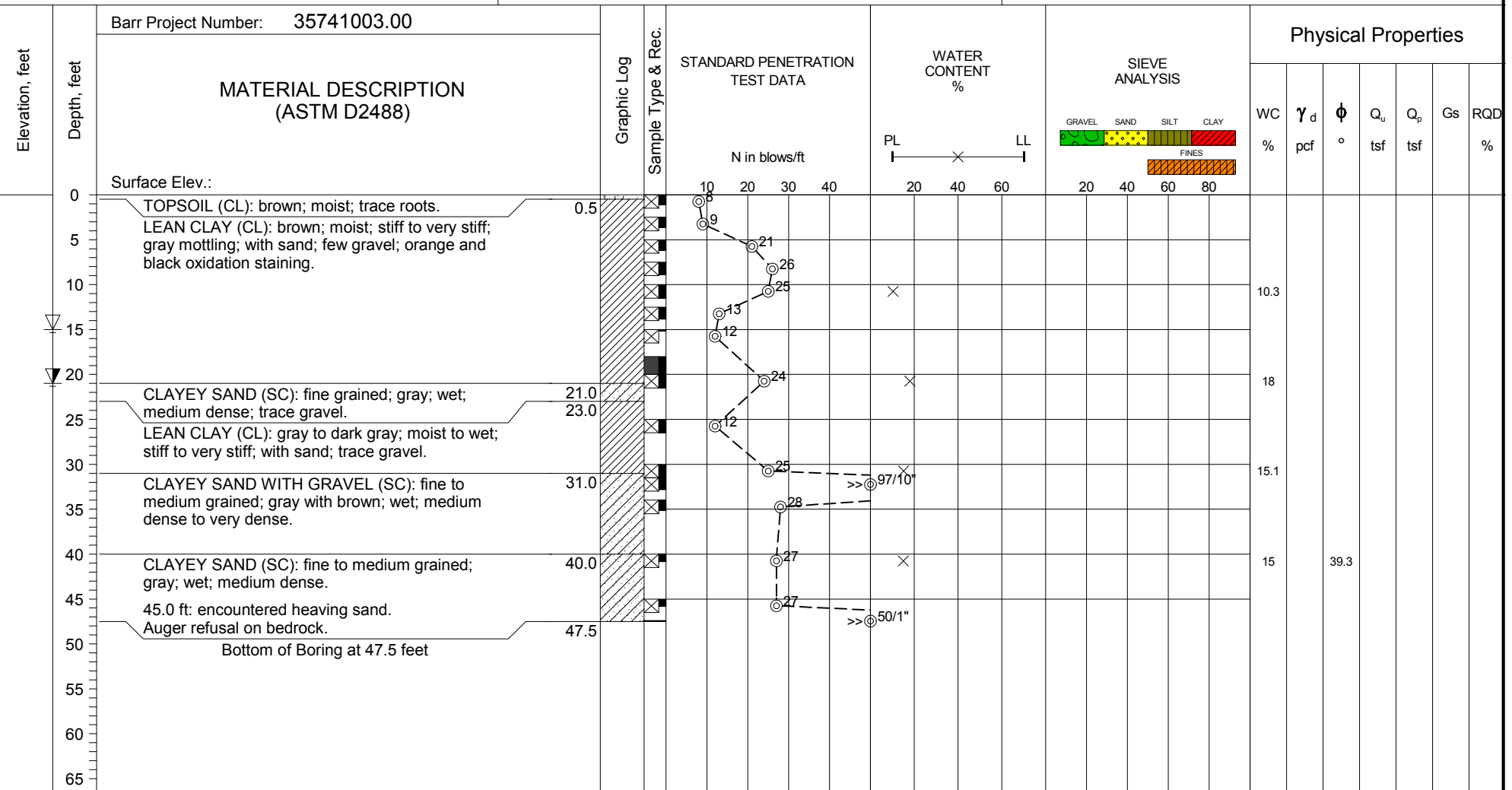
LOG OF BORING GEO-030

Sheet 1 of 1

Project: Seneca Wind Project

Location: Seneca County, Ohio

Client: sPower



Completion Depth: 47.5
Date Boring Started: 4/18/18
Date Boring Completed: 4/18/18
Logged By: DJZ
Drilling Contractor: EPC Engineering and Testing
Drilling Method: HSA
Ground Surface Elevation:
Coordinates: UTM 17 N:4547428.1m, E:327749.1m
Datum: NAD83

Remarks: Encountered heaving sand at 45 ft. Borehole offset 10 ft south and redrilled with a head of mud in augers.

SAMPLE TYPES

SPLIT SPOON 3-inch Shelby Tube

WATER LEVELS (ft)

After Drilling 15.0
 At Time of Drilling 21.0

LEGEND

MC	Moisture Content	Q_u	Unconfined Compression
γ	Dry Unit Weight	Q_p	Hand Penetrometer UC
ϕ	Friction Angle	Gs	Specific Gravity
		RQD	Rock Quality Designation

The stratification lines represent approximate boundaries. The transition may be gradual.



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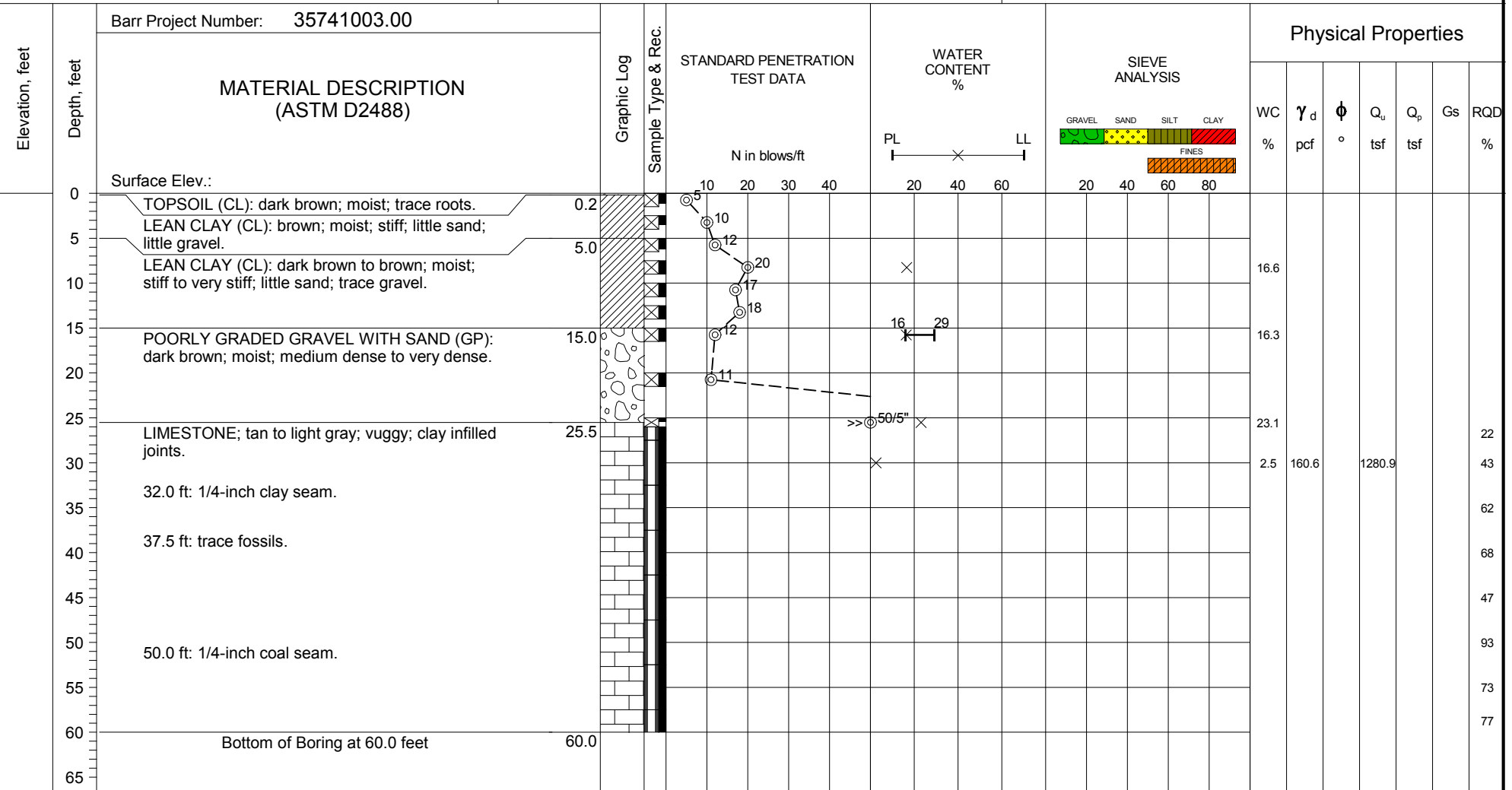
LOG OF BORING GEO-031

Sheet 1 of 1

Project: Seneca Wind Project

Location: Seneca County, Ohio

Client: sPower



Completion Depth:	60.0	Remarks:	
Date Boring Started:	4/14/18		
Date Boring Completed:	4/14/18		
Logged By:	DJZ		
Drilling Contractor:	EPC Engineering and Testing		
Drilling Method:	HSA and NQ Core		
Ground Surface Elevation:			
Coordinates:	UTM 17 N:4545328.4m, E:329526.3m		
Datum:	NAD83		

SAMPLE TYPES	WATER LEVELS (ft)	LEGEND
SPLIT SPOON	At Time of Drilling before coringDry	MC Moisture Content
ROCK CORE		γ_d Dry Unit Weight
		ϕ Friction Angle
		Q_u Unconfined Compression
		Q_p Hand Penetrometer UC
		Gs Specific Gravity
		RQD Rock Quality Designation

The stratification lines represent approximate boundaries. The transition may be gradual.



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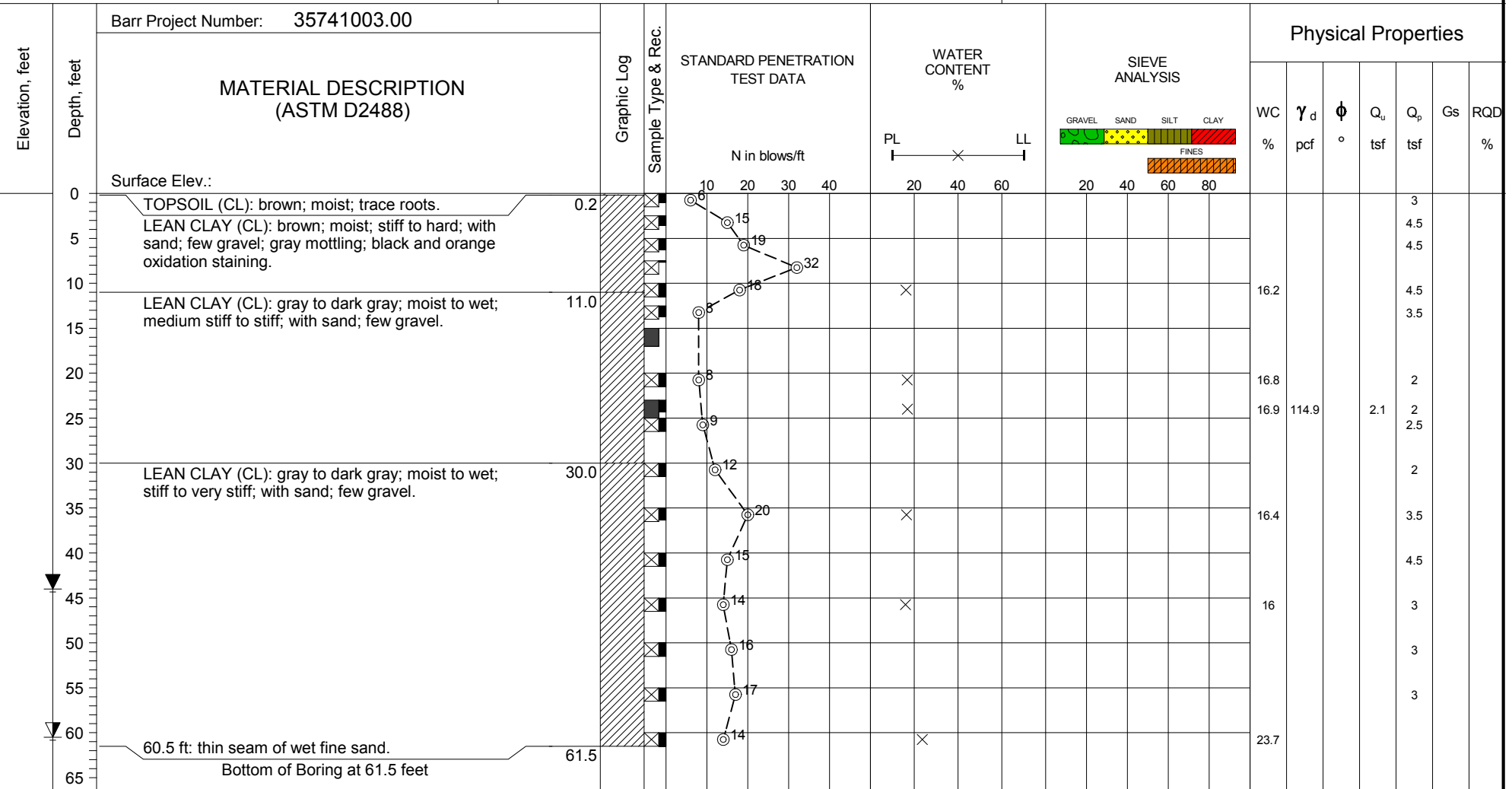
LOG OF BORING GEO-033

Sheet 1 of 1

Project: Seneca Wind Project

Location: Seneca County, Ohio

Client: sPower



Completion Depth: 61.5
Date Boring Started: 4/23/18
Date Boring Completed: 4/23/18
Logged By: DJZ
Drilling Contractor: EPC Engineering and Testing
Drilling Method: HSA
Ground Surface Elevation:
Coordinates: UTM 17 N:4551644.0m, E:337479.1m
Datum: NAD83

Remarks:

SAMPLE TYPES

SPLIT SPOON 3-inch Shelby Tube

WATER LEVELS (ft)

End of Drilling 44.0
 At Time of Drilling 60.5

LEGEND

MC	Moisture Content	Q_u	Unconfined Compression
γ	Dry Unit Weight	Q_p	Hand Penetrometer UC
ϕ	Friction Angle	Gs	Specific Gravity
		RQD	Rock Quality Designation





The stratification lines represent approximate boundaries. The transition may be gradual.



Sheet 1 of 1

Client: sPower

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SAMPLE TYPES		WATER LEVELS (ft)		LEGEND	
 SPLIT SPOON  3-inch Shelby Tube	 End of Drilling	30.0	MC	Moisture Content	Q _u Unconfined Compression
	 At Time of Drilling	20.0	γ	Dry Unit Weight	Q _p Hand Penetrometer UC
			φ	Friction Angle	G _s Specific Gravity
					RQD Rock Quality Designation

The stratification lines represent approximate boundaries. The transition may be gradual.



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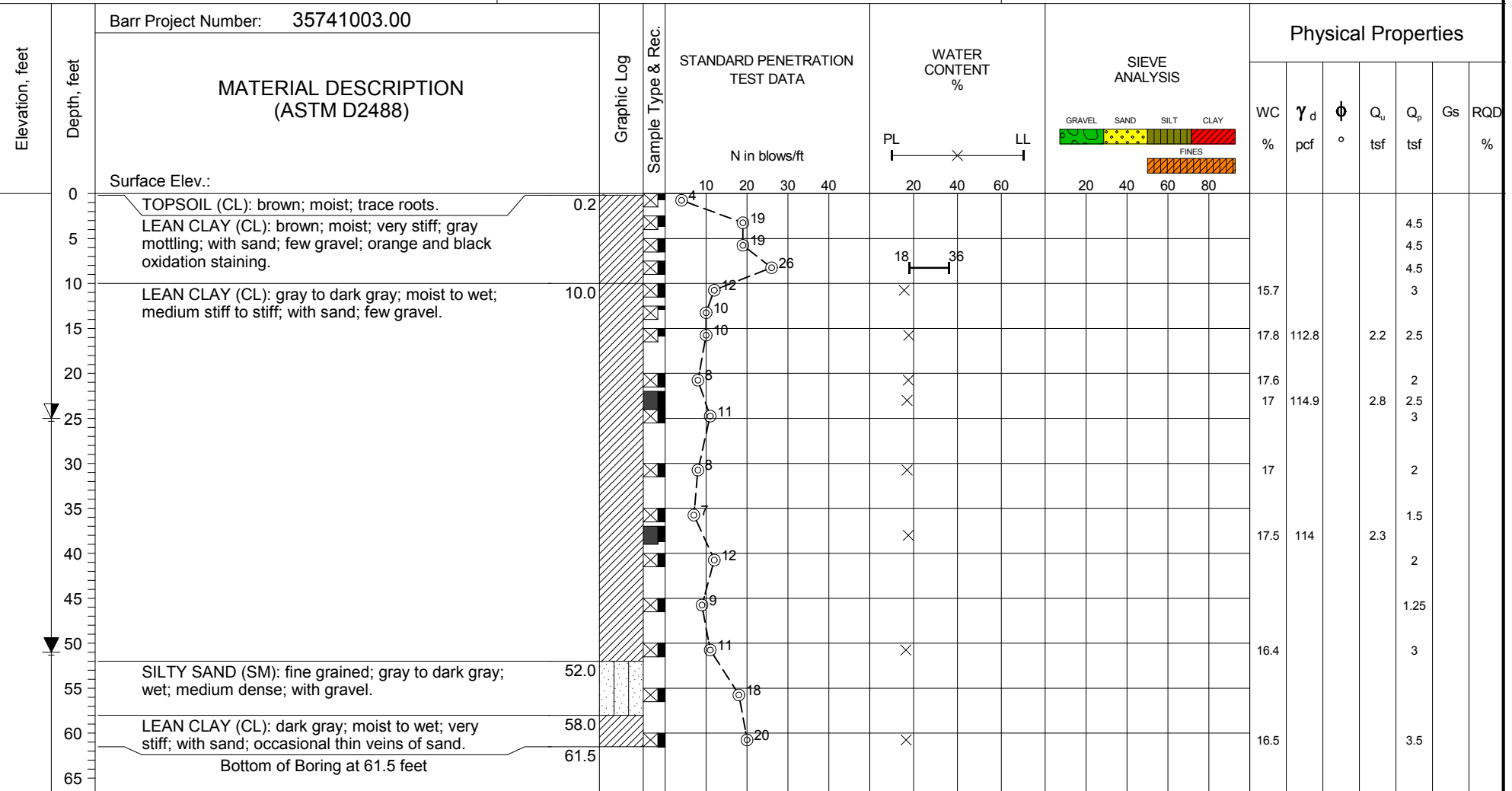
LOG OF BORING GEO-035

Sheet 1 of 1

Project: Seneca Wind Project

Location: Seneca County, Ohio

Client: sPower



Completion Depth: 61.5
Date Boring Started: 5/2/18
Date Boring Completed: 5/2/18
Logged By: DJZ
Drilling Contractor: EPC Engineering and Testing
Drilling Method: HSA
Ground Surface Elevation: UTM 17 N:4550629.4m, E:338124.3m
Coordinates: UTM 17 N:4550629.4m, E:338124.3m
Datum: NAD83

Remarks:

SAMPLE TYPES

⊠ SPLIT SPOON 3-inch Shelby Tube

WATER LEVELS (ft)

▼ End of Drilling 51.0
▼ At Time of Drilling 25.0

LEGEND

MC Moisture Content Q_u Unconfined Compression
 γ Dry Unit Weight Q_p Hand Penetrometer UC
 ϕ Friction Angle Gs Specific Gravity
RQD Rock Quality Designation

The stratification lines represent approximate boundaries. The transition may be gradual.



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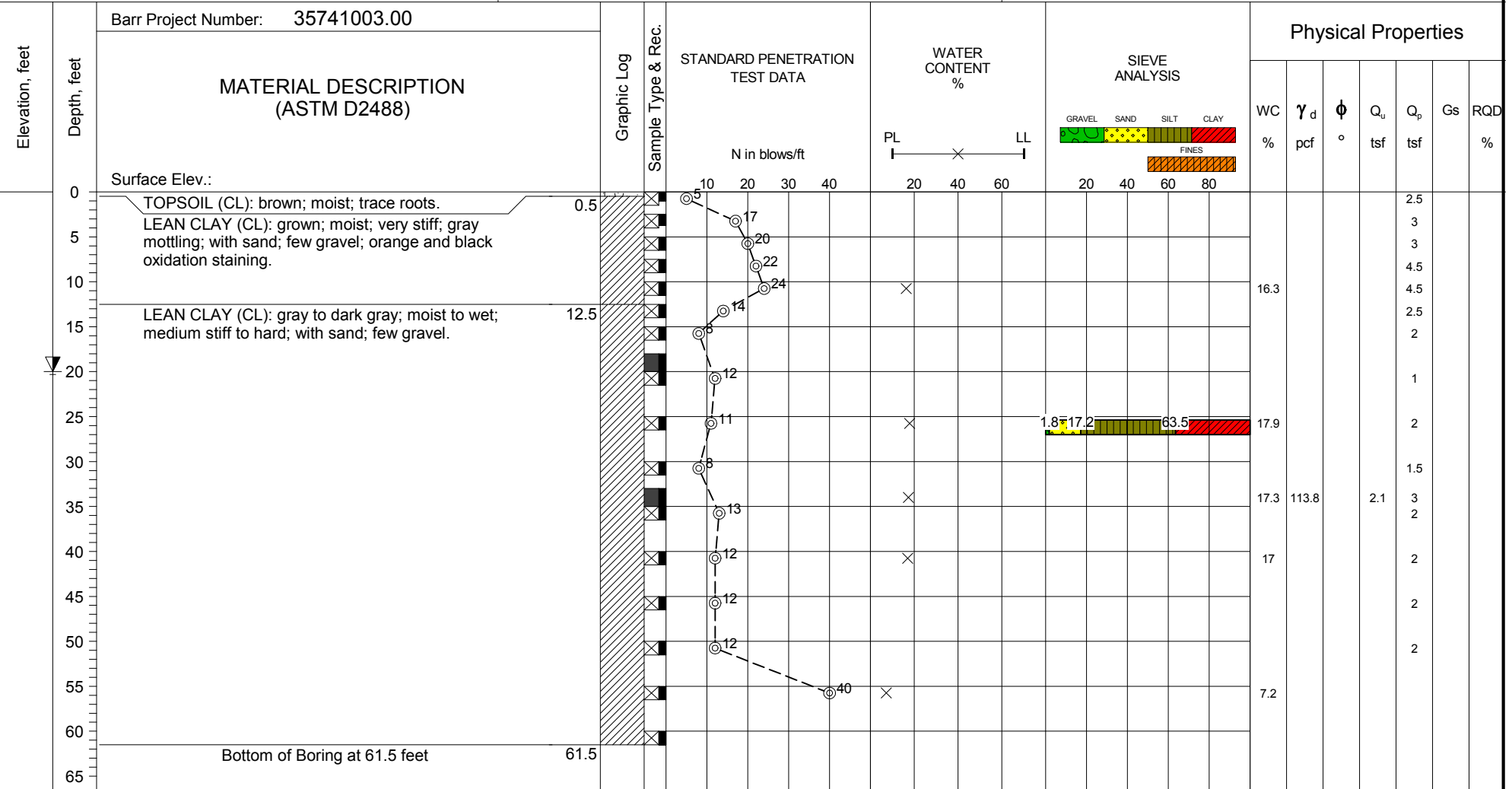
LOG OF BORING GEO-036

Sheet 1 of 1

Project: Seneca Wind Project

Location: Seneca County, Ohio

Client: sPower



Completion Depth: 61.5
Date Boring Started: 4/24/18
Date Boring Completed: 4/24/18
Logged By: DJZ
Drilling Contractor: EPC Engineering and Testing
Drilling Method: HSA
Ground Surface Elevation:
Coordinates: UTM 17 N:4550103.1m, E:338202.1m
Datum: NAD83

Remarks:

SAMPLE TYPES

⊠ SPLIT SPOON 3-inch Shelby Tube

WATER LEVELS (ft)

▼ End of Drilling
▼ Dry
▼ At Time of Drilling 20.0

LEGEND

MC Moisture Content Q_u Unconfined Compression
 γ Dry Unit Weight Q_p Hand Penetrometer UC
 ϕ Friction Angle Gs Specific Gravity
RQD Rock Quality Designation

The stratification lines represent approximate boundaries. The transition may be gradual.



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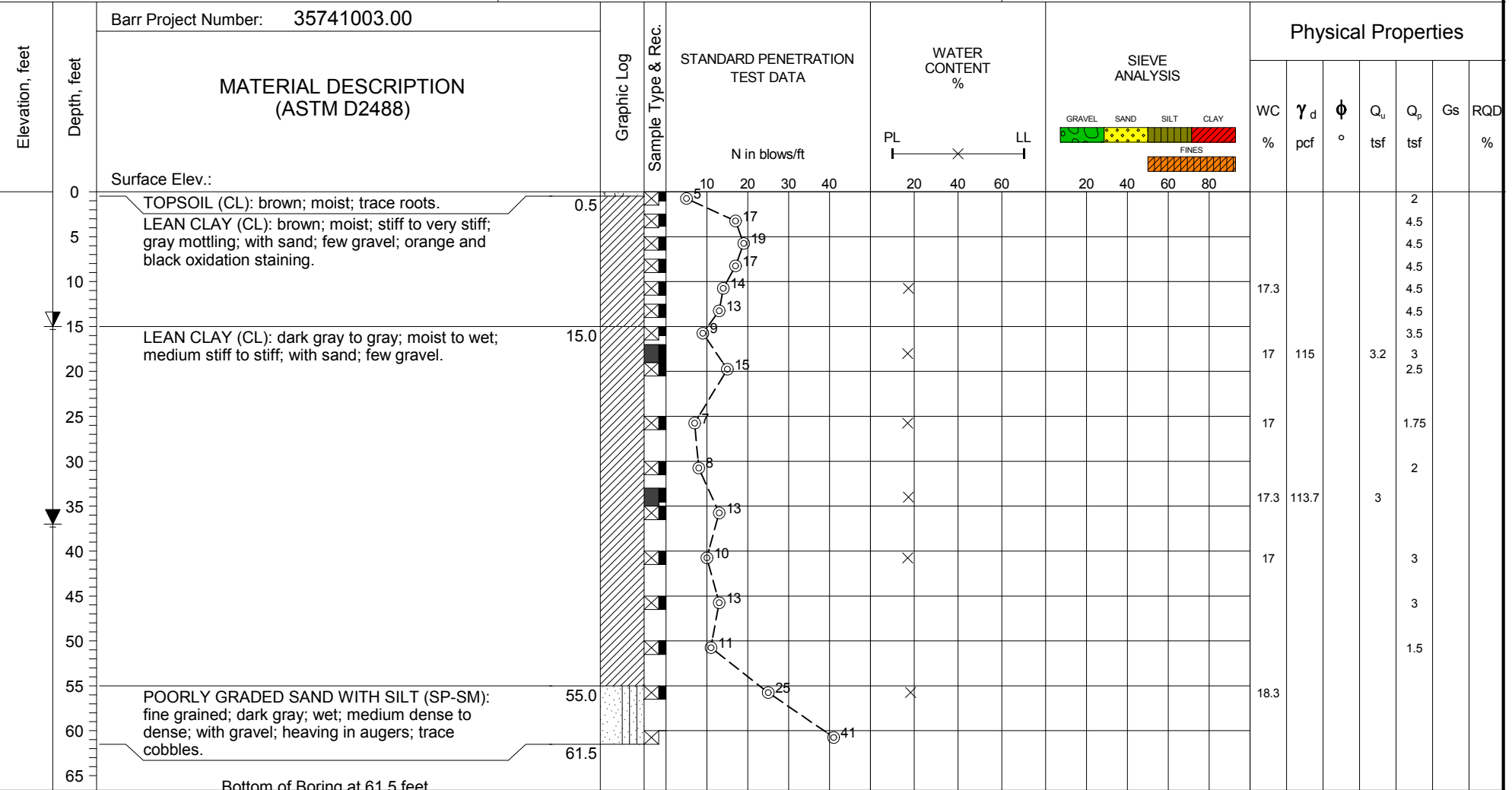
LOG OF BORING GEO-037

Sheet 1 of 1

Project: Seneca Wind Project

Location: Seneca County, Ohio

Client: sPower



Completion Depth: 61.5
Date Boring Started: 4/25/18
Date Boring Completed: 4/25/18
Logged By: DJZ
Drilling Contractor: EPC Engineering and Testing
Drilling Method: HSA
Ground Surface Elevation:
Coordinates: UTM 17 N:4549927.6m, E:338928.3m
Datum: NAD83

Remarks:

SAMPLE TYPES

⊠ SPLIT SPOON 3-inch Shelby Tube

WATER LEVELS (ft)

▼ End of Drilling 37.0
▼ At Time of Drilling 15.0

LEGEND

MC Moisture Content Q_u Unconfined Compression
 γ Dry Unit Weight Q_p Hand Penetrometer UC
 ϕ Friction Angle Gs Specific Gravity
RQD Rock Quality Designation

The stratification lines represent approximate boundaries. The transition may be gradual.



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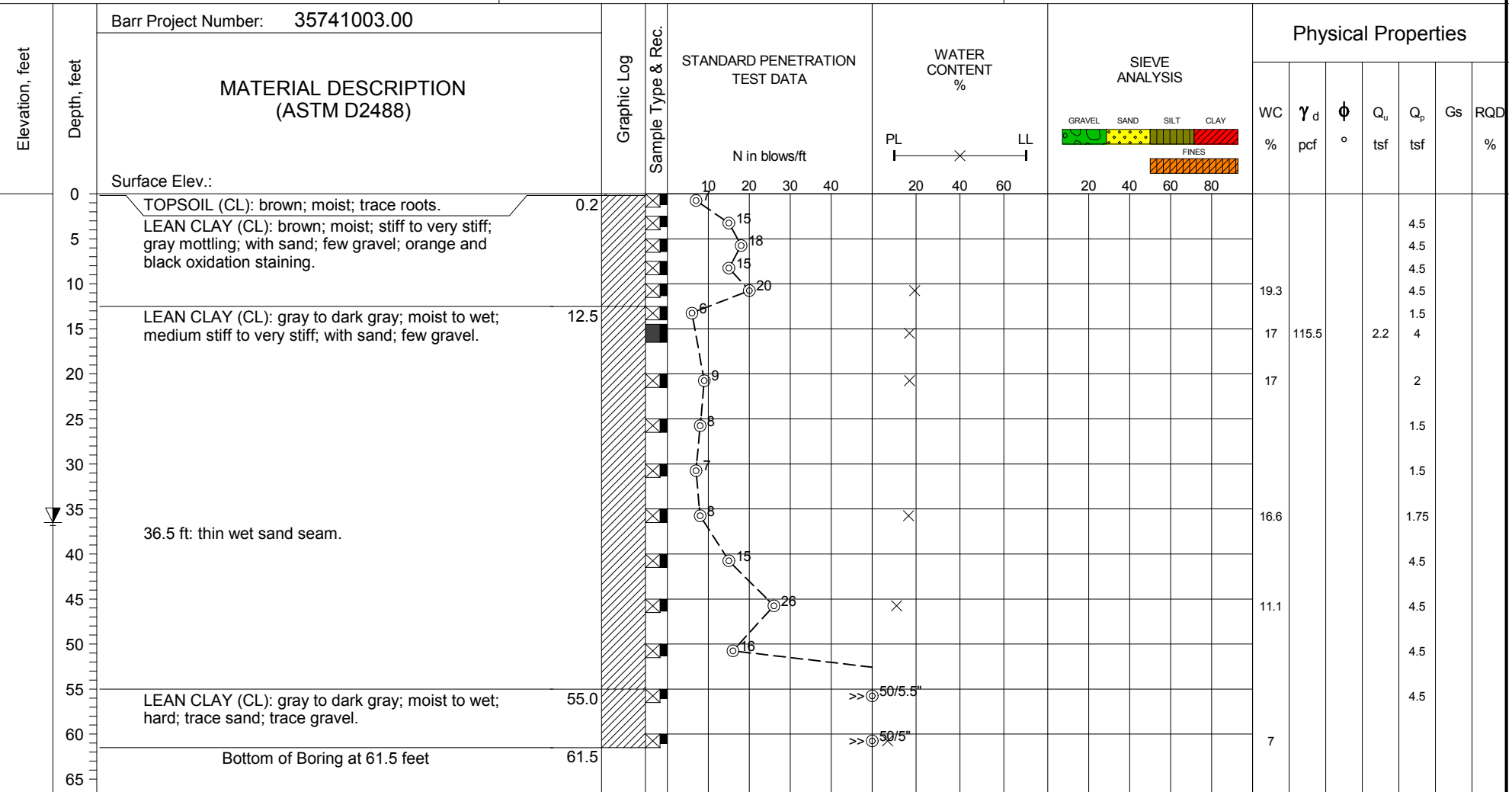
LOG OF BORING GEO-038

Sheet 1 of 1





Project: Seneca Wind Project

Location: Seneca County, Ohio

Client: sPower



Completion Depth:	61.5	Remarks:			
Date Boring Started:	4/25/18				
Date Boring Completed:	4/25/18				
Logged By:	DJZ				
Drilling Contractor:	EPC Engineering and Testing				
Drilling Method:	HSA				
Ground Surface Elevation:					
Coordinates:	UTM 17 N:4549899.6m, E:339979.5m				
Datum:	NAD83				

SAMPLE TYPES		WATER LEVELS (ft)		LEGEND	
 SPLIT SPOON	 3-inch Shelby Tube	 End of Drilling Dry		MC Moisture Content	Q_u Unconfined Compression
		 At Time of Drilling	36.5	γ Dry Unit Weight	Q_p Hand Penetrometer UC
				ϕ Friction Angle	Gs Specific Gravity
					RQD Rock Quality Designation

The stratification lines represent approximate boundaries. The transition may be gradual.



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LOG OF BORING GEO-039

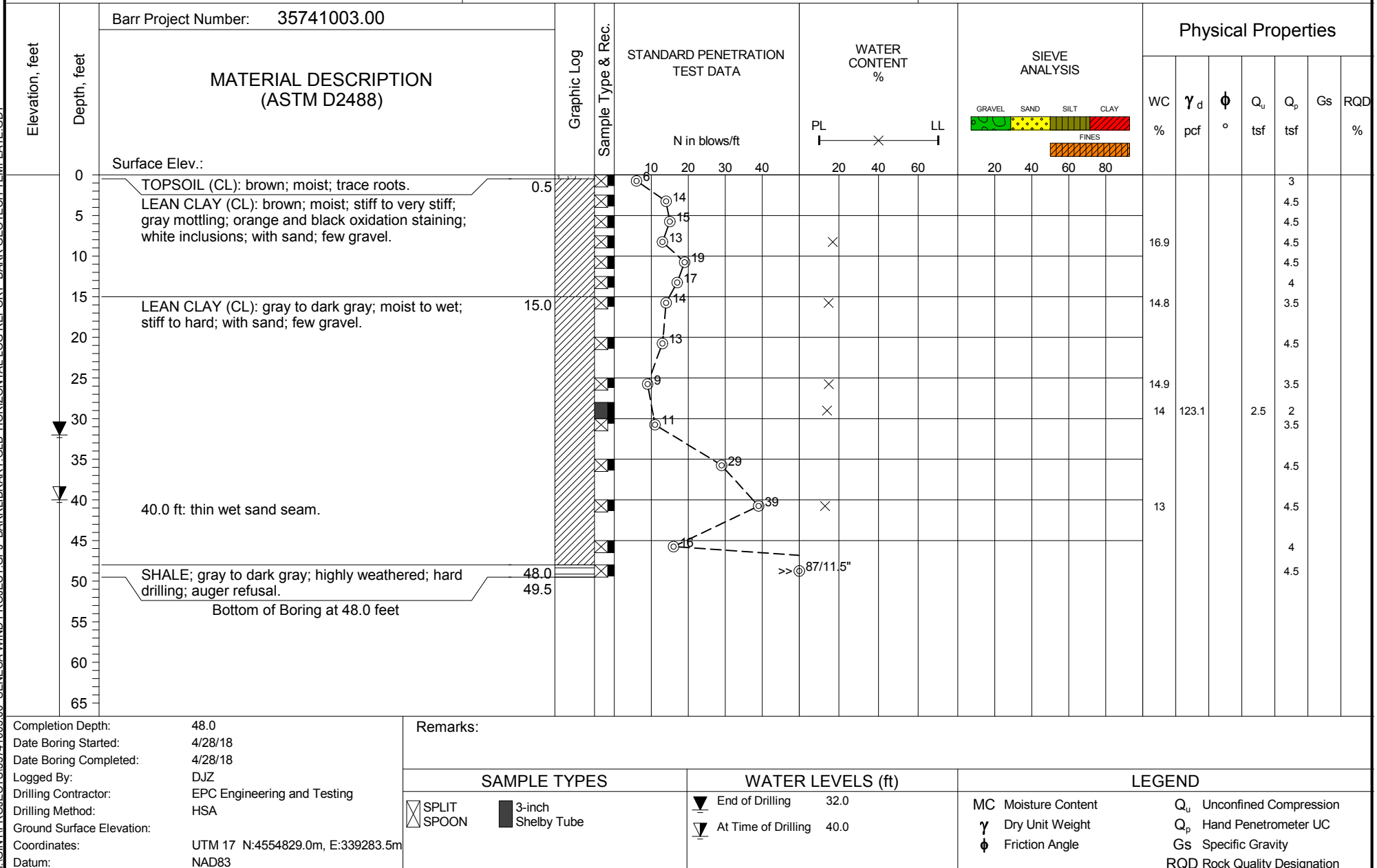
Sheet 1 of 1

Project: Seneca Wind Project

Location: Seneca County, Ohio

Client: sPower

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The stratification lines represent approximate boundaries. The transition may be gradual.



Sheet 1 of 1

Client: sPower



MC	Moisture Content	Q_u	Unconfined Compression
γ	Dry Unit Weight	Q_p	Hand Penetrometer UC
ϕ	Friction Angle	Gs	Specific Gravity
		RQD	Rock Quality Designation

The stratification lines represent approximate boundaries. The transition may be gradual.



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LOG OF BORING GEO-041

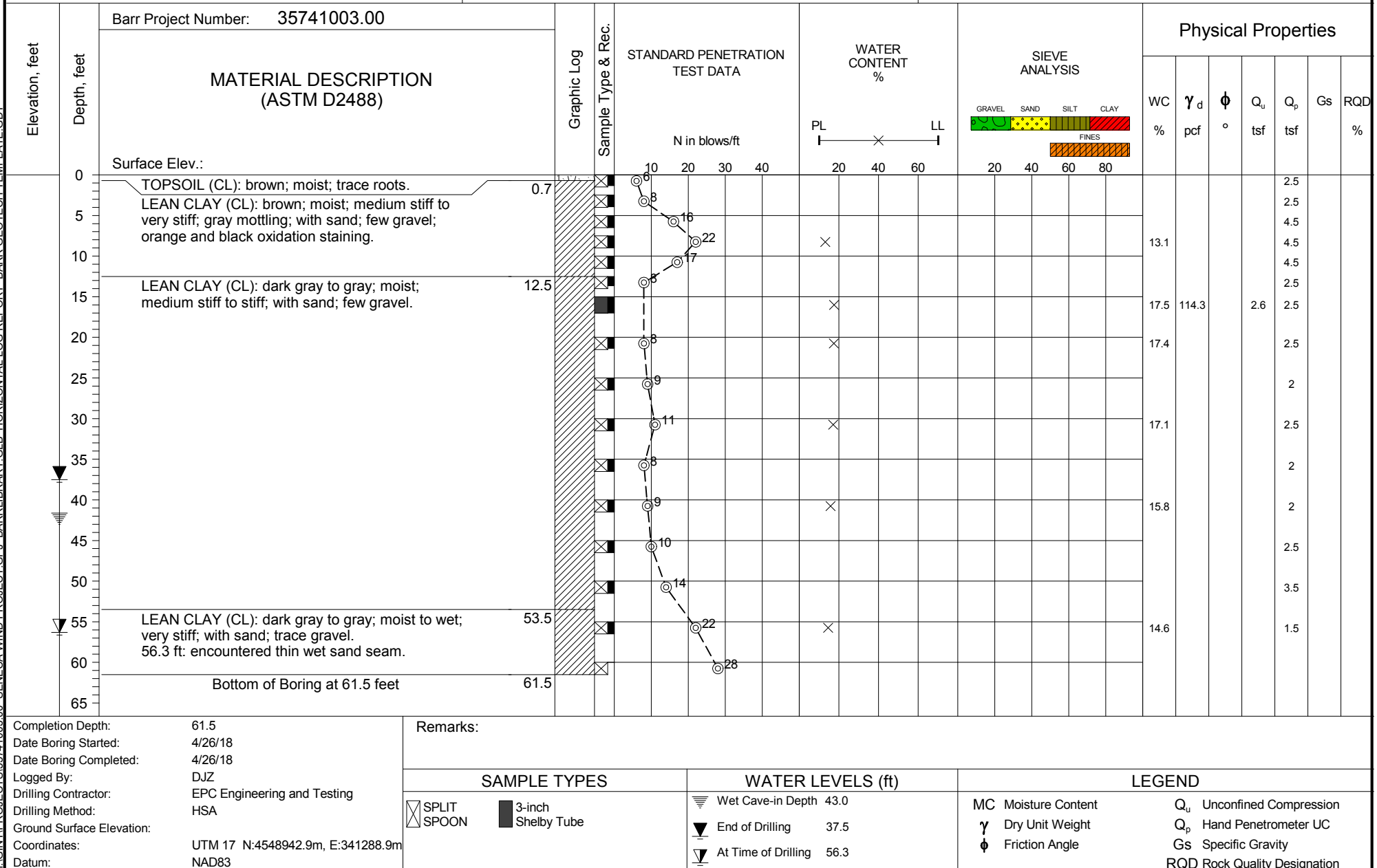
Sheet 1 of 1

Project: Seneca Wind Project

Location: Seneca County, Ohio

Client: sPower

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The stratification lines represent approximate boundaries. The transition may be gradual.



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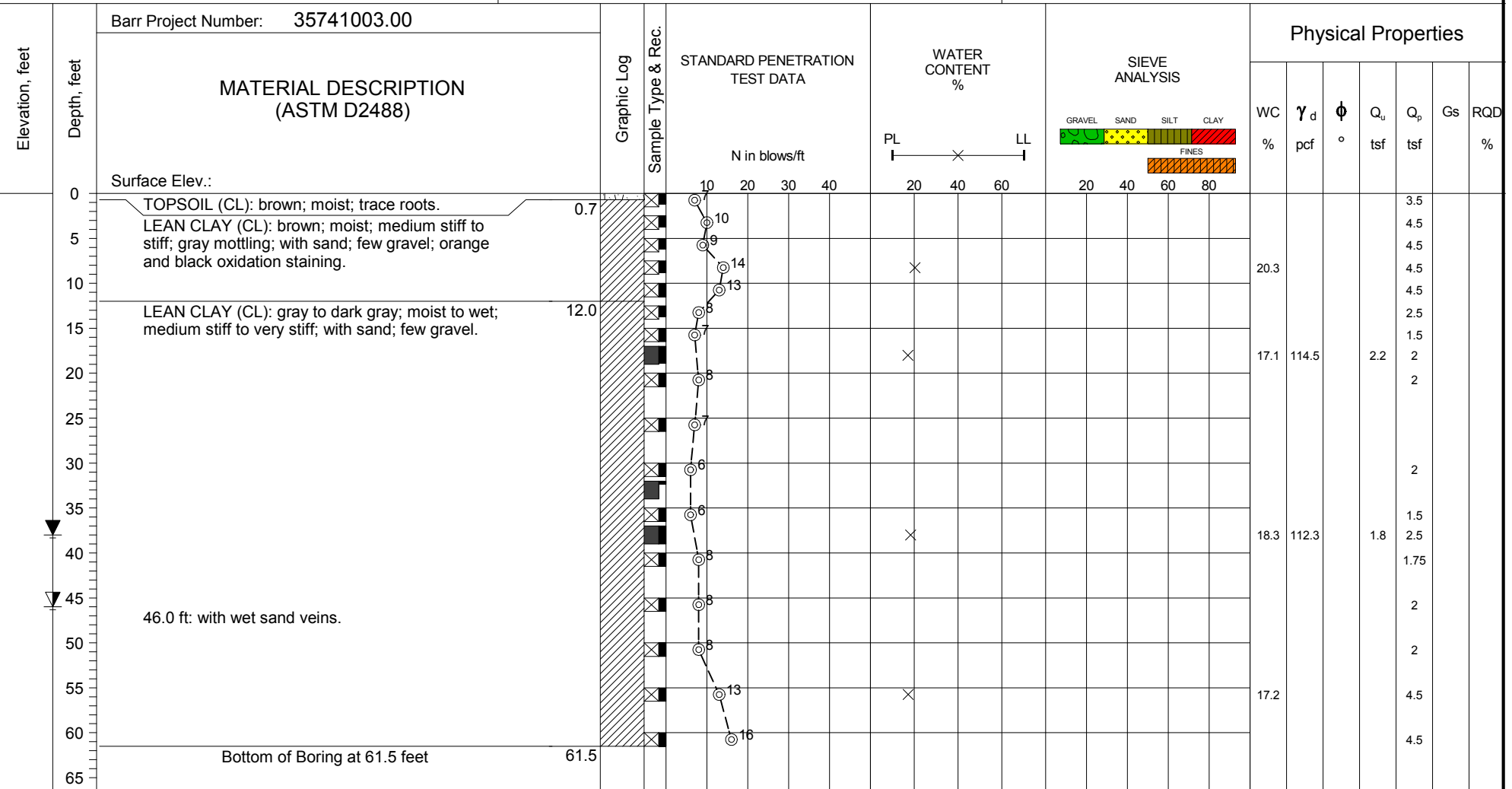
LOG OF BORING GEO-042

Sheet 1 of 1

Project: Seneca Wind Project

Location: Seneca County, Ohio

Client: sPower



Completion Depth: 61.5
Date Boring Started: 5/8/18
Date Boring Completed: 5/8/18
Logged By: DJZ
Drilling Contractor: EPC Engineering and Testing
Drilling Method: HSA
Ground Surface Elevation:
Coordinates: UTM 17 N:4548991.7m, E:344231.2m
Datum: NAD83

Remarks:

SAMPLE TYPES

⊠ SPLIT SPOON 3-inch Shelby Tube

WATER LEVELS (ft)

▼ End of Drilling 38.0
▼ At Time of Drilling 46.0

LEGEND

MC	Moisture Content	Q_u	Unconfined Compression
γ	Dry Unit Weight	Q_p	Hand Penetrometer UC
ϕ	Friction Angle	Gs	Specific Gravity
		RQD	Rock Quality Designation

The stratification lines represent approximate boundaries. The transition may be gradual.



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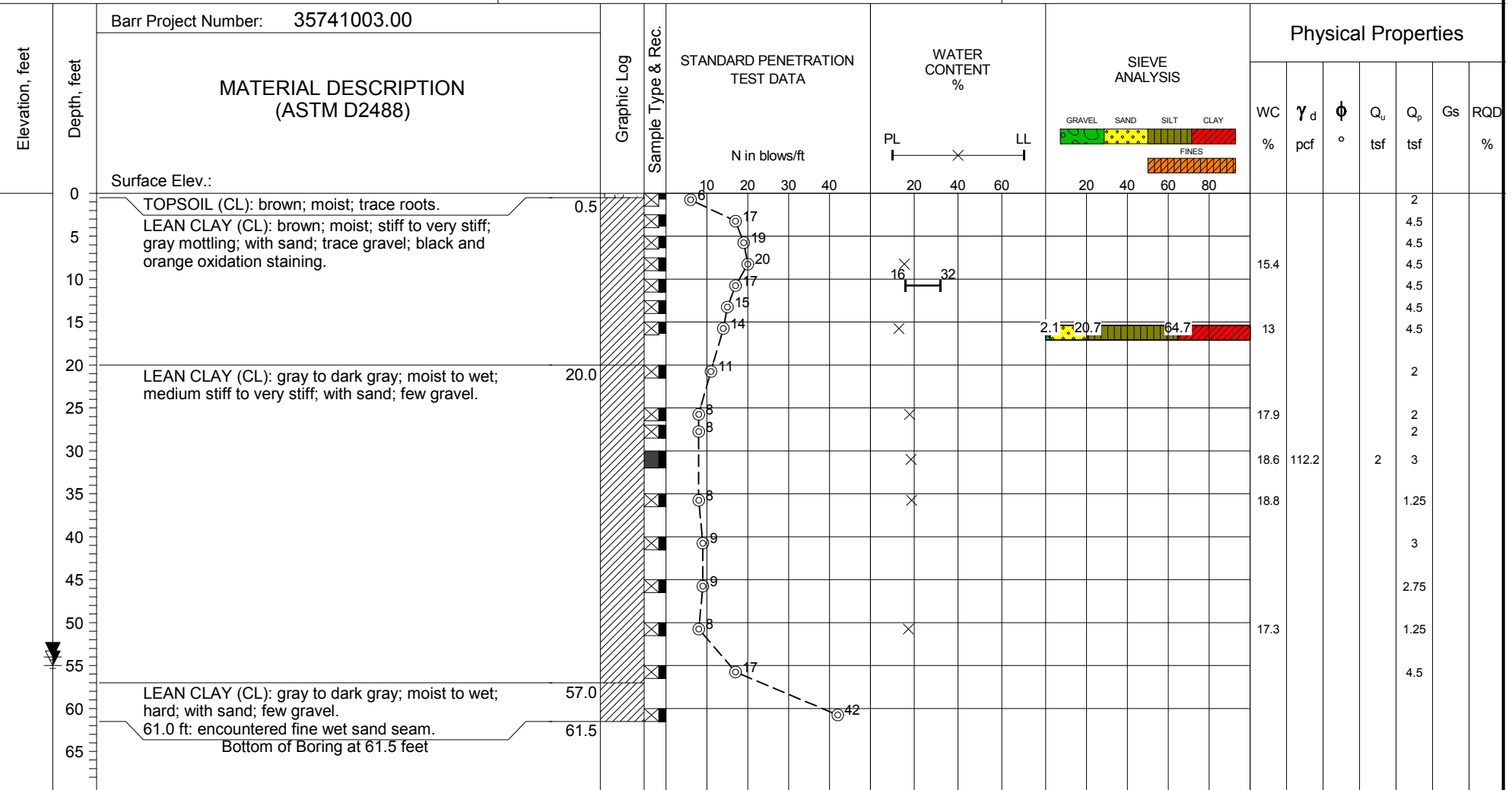
LOG OF BORING GEO-043

Sheet 1 of 1

Project: Seneca Wind Project

Location: Seneca County, Ohio

Client: sPower



Completion Depth: 61.5
Date Boring Started: 4/29/18
Date Boring Completed: 4/29/18
Logged By: DJZ
Drilling Contractor: EPC Engineering and Testing
Drilling Method: HSA
Ground Surface Elevation:
Coordinates: UTM 17 N:4548203.7m, E:343986.5m
Datum: NAD83

Remarks:

SAMPLE TYPES

⊠ SPLIT SPOON 3-inch Shelby Tube

WATER LEVELS (ft)

▼ End of Drilling 54.0
▼ At Time of Drilling 55.0

LEGEND

MC Moisture Content
 γ Dry Unit Weight
 ϕ Friction Angle
 Q_u Unconfined Compression
 Q_p Hand Penetrometer UC
Gs Specific Gravity
RQD Rock Quality Designation

The stratification lines represent approximate boundaries. The transition may be gradual.

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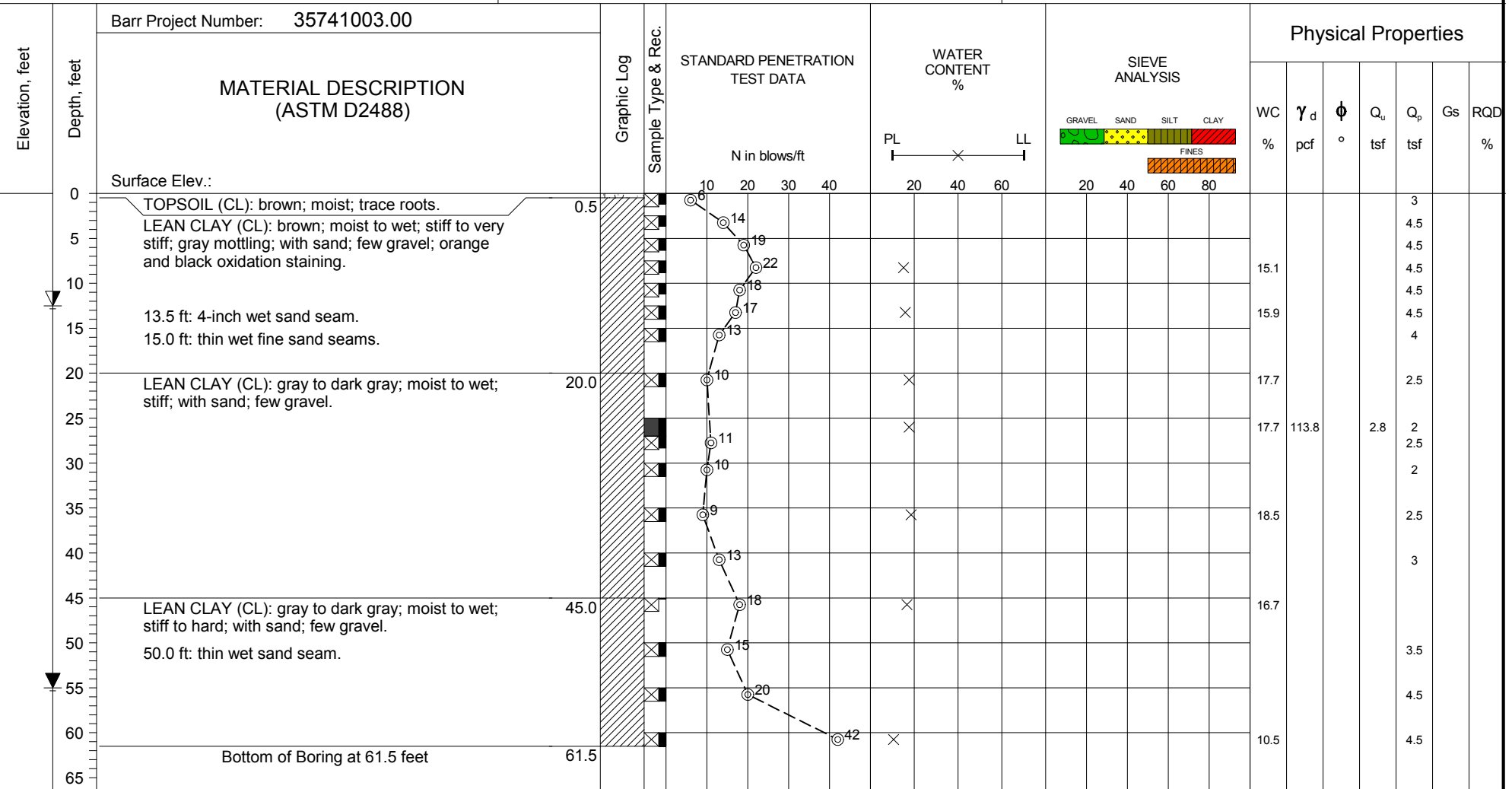
LOG OF BORING GEO-044

Sheet 1 of 1

Project: Seneca Wind Project

Location: Seneca County, Ohio

Client: sPower



Completion Depth: 61.5
Date Boring Started: 4/29/18
Date Boring Completed: 4/29/18
Logged By: DJZ
Drilling Contractor: EPC Engineering and Testing
Drilling Method: HSA
Ground Surface Elevation:
Coordinates: UTM 17 N:4548181.0m, E:344107.0m
Datum: NAD83

Remarks:

SAMPLE TYPES

⊠ SPLIT SPOON 3-inch Shelby Tube

WATER LEVELS (ft)

▼ End of Drilling 55.0
▼ At Time of Drilling 12.5

LEGEND

MC Moisture Content Q_u Unconfined Compression
 γ Dry Unit Weight Q_p Hand Penetrometer UC
 ϕ Friction Angle Gs Specific Gravity
RQD Rock Quality Designation

The stratification lines represent approximate boundaries. The transition may be gradual.

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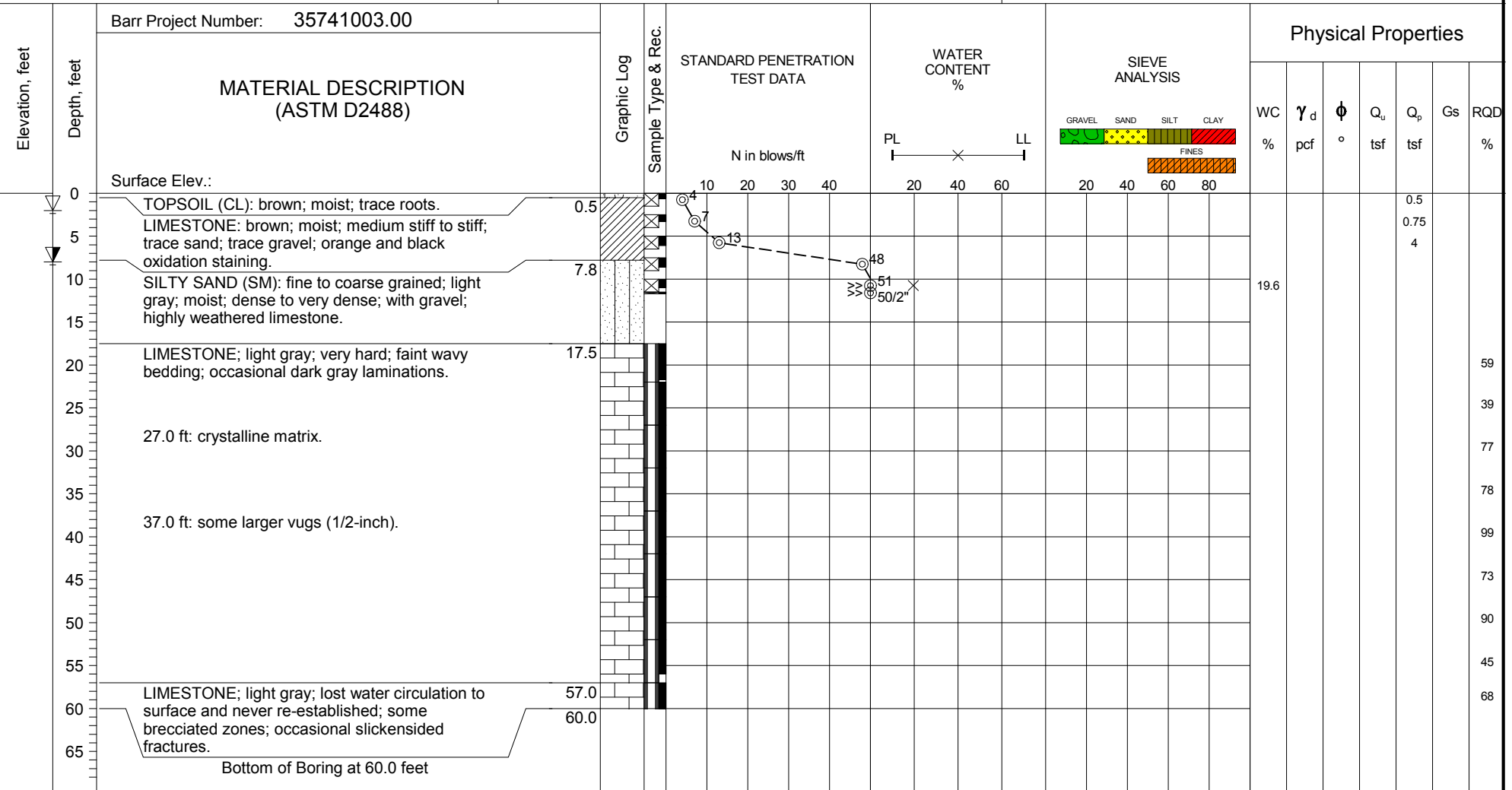
LOG OF BORING GEO-045

Sheet 1 of 1

Project: Seneca Wind Project

Location: Seneca County, Ohio

Client: sPower



Completion Depth: 60.0
Date Boring Started: 4/4/18
Date Boring Completed: 4/5/18
Logged By: BWL
Drilling Contractor: EPC Engineering and Testing
Drilling Method: HSA and NQ Core
Ground Surface Elevation: UTM 17 N:4542927.6m, E:326484.0m
Coordinates: UTM 17 N:4542927.6m, E:326484.0m
Datum: NAD83

Remarks: Several small sinkholes 1-5 ft diameter and 1-2 feet deep observed 300 feet southeast of boring location in waterway.

SAMPLE TYPES

⊠ SPLIT SPOON █ ROCK CORE

WATER LEVELS (ft)

▼ At Time of Drilling 8.0
▽ 14 hrs After Drilling 2.0

LEGEND

MC Moisture Content Q_u Unconfined Compression
 γ Dry Unit Weight Q_p Hand Penetrometer UC
 ϕ Friction Angle Gs Specific Gravity
RQD Rock Quality Designation

The stratification lines represent approximate boundaries. The transition may be gradual.



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LOG OF BORING GEO-047

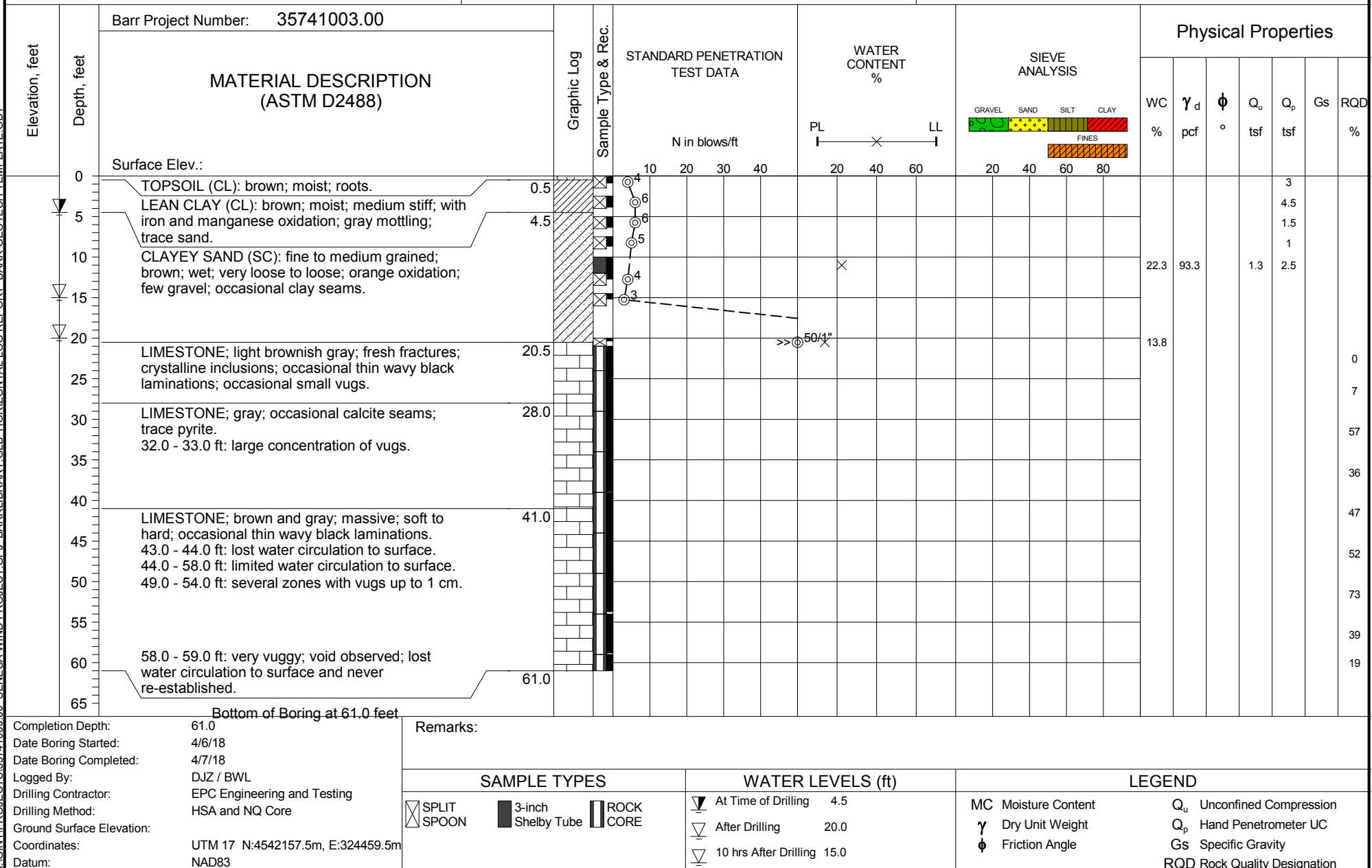
Sheet 1 of 1

Project: Seneca Wind Project

Location: Seneca County, Ohio

Client: sPower

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The stratification lines represent approximate boundaries. The transition may be gradual.



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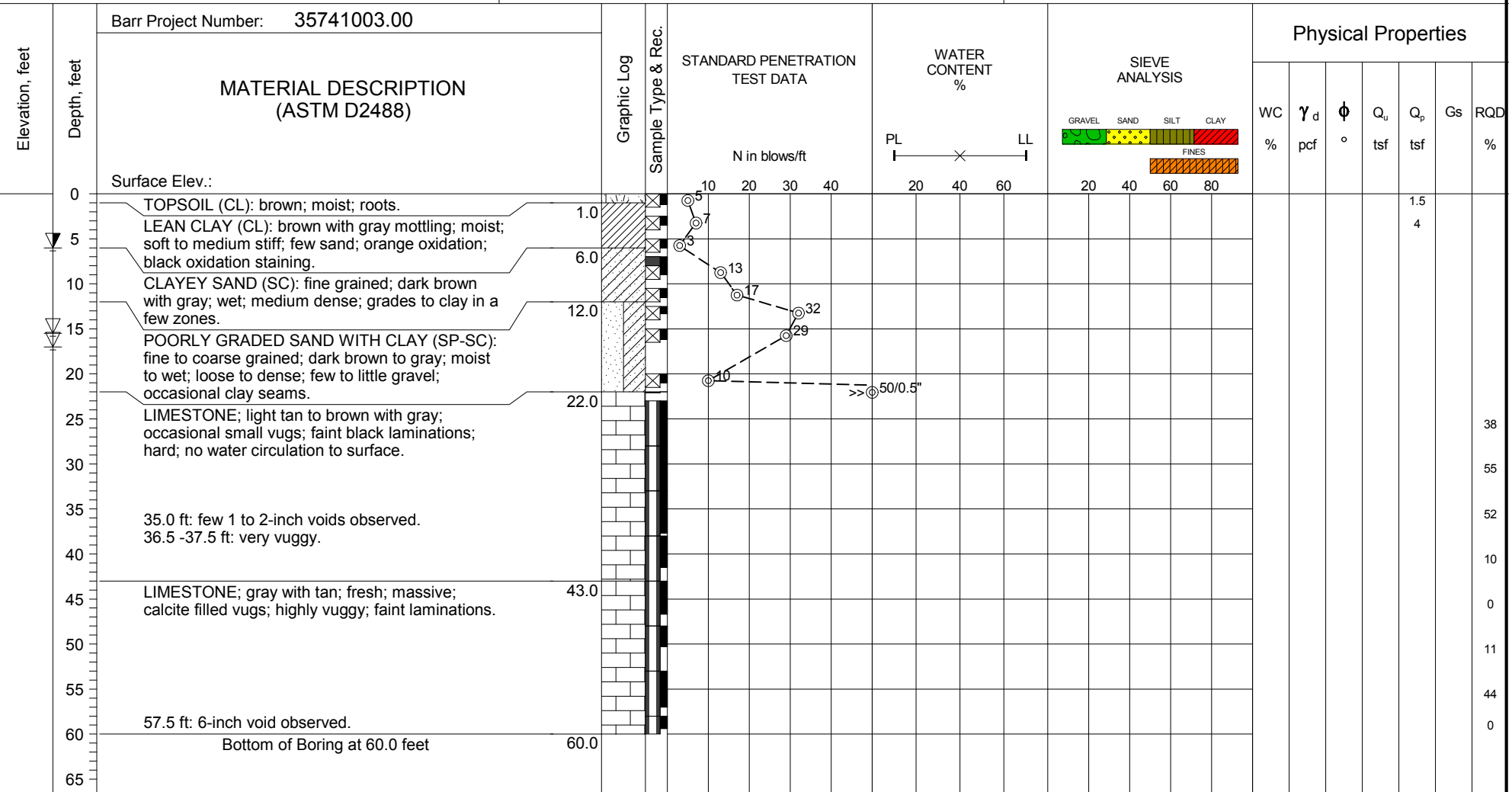
LOG OF BORING GEO-048

Sheet 1 of 1

Project: Seneca Wind Project

Location: Seneca County, Ohio

Client: sPower



Completion Depth: 60.0
Date Boring Started: 4/7/18
Date Boring Completed: 4/8/18
Logged By: DJZ / BWL
Drilling Contractor: EPC Engineering and Testing
Drilling Method: HSA and NQ Core
Ground Surface Elevation:
Coordinates: UTM 17 N:4542147.8m, E:324163.3m
Datum: NAD83

Remarks:

SAMPLE TYPES

SPLIT SPOON 3-inch Shelby Tube ROCK CORE

WATER LEVELS (ft)

At Time of Drilling 6.0
 After Drilling 15.5
 12 hrs After Drilling 17.0

LEGEND

MC Moisture Content Q_u Unconfined Compression
 γ Dry Unit Weight Q_p Hand Penetrometer UC
 ϕ Friction Angle Gs Specific Gravity
RQD Rock Quality Designation

The stratification lines represent approximate boundaries. The transition may be gradual.



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LOG OF BORING GEO-049

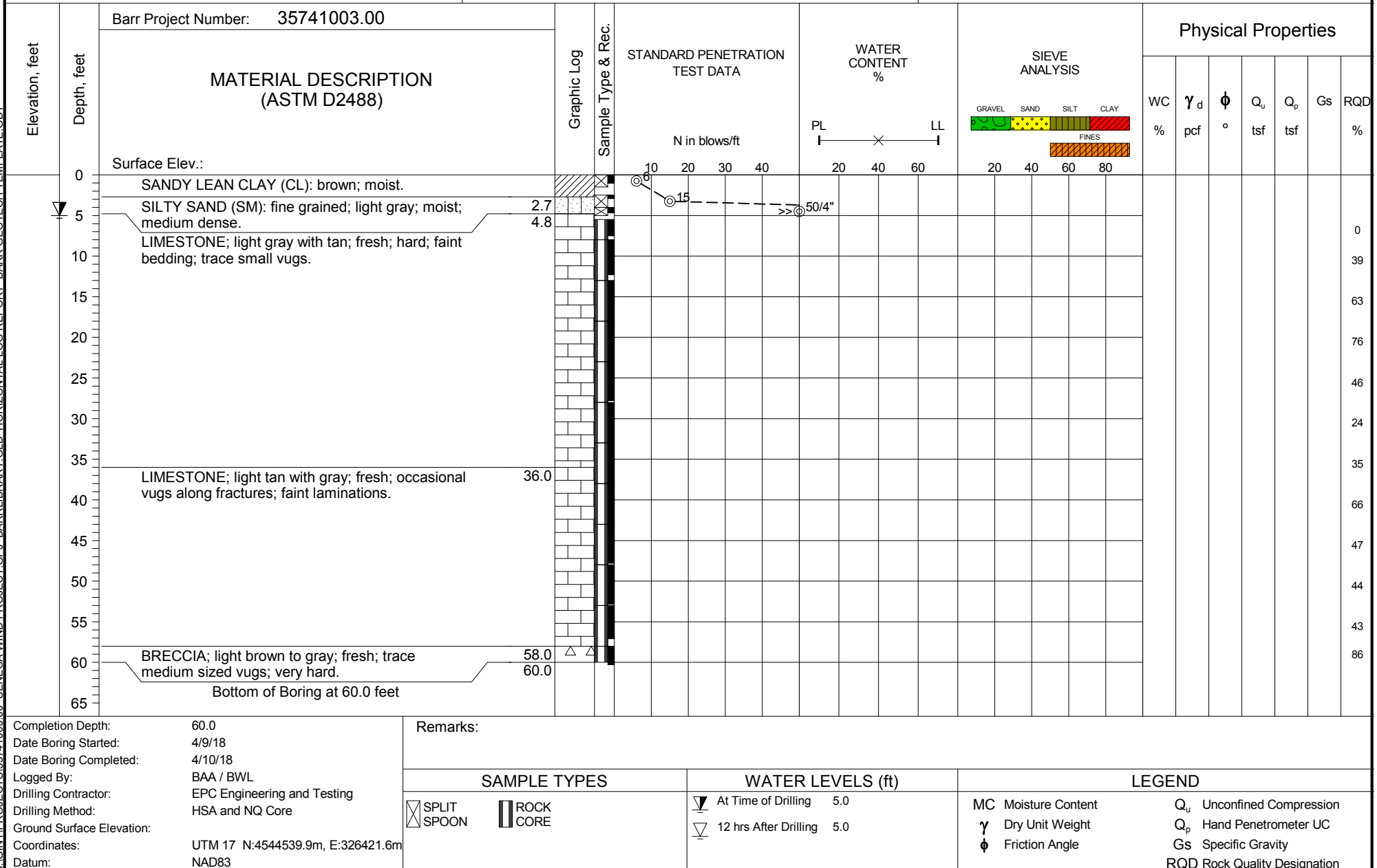
Sheet 1 of 1

Project: Seneca Wind Project

Location: Seneca County, Ohio

Client: sPower

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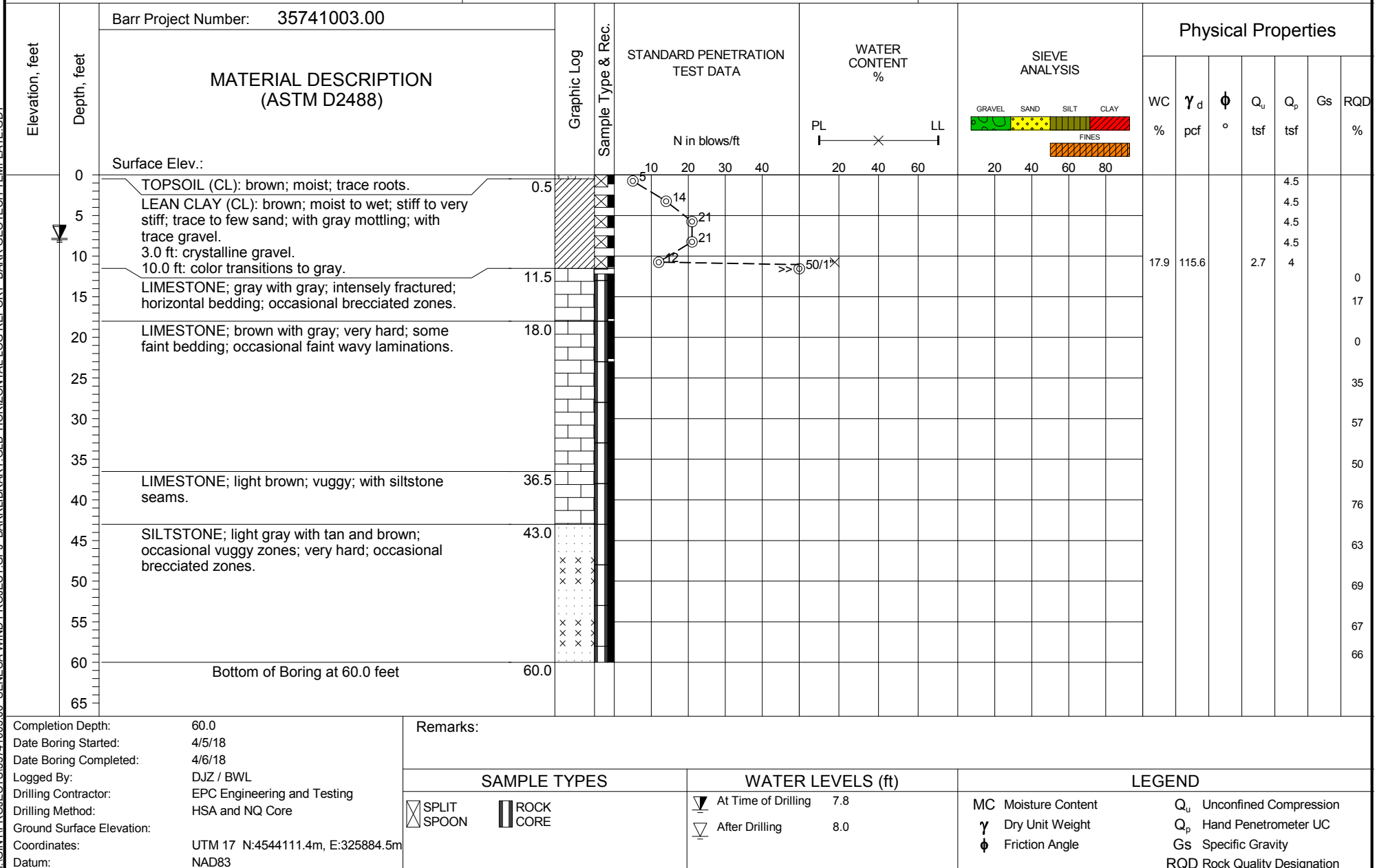
Sheet 1 of 1

Project: Seneca Wind Project

Location: Seneca County, Ohio

Client: sPower

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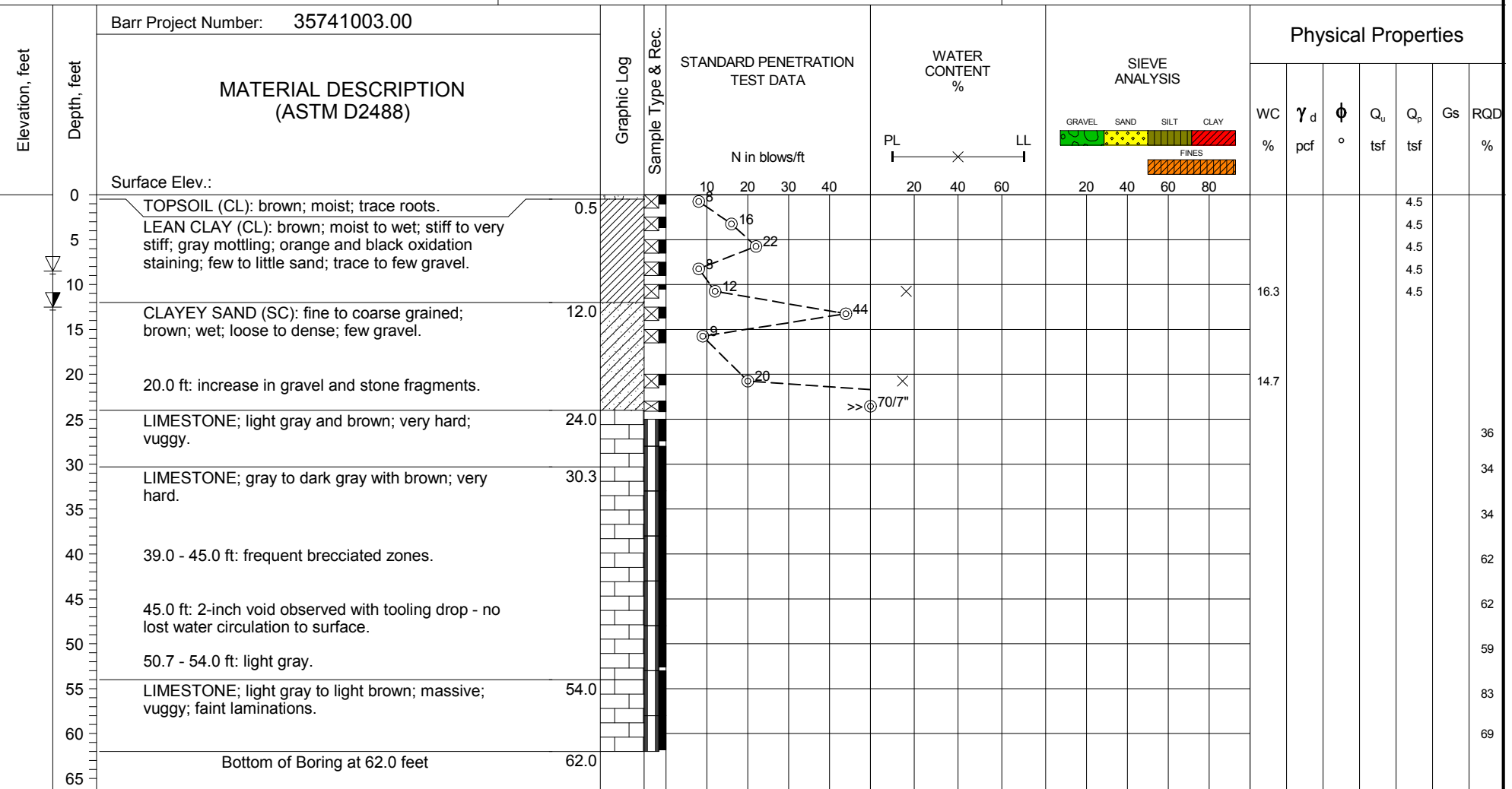
LOG OF BORING GEO-051

Sheet 1 of 1

Project: Seneca Wind Project

Location: Seneca County, Ohio

Client: sPower



Completion Depth: 62.0
Date Boring Started: 4/5/18
Date Boring Completed: 4/5/18
Logged By: DJZ / BWL
Drilling Contractor: EPC Engineering and Testing
Drilling Method: HSA and NQ Core
Ground Surface Elevation:
Coordinates: UTM 17 N:4543626.0m, E:325869.2m
Datum: NAD83

Remarks:

SAMPLE TYPES

SPLIT SPOON ROCK CORE

WATER LEVELS (ft)

At Time of Drilling 12.5
 After Drilling 8.5

LEGEND

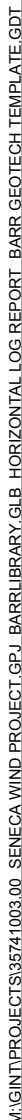
MC Moisture Content Q_u Unconfined Compression
 γ Dry Unit Weight Q_p Hand Penetrometer UC
 ϕ Friction Angle Gs Specific Gravity
RQD Rock Quality Designation

The stratification lines represent approximate boundaries. The transition may be gradual.



Sheet 1 of 1

Client: sPower



MC	Moisture Content	Q_u	Unconfined Compression
γ	Dry Unit Weight	Q_p	Hand Penetrometer UC
ϕ	Friction Angle	Gs	Specific Gravity
		RQD	Rock Quality Designation

The stratification lines represent approximate boundaries. The transition may be gradual.



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LOG OF BORING GEO-053

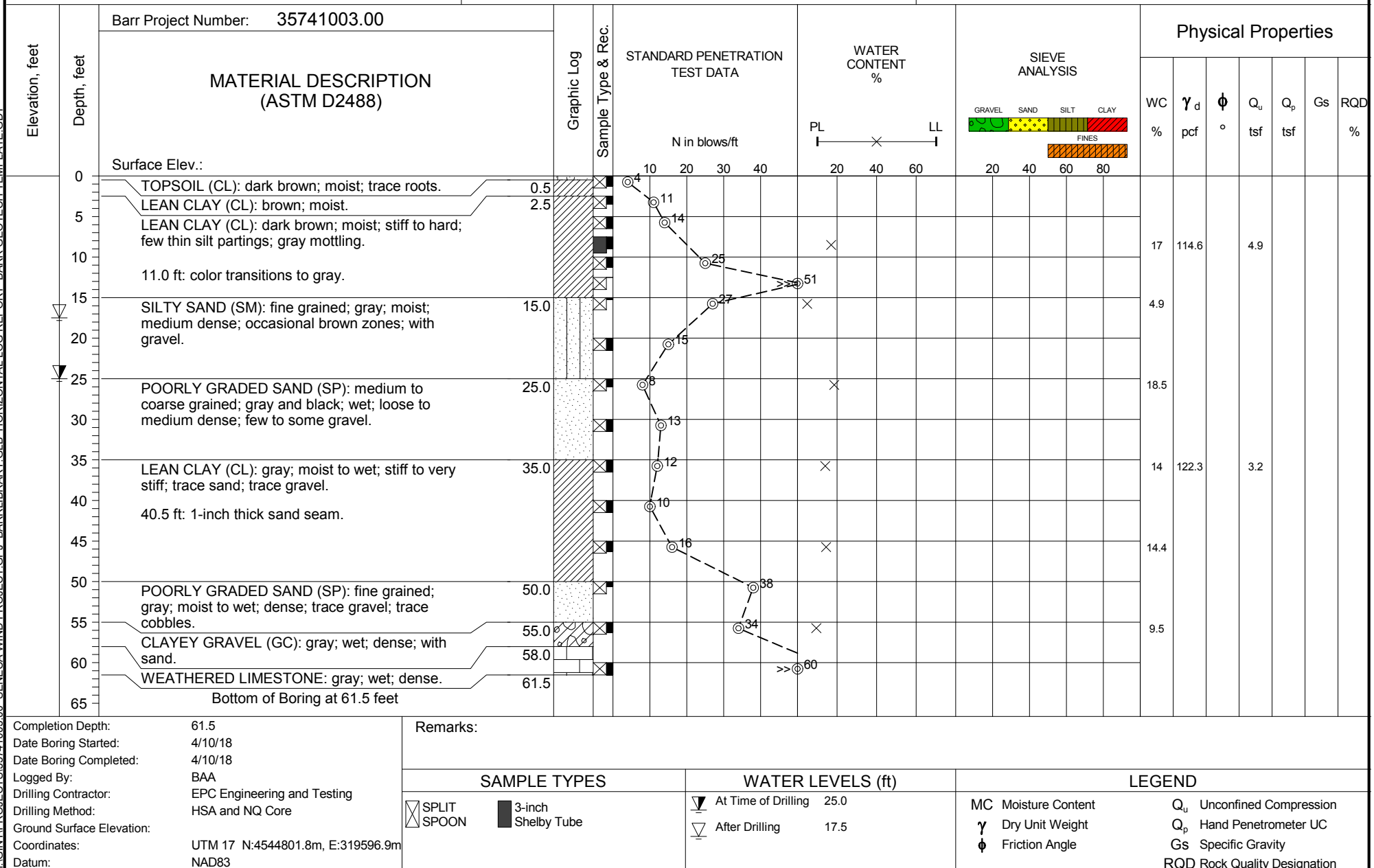
Sheet 1 of 1

Project: Seneca Wind Project

Location: Seneca County, Ohio

Client: sPower

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The stratification lines represent approximate boundaries. The transition may be gradual.



Sheet 1 of 1

Client: sPower



Remarks:					
SAMPLE TYPES		WATER LEVELS (ft)		LEGEND	
 SPLIT SPOON  3-inch Shelby Tube	 End of Drilling 50.0  At Time of Drilling 35.5	MC Moisture Content γ Dry Unit Weight ϕ Friction Angle	Q_u Unconfined Compression Q_p Hand Penetrometer UC G_s Specific Gravity RQD Rock Quality Designation		

The stratification lines represent approximate boundaries. The transition may be gradual.



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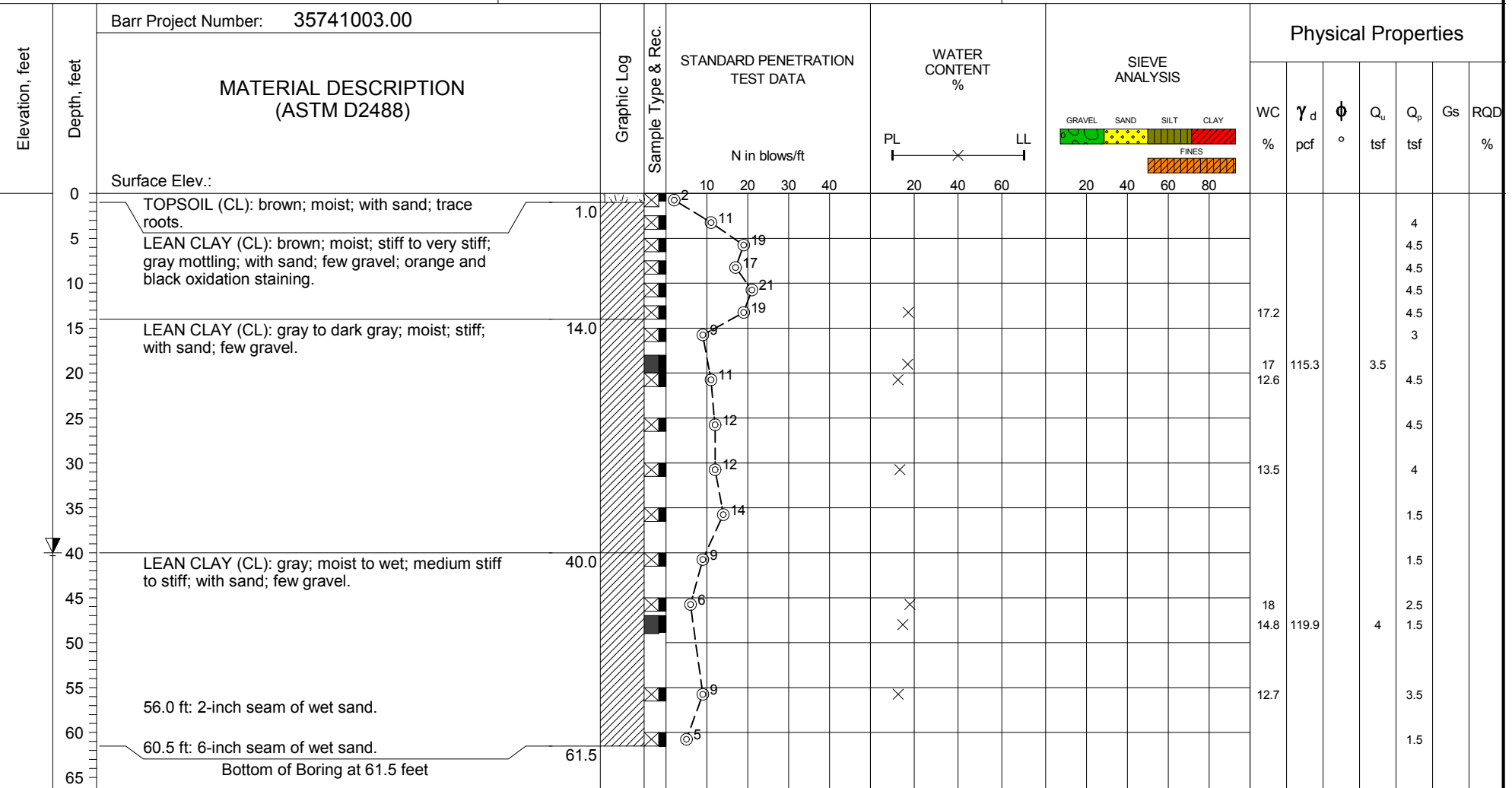
LOG OF BORING GEO-059

Sheet 1 of 1

Project: Seneca Wind Project

Location: Seneca County, Ohio

Client: sPower



Completion Depth: 61.5
Date Boring Started: 4/27/18
Date Boring Completed: 4/27/18
Logged By: DJZ
Drilling Contractor: EPC Engineering and Testing
Drilling Method: HSA
Ground Surface Elevation:
Coordinates: UTM 17 N:4547019.2m, E:345658.2m
Datum: NAD83

Remarks:

SAMPLE TYPES

⊠ SPLIT SPOON 3-inch Shelby Tube

WATER LEVELS (ft)

▼ End of Drilling
▼ At Time of Drilling 40.0

LEGEND

MC Moisture Content Q_u Unconfined Compression
 γ Dry Unit Weight Q_p Hand Penetrometer UC
 ϕ Friction Angle Gs Specific Gravity
RQD Rock Quality Designation

The stratification lines represent approximate boundaries. The transition may be gradual.



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LOG OF BORING GEO-060

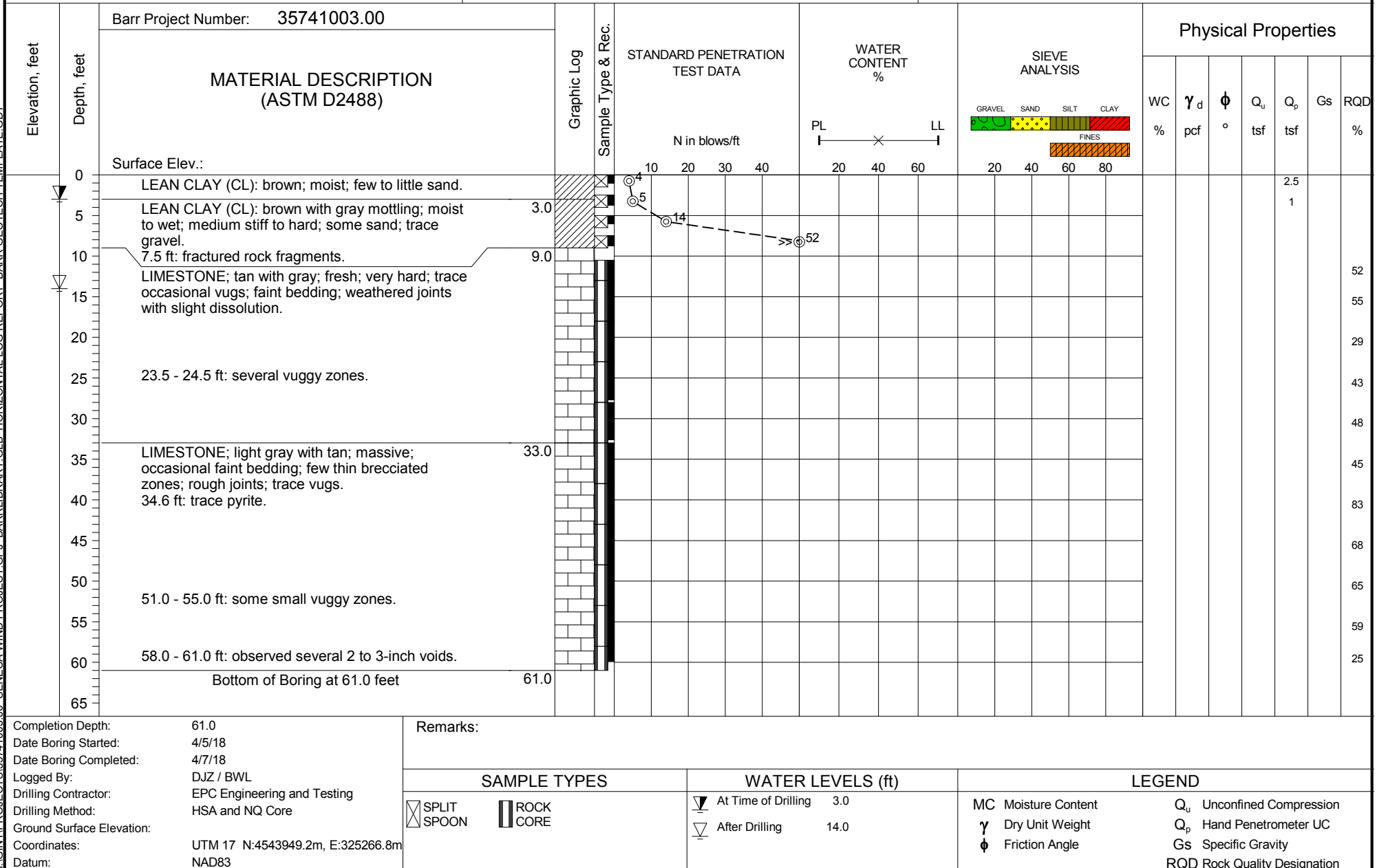
Sheet 1 of 1

Project: Seneca Wind Project

Location: Seneca County, Ohio

Client: sPower

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The stratification lines represent approximate boundaries. The transition may be gradual.



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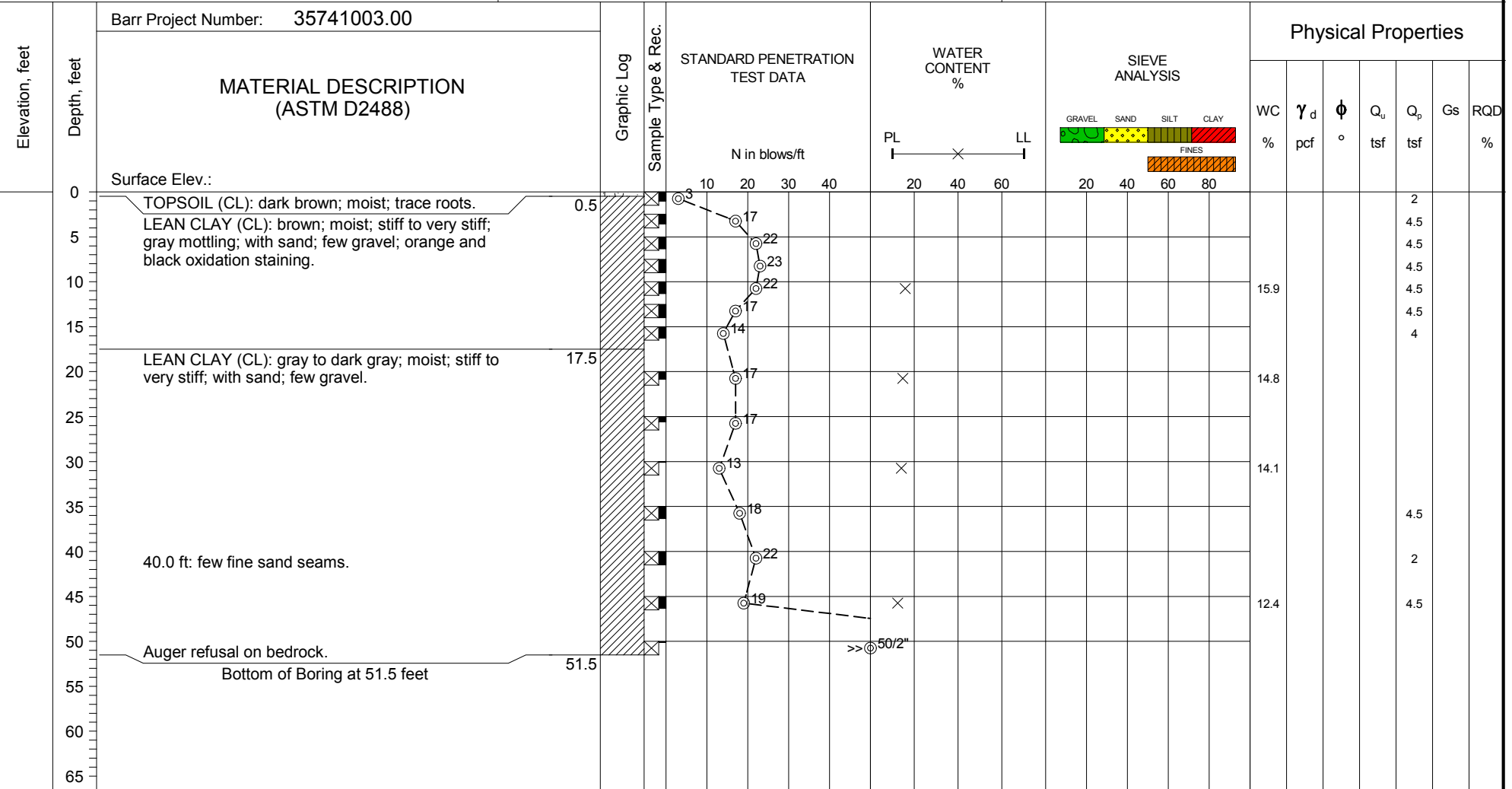
LOG OF BORING GEO-061

Sheet 1 of 1

Project: Seneca Wind Project

Location: Seneca County, Ohio

Client: sPower



Completion Depth: 51.5
Date Boring Started: 4/21/18
Date Boring Completed: 4/21/18
Logged By: DJZ
Drilling Contractor: EPC Engineering and Testing
Drilling Method: HSA
Ground Surface Elevation:
Coordinates: UTM 17 N:4553223.6m, E:336017.9m
Datum: NAD83

Remarks:

SAMPLE TYPES

⊠ SPLIT
⊠ SPOON

WATER LEVELS (ft)

▼ End of Drilling
Dry
▼ At Time of Drilling
Dry

LEGEND

MC	Moisture Content	Q_u	Unconfined Compression
γ	Dry Unit Weight	Q_p	Hand Penetrometer UC
ϕ	Friction Angle	Gs	Specific Gravity
		RQD	Rock Quality Designation

The stratification lines represent approximate boundaries. The transition may be gradual.



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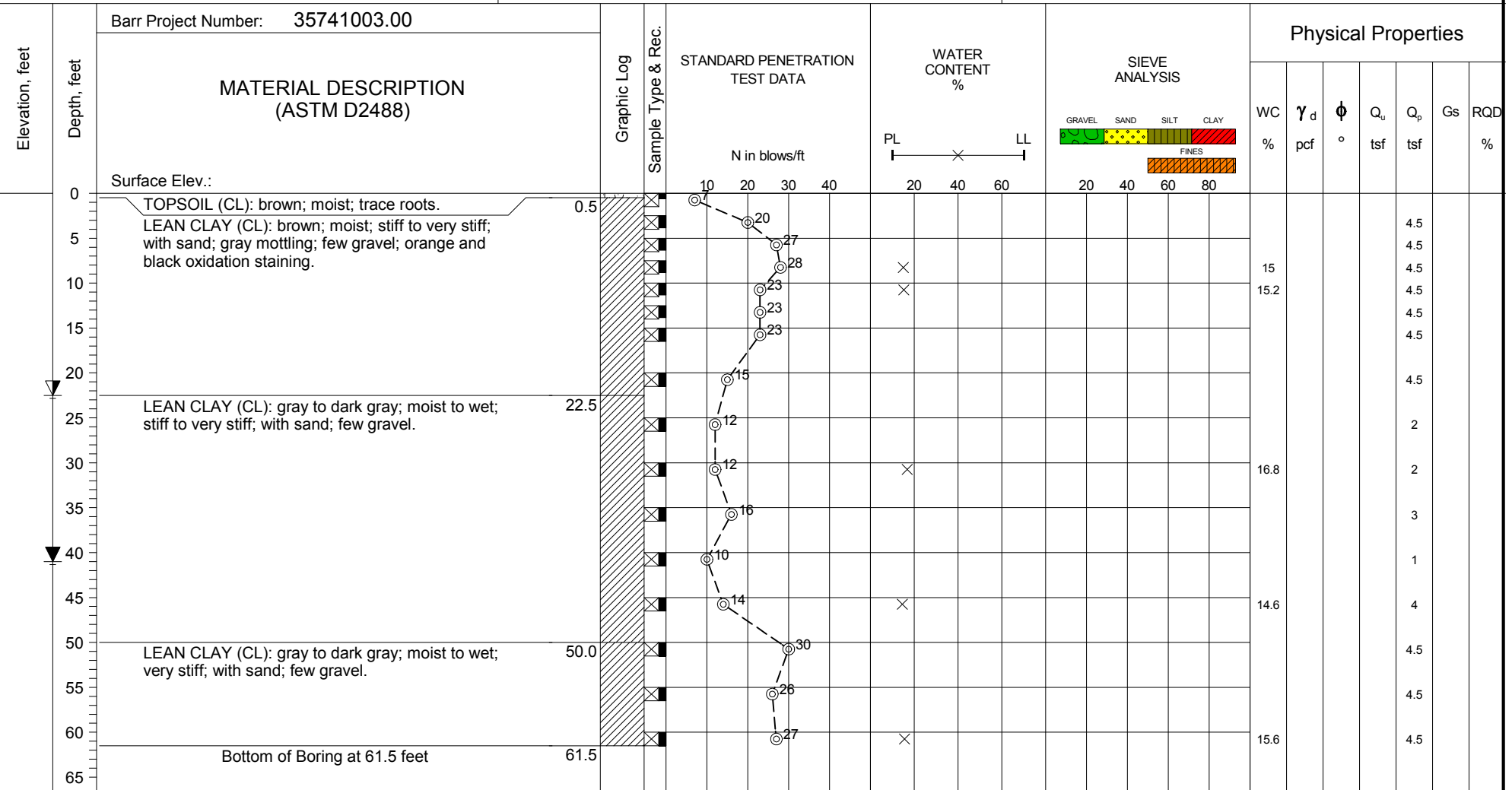
LOG OF BORING GEO-062

Sheet 1 of 1

Project: Seneca Wind Project

Location: Seneca County, Ohio

Client: sPower



Completion Depth:	61.5	Remarks:
Date Boring Started:	5/1/18	
Date Boring Completed:	5/1/18	
Logged By:	DJZ	
Drilling Contractor:	EPC Engineering and Testing	
Drilling Method:	HSA	
Ground Surface Elevation:		
Coordinates:	UTM 17 N:4548686.1m, E:328618.5m	
Datum:	NAD83	

SAMPLE TYPES	WATER LEVELS (ft)	LEGEND
SPLIT SPOON	End of Drilling 41.0	MC Moisture Content
	At Time of Drilling 22.5	γ_d Dry Unit Weight
		ϕ Friction Angle
		Q_u Unconfined Compression
		Q_p Hand Penetrometer UC
		Gs Specific Gravity
		RQD Rock Quality Designation

The stratification lines represent approximate boundaries. The transition may be gradual.



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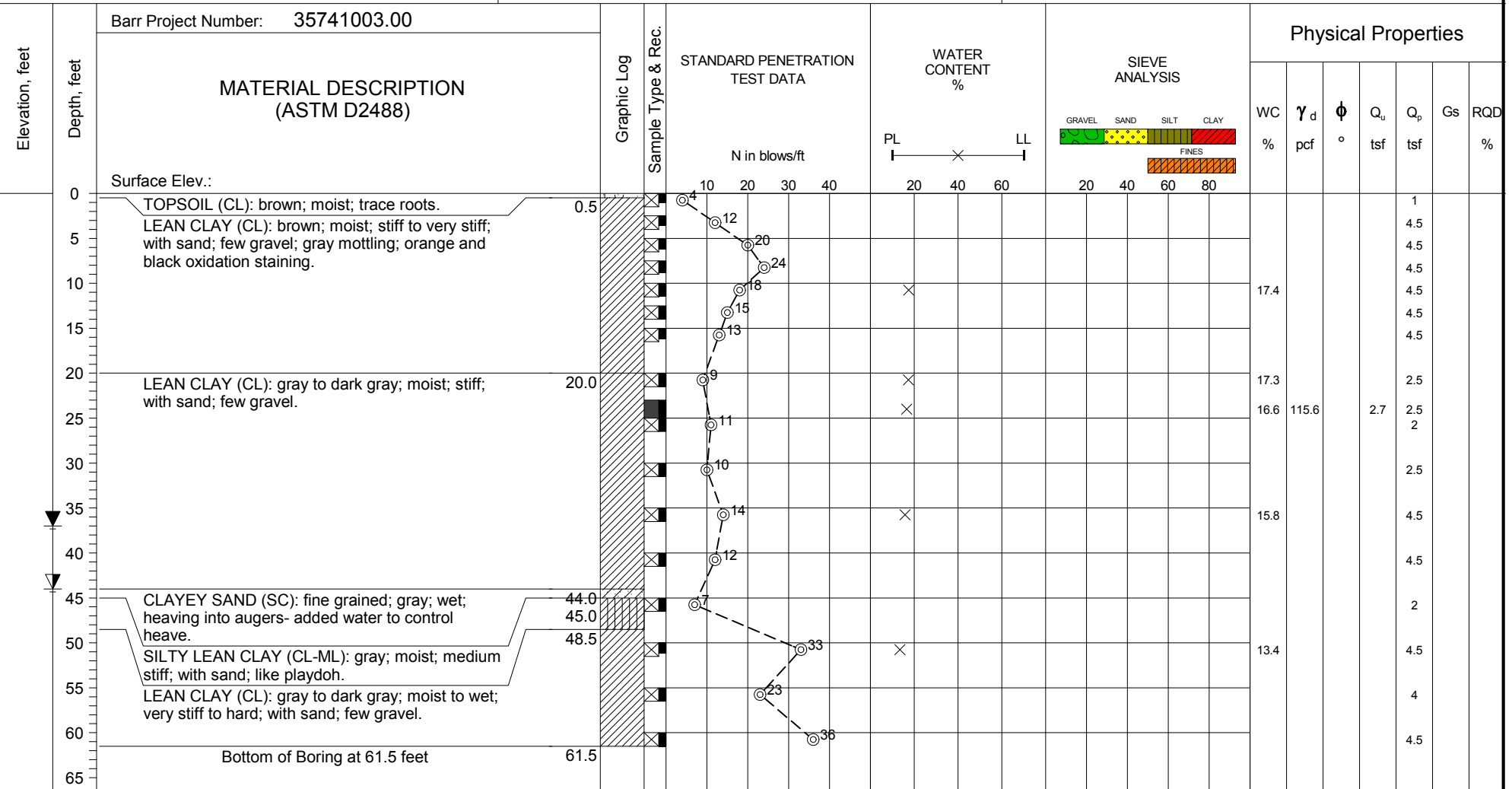
LOG OF BORING GEO-063

Sheet 1 of 1

Project: Seneca Wind Project

Location: Seneca County, Ohio

Client: sPower



Completion Depth: 61.5
Date Boring Started: 4/22/18
Date Boring Completed: 4/22/18
Logged By: DJZ
Drilling Contractor: EPC Engineering and Testing
Drilling Method: HSA
Ground Surface Elevation:
Coordinates: UTM 17 N:4552372.8m, E:337543.9m
Datum: NAD83

Remarks:

SAMPLE TYPES

⊠ SPLIT SPOON 3-inch Shelby Tube

WATER LEVELS (ft)

▼ End of Drilling 37.0
▼ At Time of Drilling 44.0

LEGEND

MC	Moisture Content	Q_u	Unconfined Compression
γ	Dry Unit Weight	Q_p	Hand Penetrometer UC
ϕ	Friction Angle	Gs	Specific Gravity
		RQD	Rock Quality Designation

The stratification lines represent approximate boundaries. The transition may be gradual.



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LOG OF BORING GEO-066

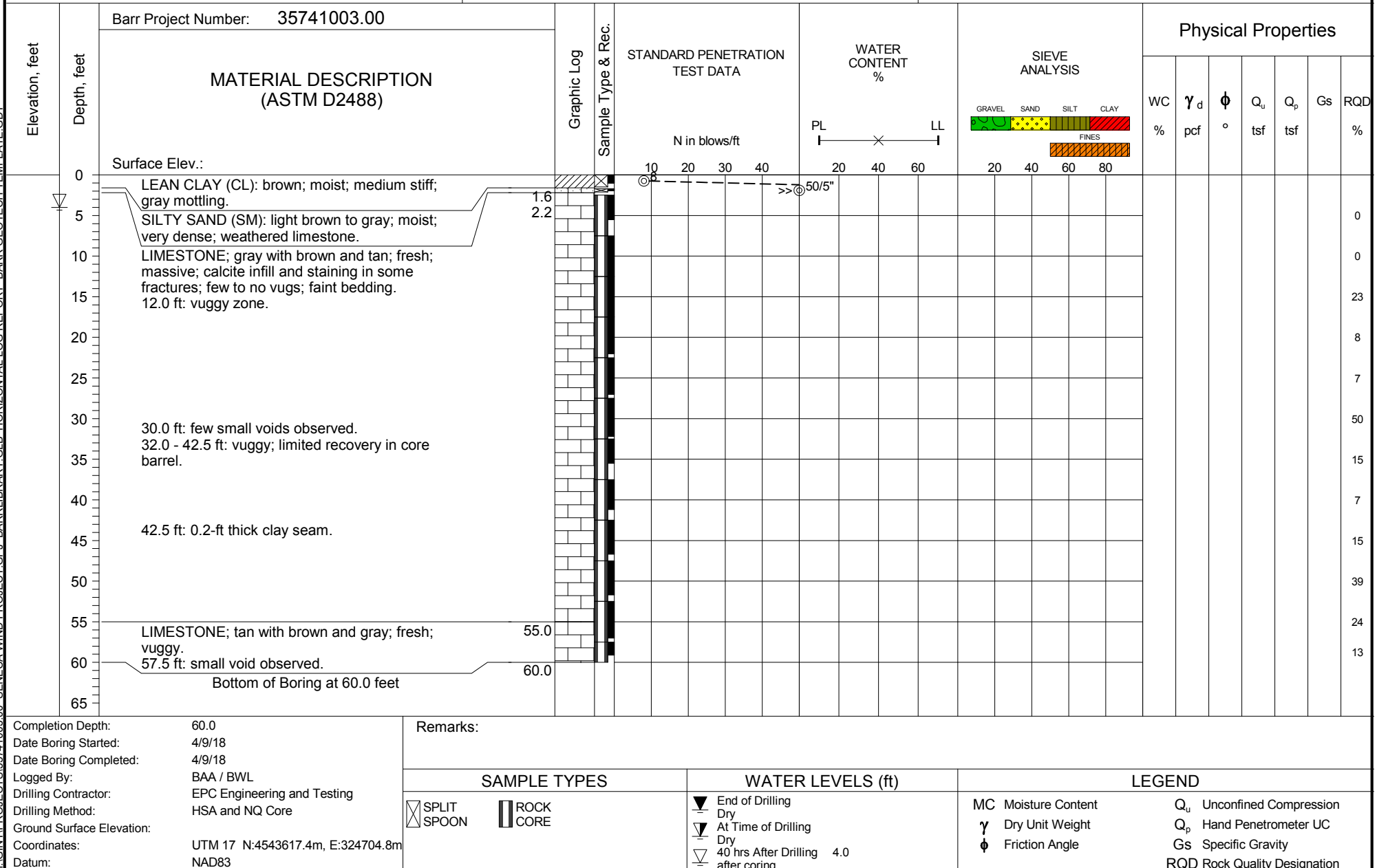
Sheet 1 of 1

Project: Seneca Wind Project

Location: Seneca County, Ohio

Client: sPower

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The stratification lines represent approximate boundaries. The transition may be gradual.



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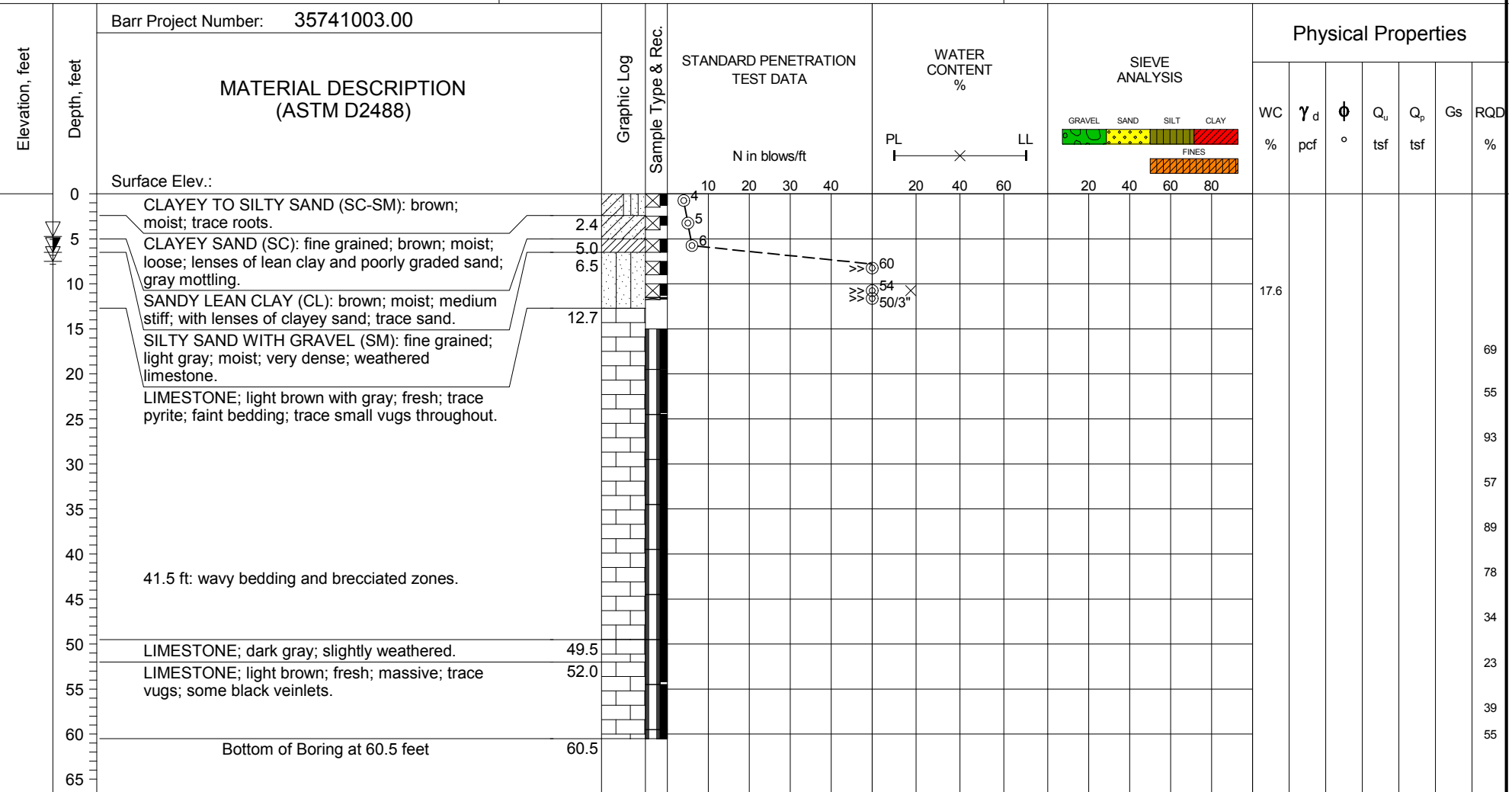
LOG OF BORING GEO-068

Sheet 1 of 1

Project: Seneca Wind Project

Location: Seneca County, Ohio

Client: sPower



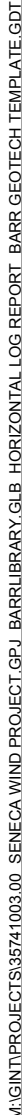
Completion Depth:	60.5	Remarks:
Date Boring Started:	4/9/18	
Date Boring Completed:	4/10/18	
Logged By:	BAA / BWL	
Drilling Contractor:	EPC Engineering and Testing	
Drilling Method:	HSA and NQ Core	
Ground Surface Elevation:		
Coordinates:	UTM 17 N:4544367.3m, E:326870.1m	
Datum:	NAD83	
SAMPLE TYPES		WATER LEVELS (ft)
SPLIT SPOON ROCK CORE		At Time of Drilling 6.5
		After Drilling 7.5
		12 hrs After Drilling 4.8
LEGEND		
MC	Moisture Content	Q_u Unconfined Compression
γ	Dry Unit Weight	Q_p Hand Penetrometer UC
ϕ	Friction Angle	Gs Specific Gravity
		RQD Rock Quality Designation

The stratification lines represent approximate boundaries. The transition may be gradual.



Sheet 1 of 1

Client: sPower



MC	Moisture Content	Q_u	Unconfined Compression
γ	Dry Unit Weight	Q_p	Hand Penetrometer UC
ϕ	Friction Angle	Gs	Specific Gravity
		RQD	Rock Quality Designation

The stratification lines represent approximate boundaries. The transition may be gradual.



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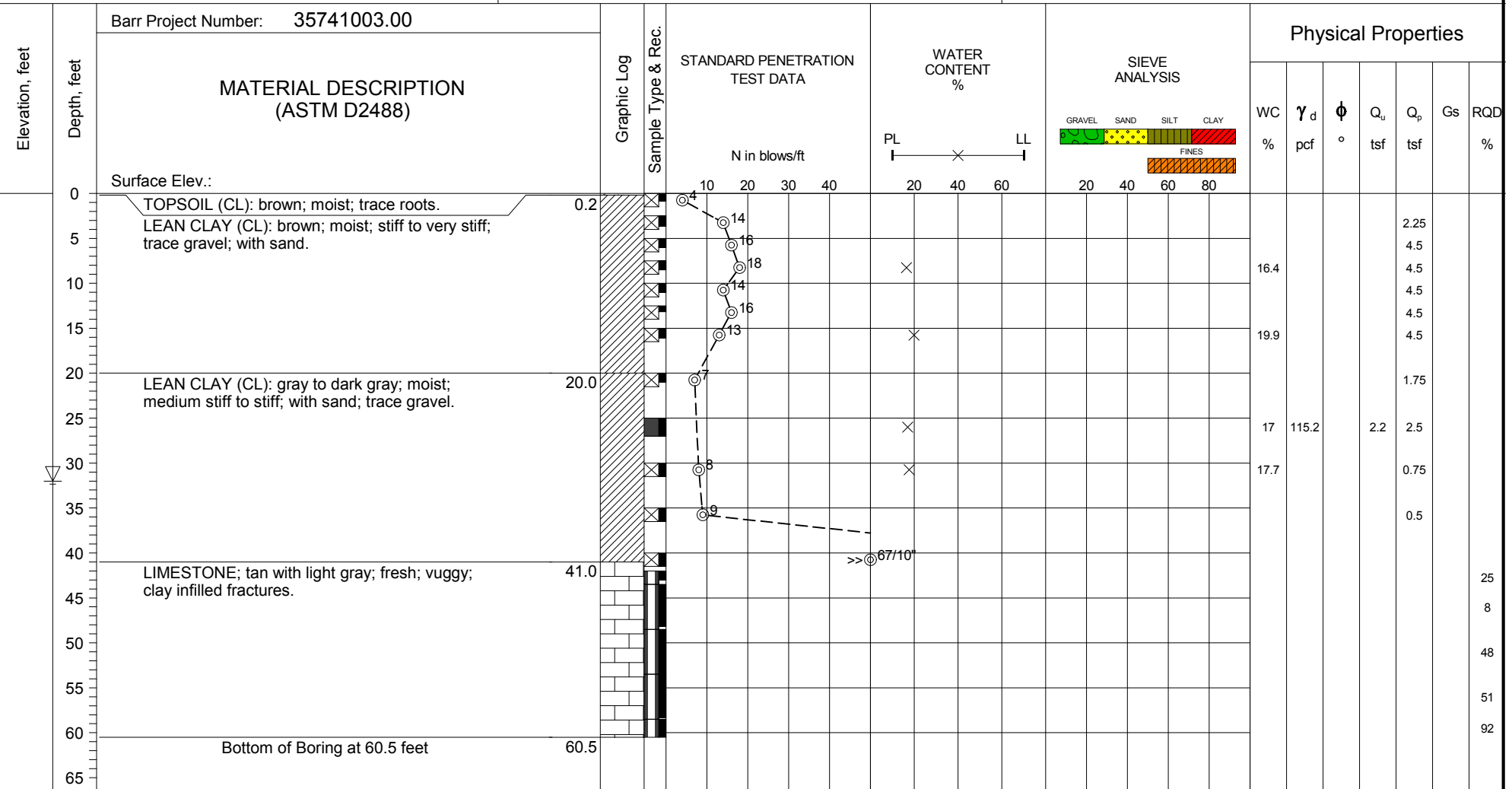
LOG OF BORING GEO-070

Sheet 1 of 1

Project: Seneca Wind Project

Location: Seneca County, Ohio

Client: sPower



Completion Depth: 60.5
Date Boring Started: 4/21/18
Date Boring Completed: 4/21/18
Logged By: IGM / DJZ
Drilling Contractor: EPC Engineering and Testing
Drilling Method: HSA and NQ Core
Ground Surface Elevation:
Coordinates: UTM 17 N:4547759.0m, E:328810.0m
Datum: NAD83

Remarks:

SAMPLE TYPES

SPLIT SPOON 3-inch Shelby Tube ROCK CORE

WATER LEVELS (ft)

After Drilling before coring 32.0
 At Time of Drilling
 Dry

LEGEND

MC Moisture Content Q_u Unconfined Compression
 γ Dry Unit Weight Q_p Hand Penetrometer UC
 ϕ Friction Angle Gs Specific Gravity
RQD Rock Quality Designation

The stratification lines represent approximate boundaries. The transition may be gradual.



Sheet 1 of 1

Client: sPower



MC	Moisture Content	Q_u	Unconfined Compression
γ	Dry Unit Weight	Q_p	Hand Penetrometer UC
ϕ	Friction Angle	Gs	Specific Gravity
		RQD	Rock Quality Designation

The stratification lines represent approximate boundaries. The transition may be gradual.



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LOG OF BORING GEO-072

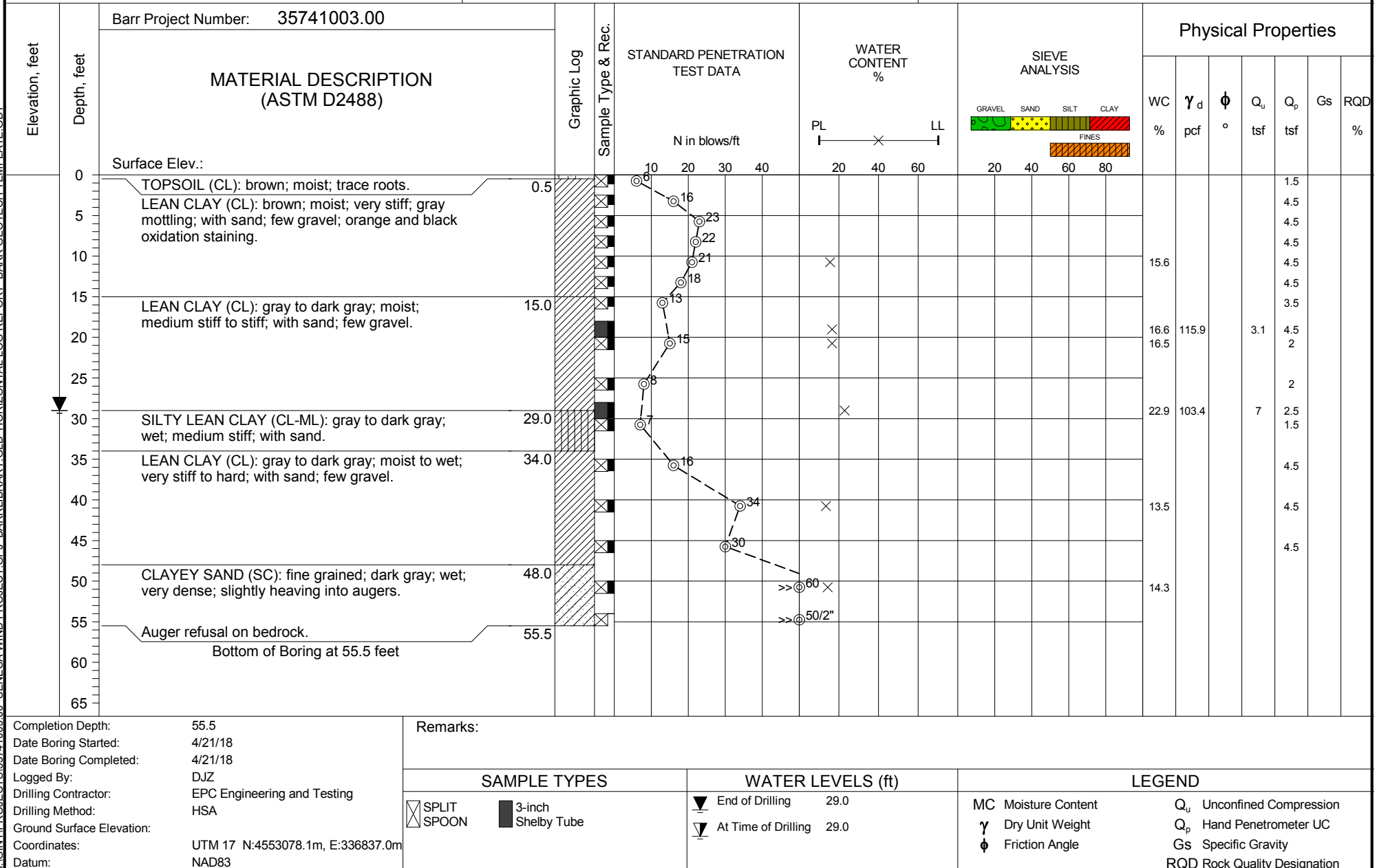
Sheet 1 of 1

Project: Seneca Wind Project

Location: Seneca County, Ohio

Client: sPower

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The stratification lines represent approximate boundaries. The transition may be gradual.



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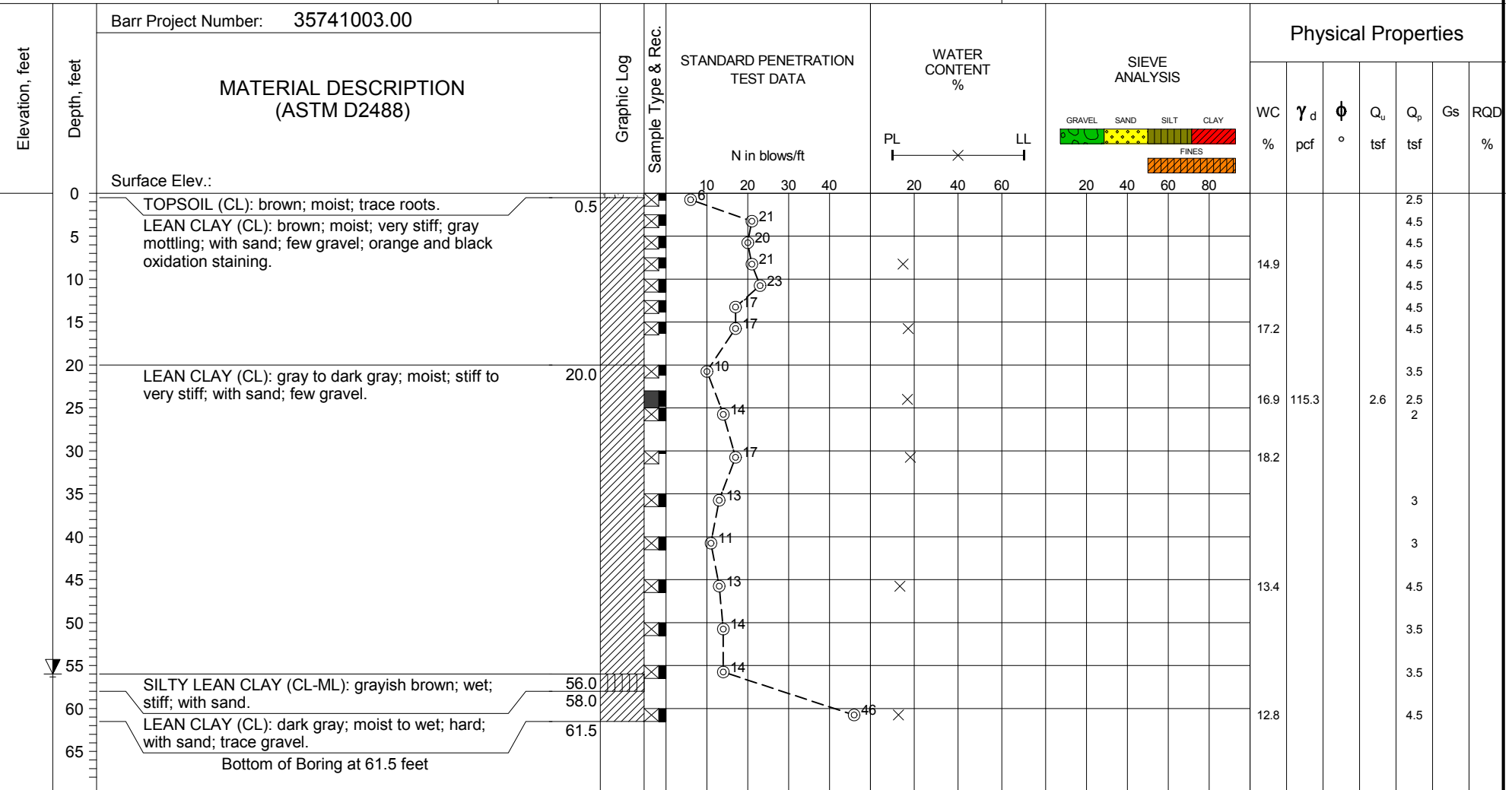
LOG OF BORING GEO-074

Sheet 1 of 1

Project: Seneca Wind Project

Location: Seneca County, Ohio

Client: sPower



Completion Depth: 61.5
Date Boring Started: 4/22/18
Date Boring Completed: 4/22/18
Logged By: DJZ
Drilling Contractor: EPC Engineering and Testing
Drilling Method: HSA
Ground Surface Elevation: UTM 17 N:4552379.1m, E:337006.9m
Coordinates: UTM 17 N:4552379.1m, E:337006.9m
Datum: NAD83

Remarks:

SAMPLE TYPES

⊠ SPLIT SPOON 3-inch Shelby Tube

WATER LEVELS (ft)

▼ End of Drilling Dry
▼ At Time of Drilling 56.0

LEGEND

MC Moisture Content Q_u Unconfined Compression
 γ Dry Unit Weight Q_p Hand Penetrometer UC
 ϕ Friction Angle Gs Specific Gravity
RQD Rock Quality Designation

The stratification lines represent approximate boundaries. The transition may be gradual.



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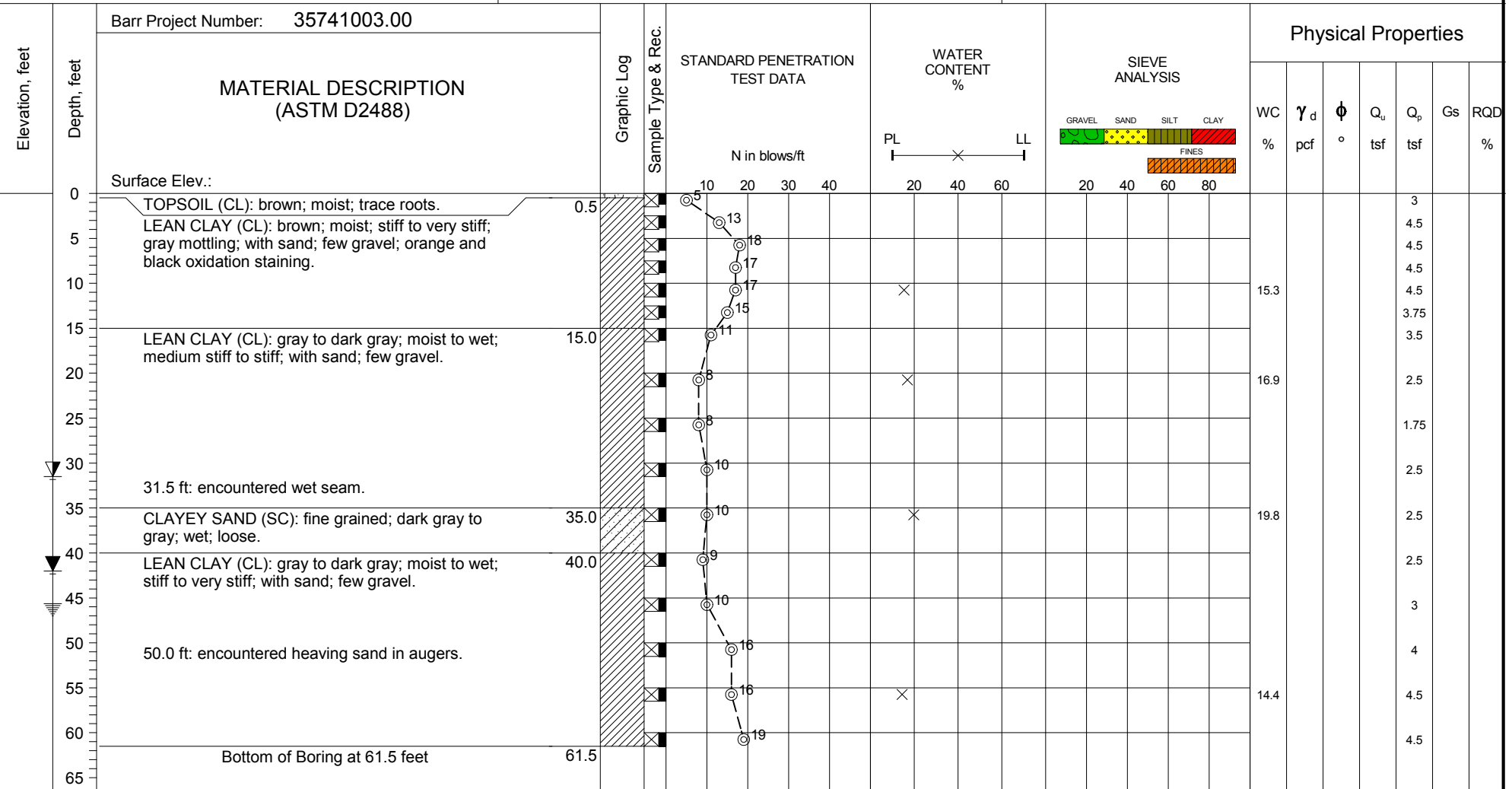
LOG OF BORING GEO-075

Sheet 1 of 1

Project: Seneca Wind Project

Location: Seneca County, Ohio

Client: sPower



Completion Depth: 61.5
Date Boring Started: 5/9/18
Date Boring Completed: 5/9/18
Logged By: DJZ
Drilling Contractor: EPC Engineering and Testing
Drilling Method: HSA
Ground Surface Elevation:
Coordinates: UTM 17 N:4551391.1m, E:338409.0m
Datum: NAD83

Remarks:

SAMPLE TYPES

⊠ SPLIT
⊠ SPOON

WATER LEVELS (ft)

Wet Cave-in Depth 47.0
End of Drilling 42.0
At Time of Drilling 31.5

LEGEND

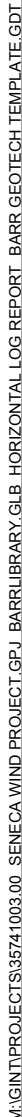
MC Moisture Content
 γ Dry Unit Weight
 ϕ Friction Angle
 Q_u Unconfined Compression
 Q_p Hand Penetrometer UC
Gs Specific Gravity
RQD Rock Quality Designation

The stratification lines represent approximate boundaries. The transition may be gradual.



Sheet 1 of 1

Client: sPower



MC	Moisture Content	Q_u	Unconfined Compression
γ	Dry Unit Weight	Q_p	Hand Penetrometer UC
ϕ	Friction Angle	Gs	Specific Gravity
		RQD	Rock Quality Designation

The stratification lines represent approximate boundaries. The transition may be gradual.



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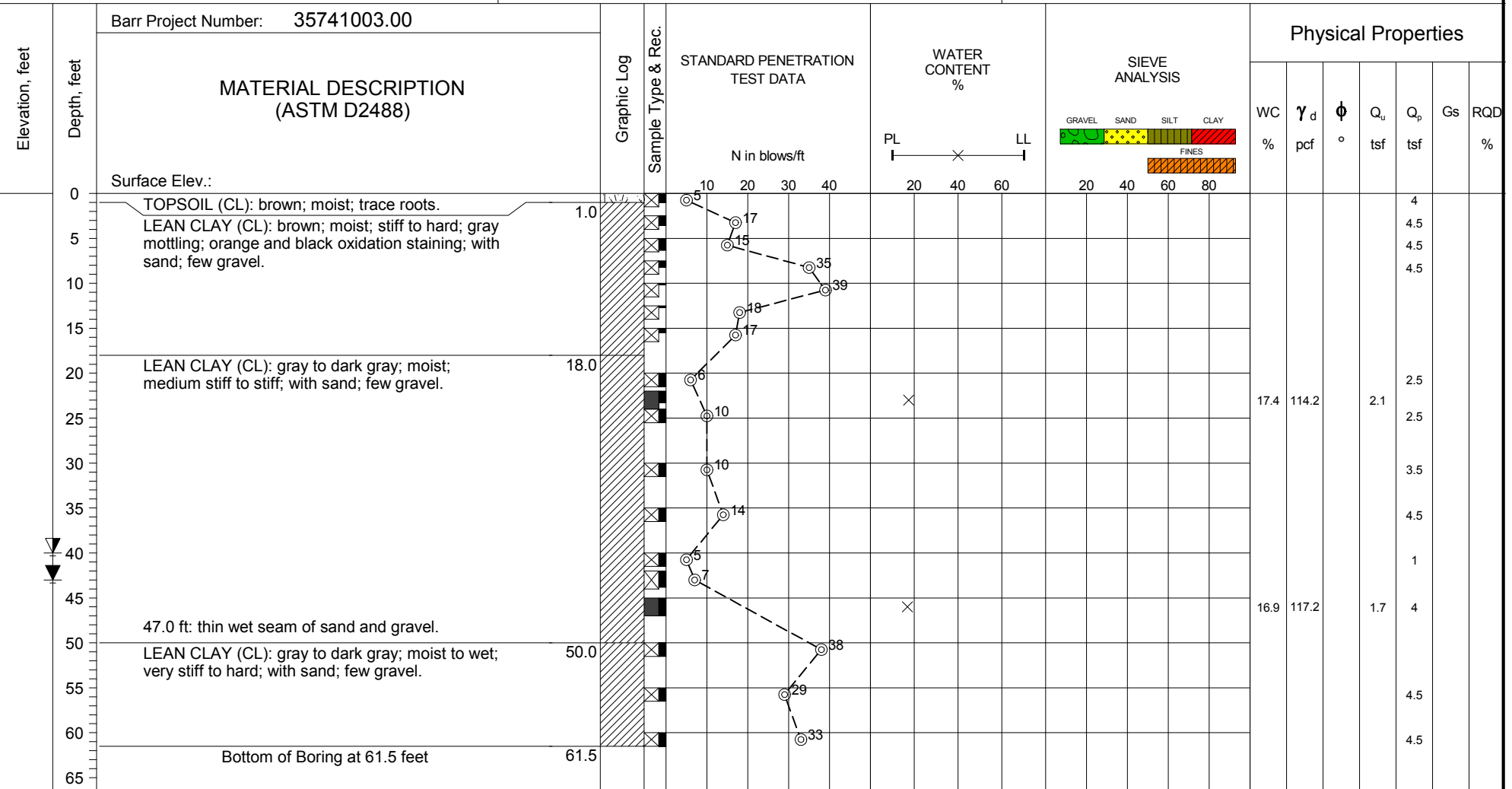
LOG OF BORING GEO-077

Sheet 1 of 1

Project: Seneca Wind Project

Location: Seneca County, Ohio

Client: sPower



Completion Depth: 61.5
Date Boring Started: 4/27/18
Date Boring Completed: 4/27/18
Logged By: DJZ
Drilling Contractor: EPC Engineering and Testing
Drilling Method: HSA
Ground Surface Elevation:
Coordinates: UTM 17 N:4548935.4m, E:344812.7m
Datum: NAD83

Remarks:

SAMPLE TYPES

⊠ SPLIT
SPOON 3-inch
Shelby Tube

WATER LEVELS (ft)

▼ End of Drilling 43.0
▼ At Time of Drilling 40.0

LEGEND

MC Moisture Content Q_u Unconfined Compression
 γ Dry Unit Weight Q_p Hand Penetrometer UC
 ϕ Friction Angle Gs Specific Gravity
RQD Rock Quality Designation

The stratification lines represent approximate boundaries. The transition may be gradual.



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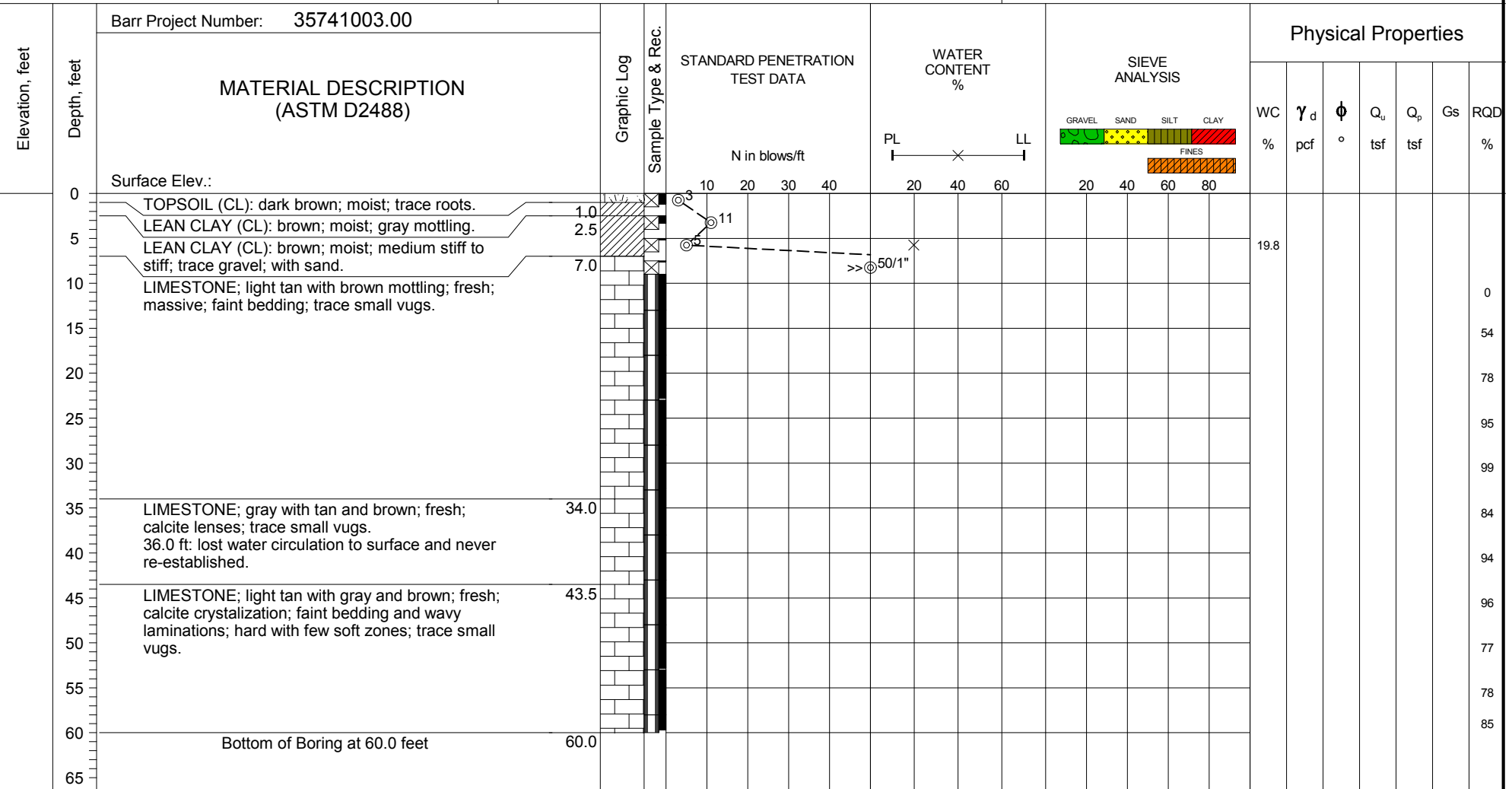
LOG OF BORING GEO-078

Sheet 1 of 1

Project: Seneca Wind Project

Location: Seneca County, Ohio

Client: sPower



Completion Depth: 60.0
Date Boring Started: 4/13/18
Date Boring Completed: 4/13/18
Logged By: BAA / BWL
Drilling Contractor: EPC Engineering and Testing
Drilling Method: HSA and NQ Core
Ground Surface Elevation:
Coordinates: UTM 17 N:4545354.8m, E:328297.0m
Datum: NAD83

Remarks:

SAMPLE TYPES

⊠ SPLIT SPOON █ ROCK CORE

WATER LEVELS (ft)

▼ At Time of Drilling before coringDry

LEGEND

MC	Moisture Content	Q_u	Unconfined Compression
γ	Dry Unit Weight	Q_p	Hand Penetrometer UC
ϕ	Friction Angle	Gs	Specific Gravity
		RQD	Rock Quality Designation

The stratification lines represent approximate boundaries. The transition may be gradual.



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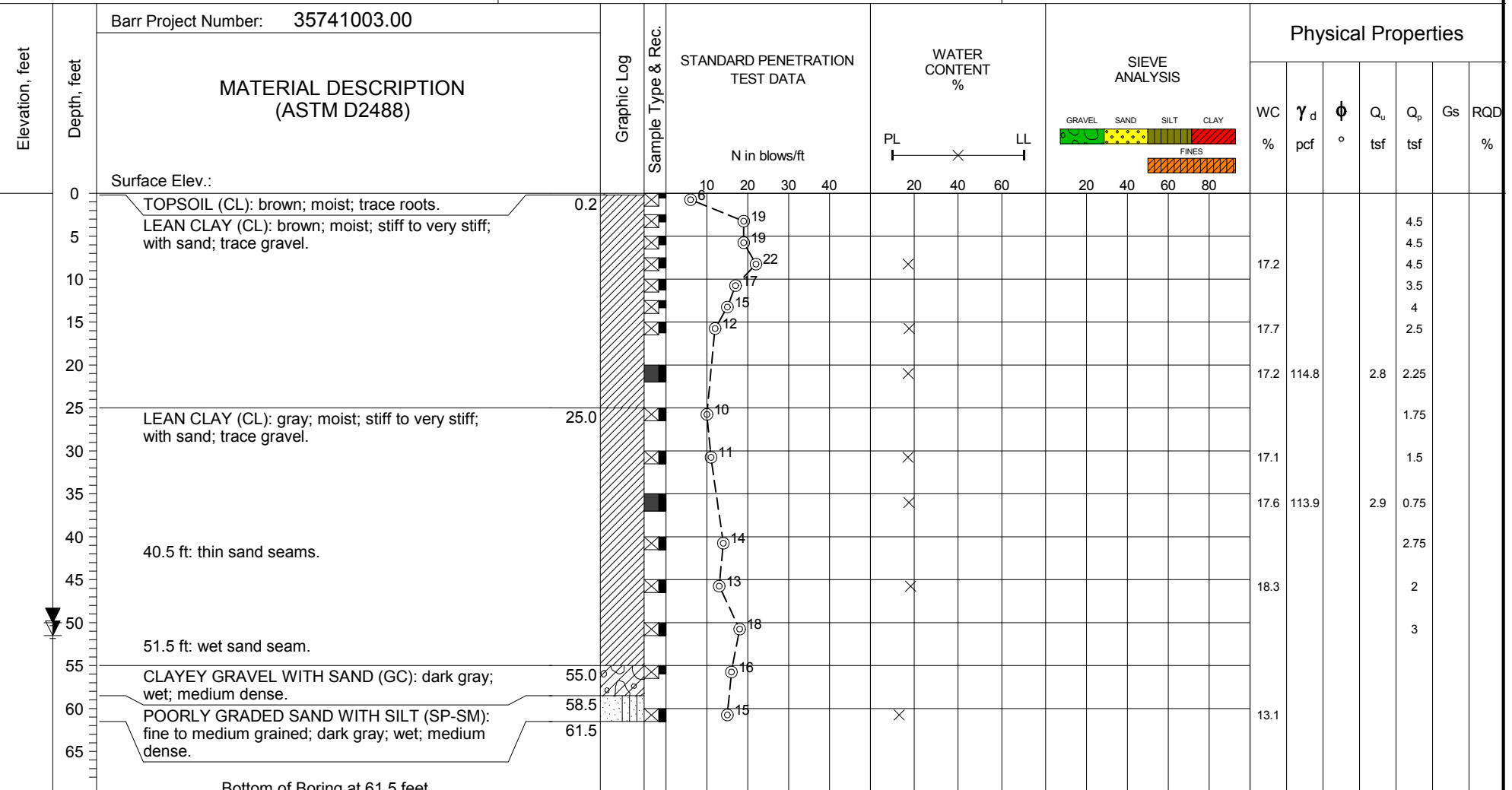
LOG OF BORING GEO-079

Sheet 1 of 1

Project: Seneca Wind Project

Location: Seneca County, Ohio

Client: sPower



Completion Depth:	61.5	Remarks:
Date Boring Started:	4/20/18	
Date Boring Completed:	4/20/18	
Logged By:	IGM	
Drilling Contractor:	EPC Engineering and Testing	
Drilling Method:	HSA and NQ Core	
Ground Surface Elevation:		
Coordinates:	UTM 17 N:4546147.4m, E:325270.7m	
Datum:	NAD83	

SAMPLE TYPES	WATER LEVELS (ft)	LEGEND
SPLIT SPOON	End of Drilling 50.0	MC Moisture Content
3-inch Shelby Tube	At Time of Drilling 51.5	Q_u Unconfined Compression
		γ Dry Unit Weight
		ϕ Friction Angle
		Q_p Hand Penetrometer UC
		Gs Specific Gravity
		RQD Rock Quality Designation

The stratification lines represent approximate boundaries. The transition may be gradual.



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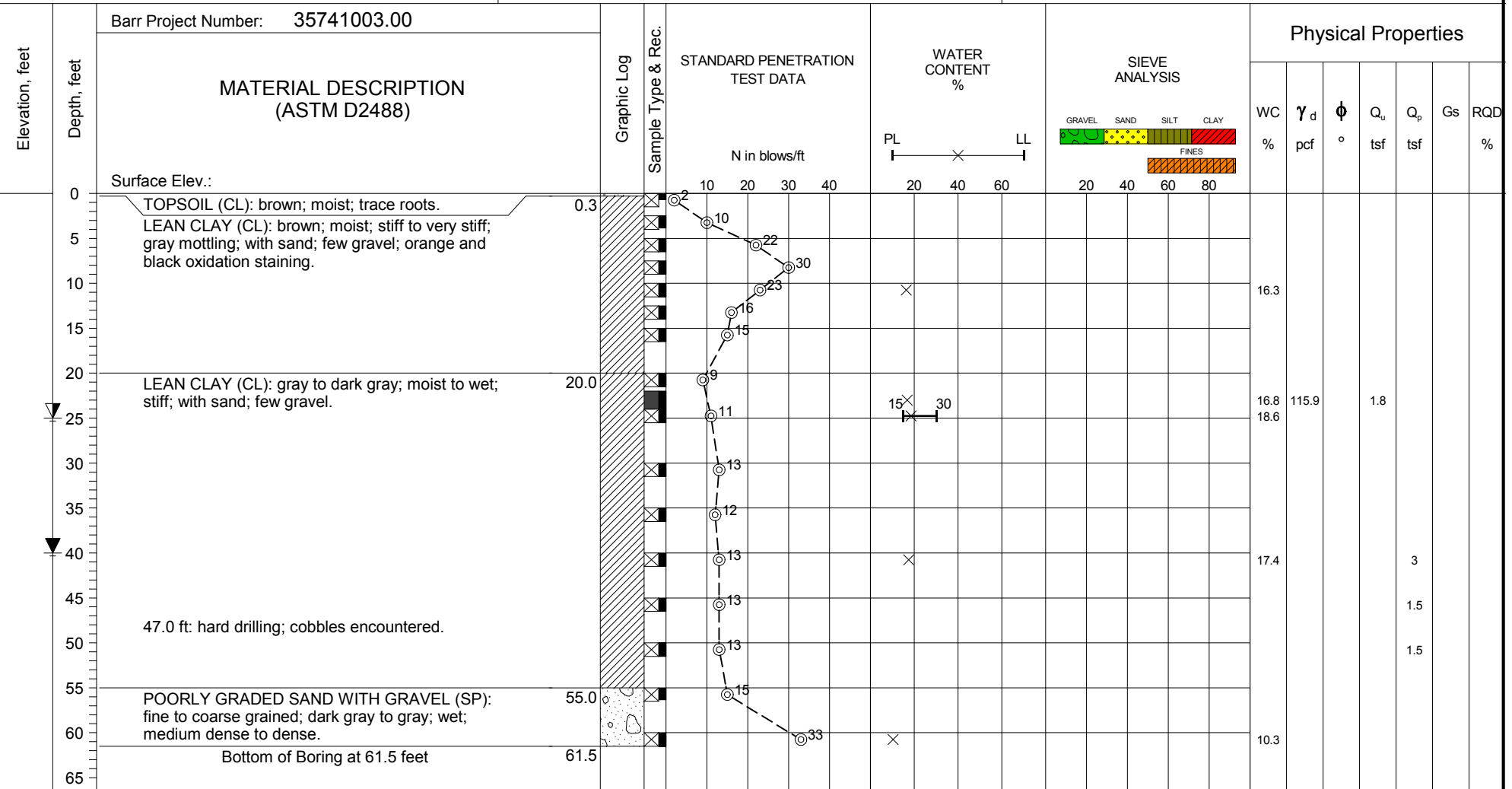
LOG OF BORING GEO-081

Sheet 1 of 1

Project: Seneca Wind Project

Location: Seneca County, Ohio

Client: sPower



Completion Depth: 61.5
Date Boring Started: 4/17/18
Date Boring Completed: 4/17/18
Logged By: DJZ
Drilling Contractor: EPC Engineering and Testing
Drilling Method: HSA
Ground Surface Elevation:
Coordinates: UTM 17 N:4547664.2m, E:326388.0m
Datum: NAD83

Remarks:

SAMPLE TYPES

⊠ SPLIT
SPOON 3-inch
Shelby Tube

WATER LEVELS (ft)

▼ End of Drilling 40.0
▼ At Time of Drilling 25.0

LEGEND

MC	Moisture Content	Q_u	Unconfined Compression
γ	Dry Unit Weight	Q_p	Hand Penetrometer UC
ϕ	Friction Angle	Gs	Specific Gravity
		RQD	Rock Quality Designation

The stratification lines represent approximate boundaries. The transition may be gradual.



Sheet 1 of 1

Client: sPower



MC	Moisture Content	Q_u	Unconfined Compression
γ	Dry Unit Weight	Q_p	Hand Penetrometer UC
ϕ	Friction Angle	Gs	Specific Gravity
		RQD	Rock Quality Designation

The stratification lines represent approximate boundaries. The transition may be gradual.



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4300 MarketPointe Drive Suite 200
Minneapolis, MN 55435
Telephone: 952-832-2600

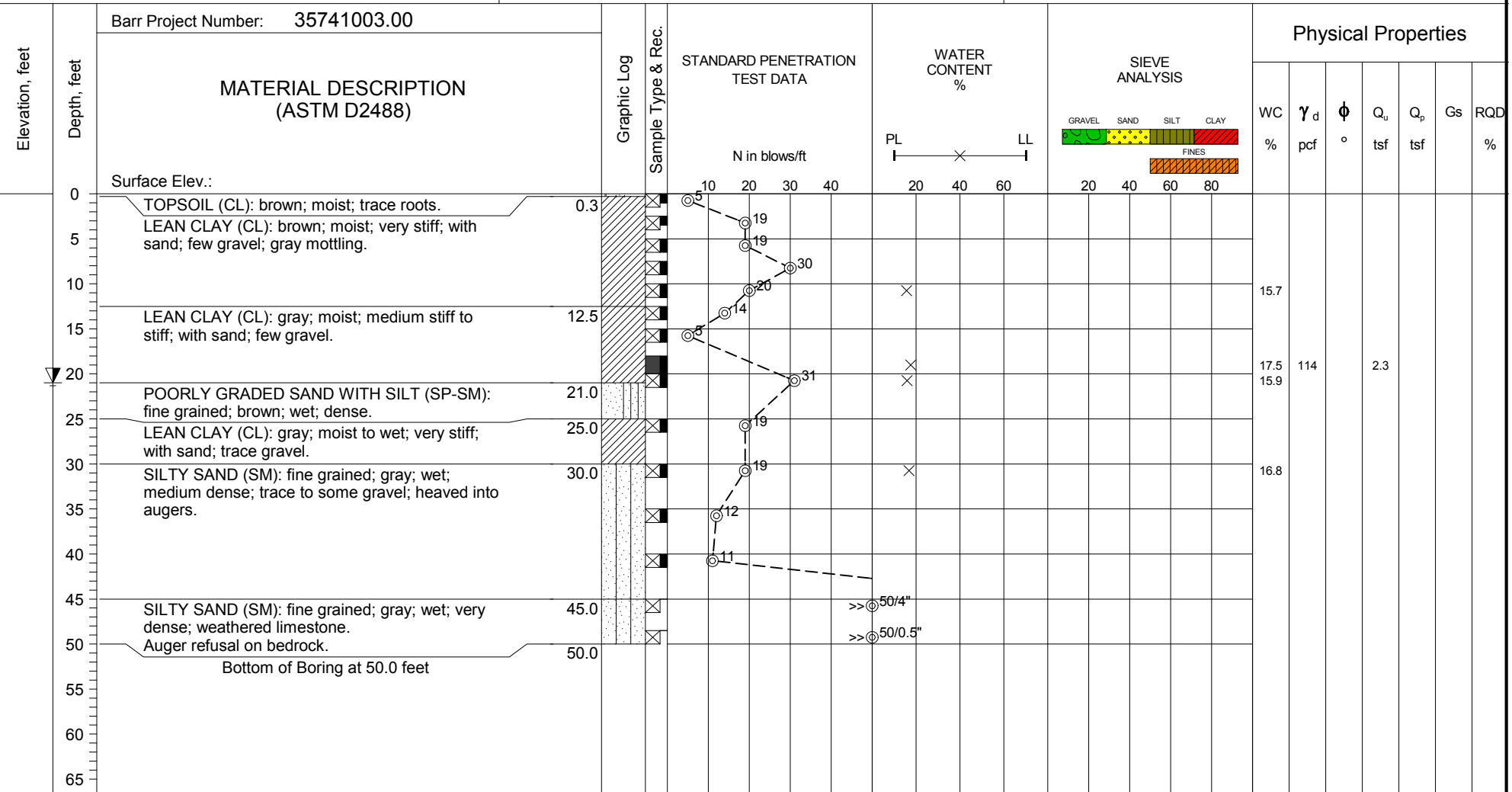
LOG OF BORING GEO-083

Sheet 1 of 1

Project: Seneca Wind Project

Location: Seneca County, Ohio

Client: sPower



Completion Depth: 50.0
Date Boring Started: 4/16/18
Date Boring Completed: 4/16/18
Logged By: CRL
Drilling Contractor: EPC Engineering and Testing
Drilling Method: HSA
Ground Surface Elevation: UTM 17 N:4546775.0m, E:326708.0m
Coordinates: UTM 17 N:4546775.0m, E:326708.0m
Datum: NAD83

Remarks:

SAMPLE TYPES

⊠ SPLIT
SPOON 3-inch
Shelby Tube

WATER LEVELS (ft)

▼ End of Drilling
Dry
▼ At Time of Drilling 21.0

LEGEND

MC Moisture Content Q_u Unconfined Compression
 γ Dry Unit Weight Q_p Hand Penetrometer UC
 ϕ Friction Angle Gs Specific Gravity
RQD Rock Quality Designation

The stratification lines represent approximate boundaries. The transition may be gradual.



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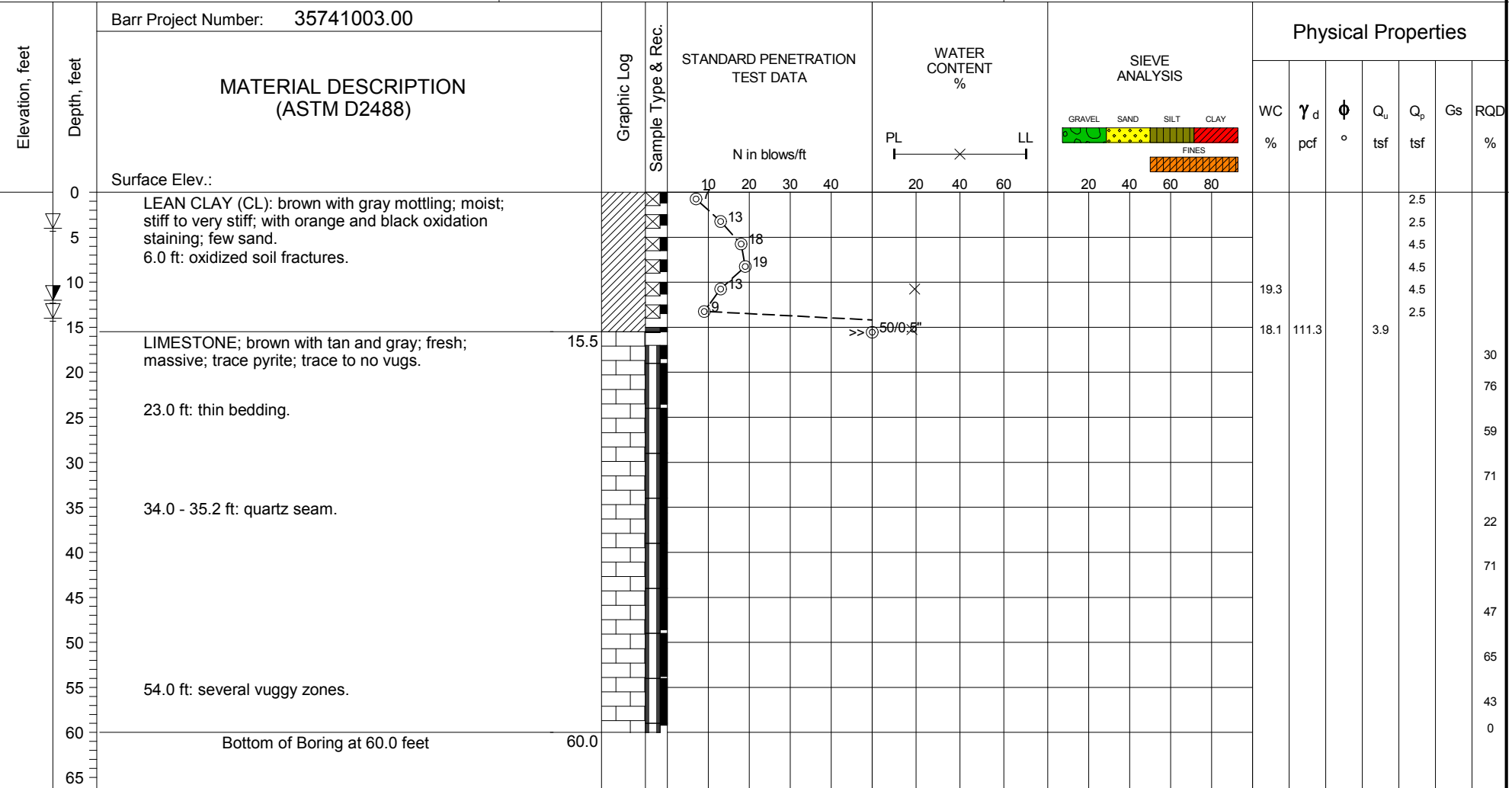
LOG OF BORING GEO-084

Sheet 1 of 1

Project: Seneca Wind Project

Location: Seneca County, Ohio

Client: sPower



Completion Depth: 60.0
Date Boring Started: 4/7/18
Date Boring Completed: 4/8/18
Logged By: DJZ / BWL
Drilling Contractor: EPC Engineering and Testing
Drilling Method: HSA and NQ Core
Ground Surface Elevation:
Coordinates: UTM 17 N:4542846.9m, E:324876.3m
Datum: NAD83

Remarks:

SAMPLE TYPES

SPLIT SPOON 3-inch Shelby Tube ROCK CORE

WATER LEVELS (ft)

At Time of Drilling 12.0
 After Drilling 14.0
 12 hrs After Drilling 4.0

LEGEND

MC Moisture Content Q_u Unconfined Compression
 γ Dry Unit Weight Q_p Hand Penetrometer UC
 ϕ Friction Angle Gs Specific Gravity
RQD Rock Quality Designation

The stratification lines represent approximate boundaries. The transition may be gradual.



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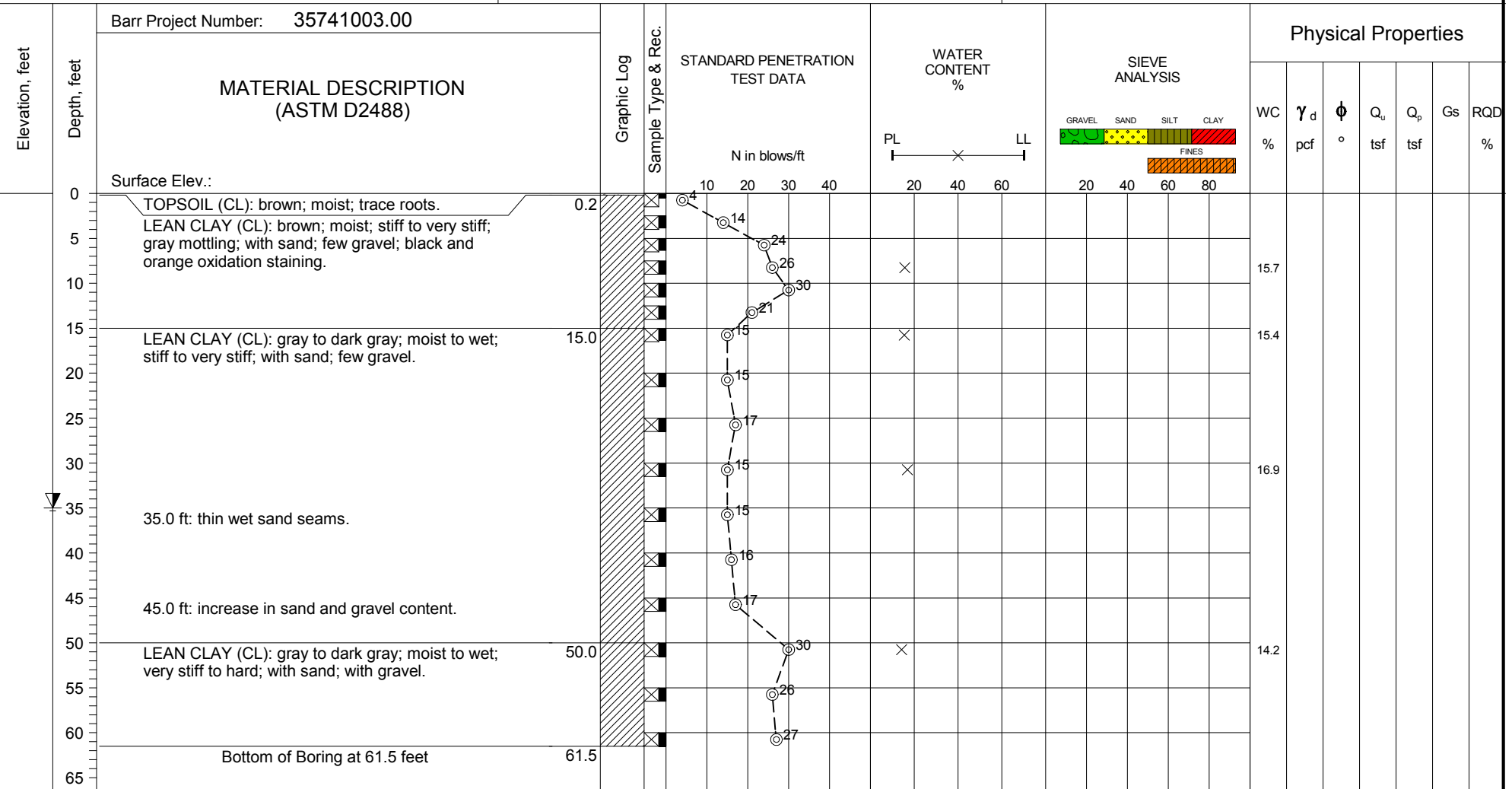
LOG OF BORING GEO-085

Sheet 1 of 1

Project: Seneca Wind Project

Location: Seneca County, Ohio

Client: sPower



Completion Depth: 61.5
Date Boring Started: 4/17/18
Date Boring Completed: 4/17/18
Logged By: DJZ
Drilling Contractor: EPC Engineering and Testing
Drilling Method: HSA
Ground Surface Elevation:
Coordinates: UTM 17 N:4547740.3m, E:327077.5m
Datum: NAD83

Remarks:

SAMPLE TYPES

⊠ SPLIT
⊠ SPOON

WATER LEVELS (ft)

▼ End of Drilling
▼ At Time of Drilling 35.0

LEGEND

MC Moisture Content
 γ Dry Unit Weight
 ϕ Friction Angle
 Q_u Unconfined Compression
 Q_p Hand Penetrometer UC
Gs Specific Gravity
RQD Rock Quality Designation

The stratification lines represent approximate boundaries. The transition may be gradual.



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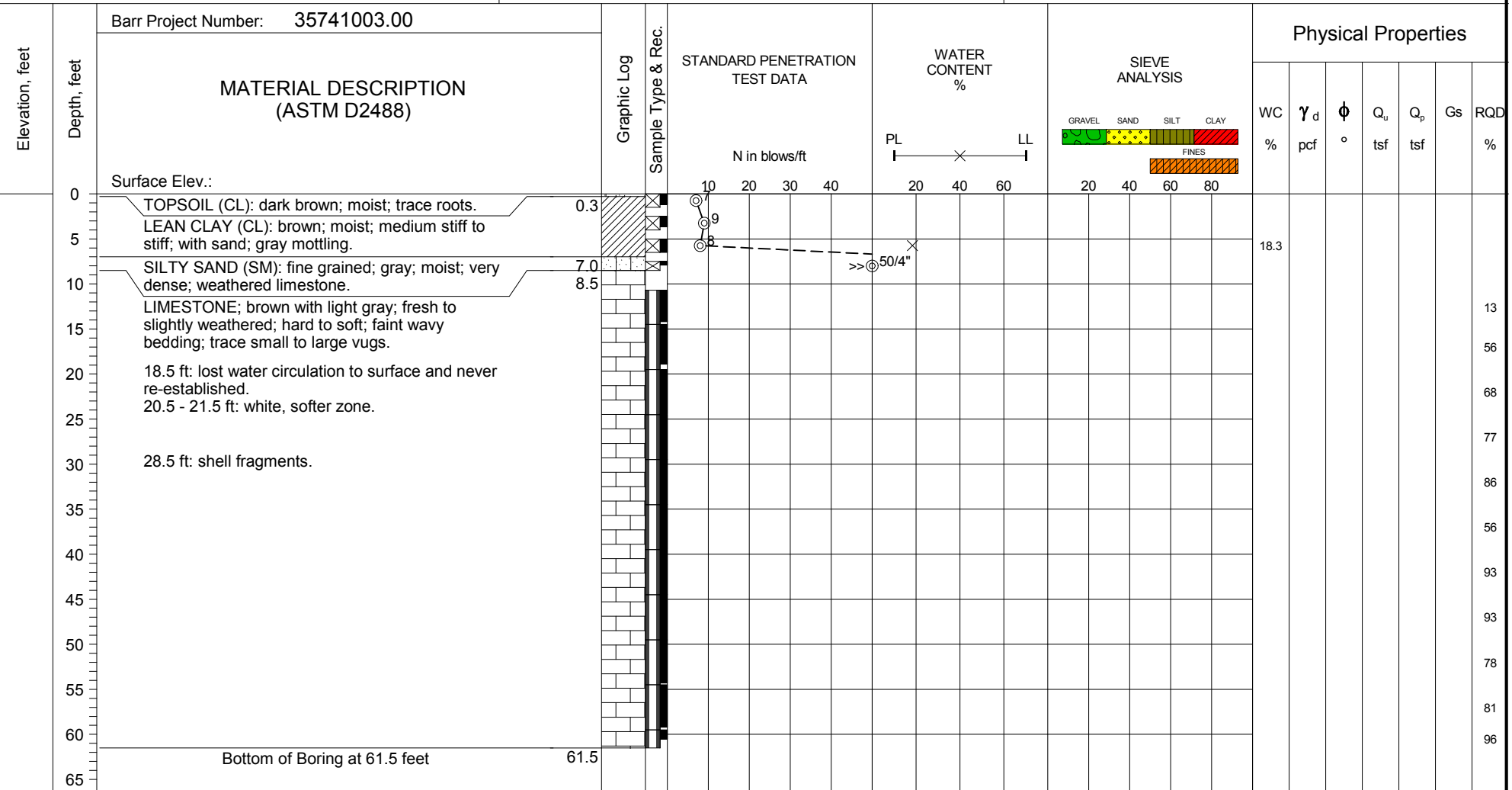
LOG OF BORING GEO-086

Sheet 1 of 1

Project: Seneca Wind Project

Location: Seneca County, Ohio

Client: sPower



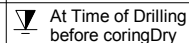
Completion Depth: 61.5
Date Boring Started: 4/12/18
Date Boring Completed: 4/12/18
Logged By: BAA / BWL
Drilling Contractor: EPC Engineering and Testing
Drilling Method: HSA and NQ Core
Ground Surface Elevation:
Coordinates: UTM 17 N:4545922.4m, E:327750.9m
Datum: NAD83

Remarks:

SAMPLE TYPES



WATER LEVELS (ft)



At Time of Drilling
before coringDry

LEGEND

MC	Moisture Content	Q_u	Unconfined Compression
γ	Dry Unit Weight	Q_p	Hand Penetrometer UC
ϕ	Friction Angle	Gs	Specific Gravity
		RQD	Rock Quality Designation

The stratification lines represent approximate boundaries. The transition may be gradual.



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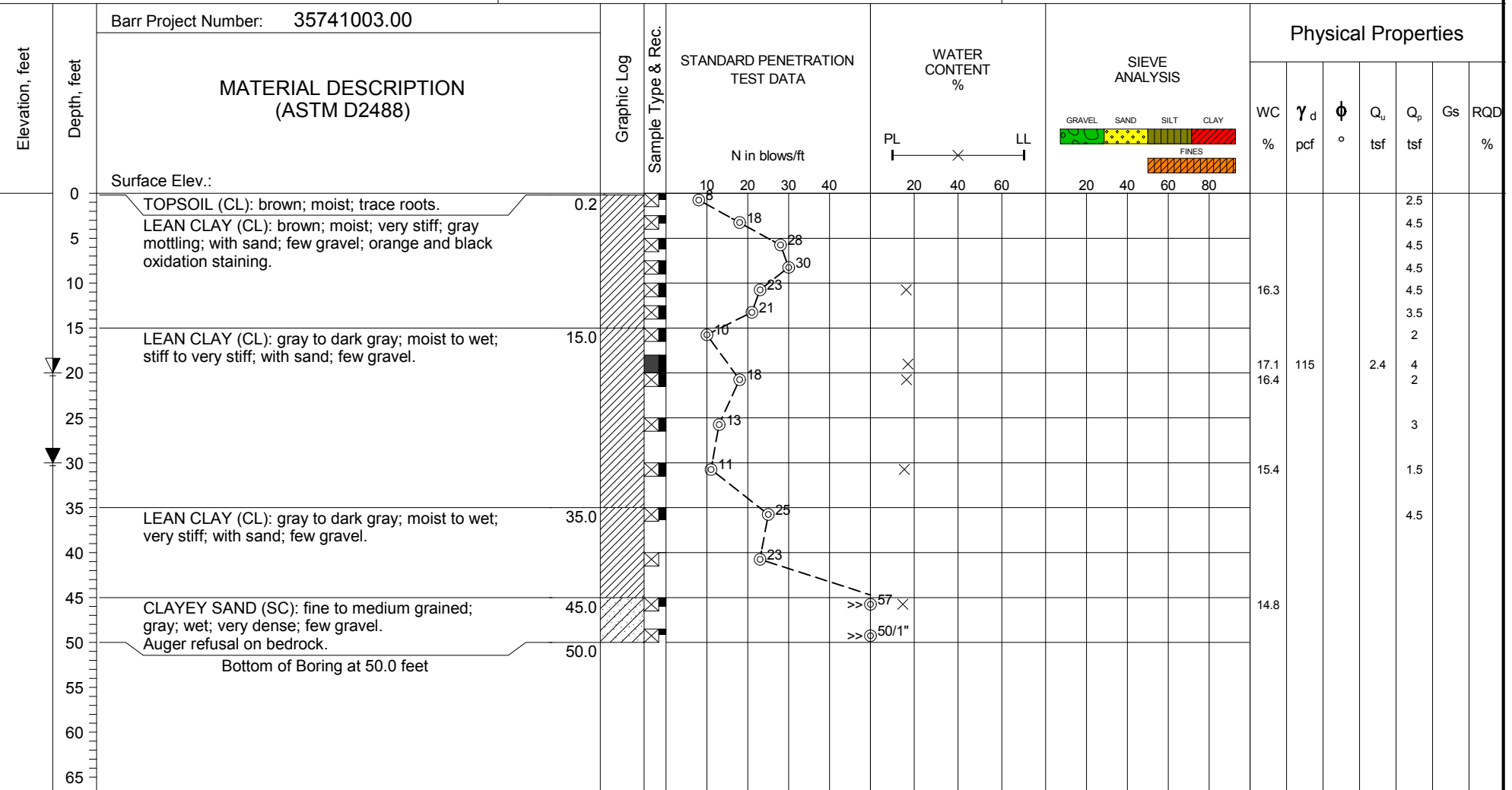
LOG OF BORING GEO-087

Sheet 1 of 1

Project: Seneca Wind Project

Location: Seneca County, Ohio

Client: sPower



Completion Depth: 50.0
Date Boring Started: 4/20/18
Date Boring Completed: 4/20/18
Logged By: DJZ
Drilling Contractor: EPC Engineering and Testing
Drilling Method: HSA
Ground Surface Elevation:
Coordinates: UTM 17 N:4547804.6m, E:327652.3m
Datum: NAD83

Remarks:

SAMPLE TYPES

⊗ SPLIT SPOON 3-inch Shelby Tube

WATER LEVELS (ft)

▼ End of Drilling 30.0
▼ At Time of Drilling 20.0

LEGEND

MC	Moisture Content	Q_u	Unconfined Compression
γ	Dry Unit Weight	Q_p	Hand Penetrometer UC
ϕ	Friction Angle	Gs	Specific Gravity
		RQD	Rock Quality Designation

The stratification lines represent approximate boundaries. The transition may be gradual.



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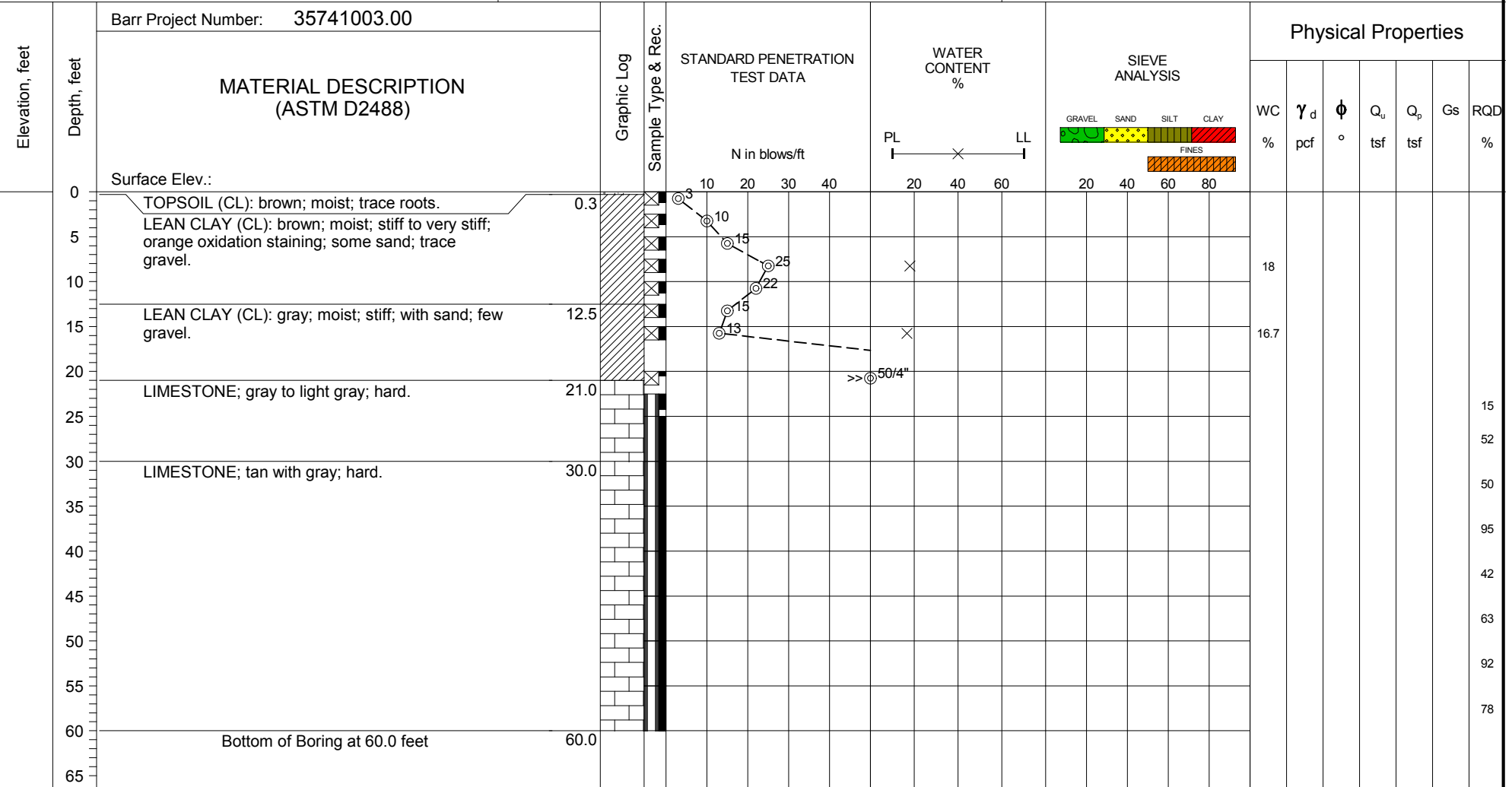
LOG OF BORING GEO-088

Sheet 1 of 1

Project: Seneca Wind Project

Location: Seneca County, Ohio

Client: sPower



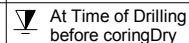
Completion Depth: 60.0
Date Boring Started: 4/14/18
Date Boring Completed: 4/15/18
Logged By: CRL / ZZ2
Drilling Contractor: EPC Engineering and Testing
Drilling Method: HSA and NQ Core
Ground Surface Elevation:
Coordinates: UTM 17 N:4545185.4m, E:329414.1m
Datum: NAD83

Remarks:

SAMPLE TYPES



WATER LEVELS (ft)



At Time of Drilling
before coringDry

LEGEND

MC	Moisture Content	Q_u	Unconfined Compression
γ	Dry Unit Weight	Q_p	Hand Penetrometer UC
ϕ	Friction Angle	Gs	Specific Gravity
		RQD	Rock Quality Designation

The stratification lines represent approximate boundaries. The transition may be gradual.



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LOG OF BORING GEO-089

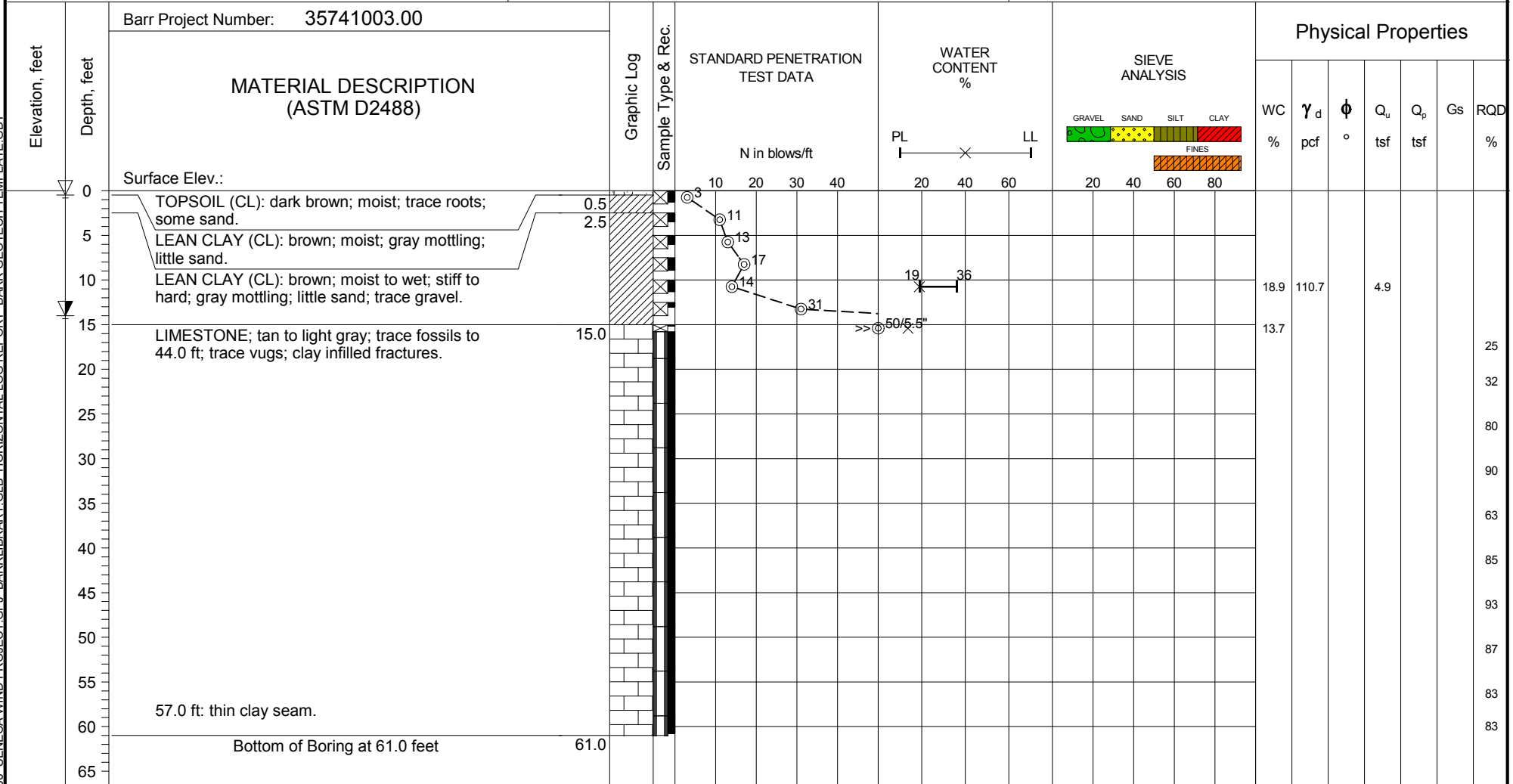
Sheet 1 of 1





Project: Seneca Wind Project

Location: Seneca County, Ohio

Client: sPower

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Completion Depth:	61.0	Remarks:					
Date Boring Started:	4/13/18						
Date Boring Completed:	4/14/18						
Logged By:	BAA / DJZ						
Drilling Contractor:	EPC Engineering and Testing						
Drilling Method:	HSA and NQ Core						
Ground Surface Elevation:							
Coordinates:	UTM 17 N:4545293.9m, E:328728.7m						
Datum:	NAD83						
		SAMPLE TYPES		WATER LEVELS (ft)		LEGEND	
		<div><div> SPLIT SPOON</div><div> ROCK CORE</div></div>		<div><div> After Drilling 0.5 before coring, sat overnight</div><div> At Time of Drilling 14.0</div></div>		<div><div>MC Moisture Content</div><div>γ Dry Unit Weight</div><div>ϕ Friction Angle</div></div> <div><div>Q_u Unconfined Compression</div><div>Q_p Hand Penetrometer UC</div><div>Gs Specific Gravity</div><div>RQD Rock Quality Designation</div></div>	

The stratification lines represent approximate boundaries. The transition may be gradual.



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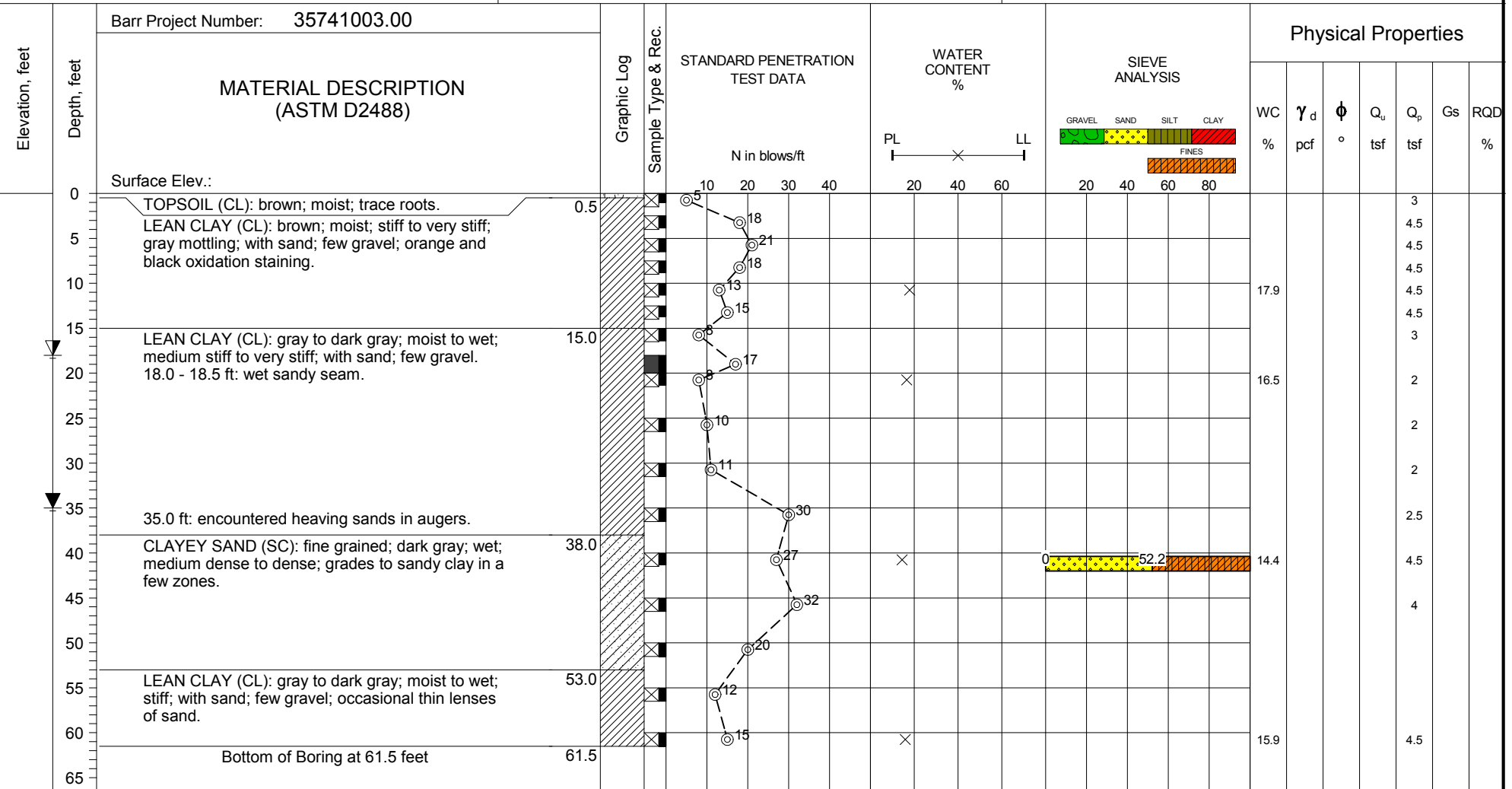
LOG OF BORING GEO-090

Sheet 1 of 1

Project: Seneca Wind Project

Location: Seneca County, Ohio

Client: sPower



Completion Depth: 61.5
Date Boring Started: 5/9/18
Date Boring Completed: 5/9/18
Logged By: DJZ
Drilling Contractor: EPC Engineering and Testing
Drilling Method: HSA
Ground Surface Elevation:
Coordinates: UTM 17 N:4550571.2m, E:339008.4m
Datum: NAD83

Remarks:

SAMPLE TYPES

⊠ SPLIT
SPOON 3-inch
Shelby Tube

WATER LEVELS (ft)

▼ End of Drilling 35.0
▼ At Time of Drilling 18.0

LEGEND

MC Moisture Content Q_u Unconfined Compression
 γ Dry Unit Weight Q_p Hand Penetrometer UC
 ϕ Friction Angle Gs Specific Gravity
RQD Rock Quality Designation






The stratification lines represent approximate boundaries. The transition may be gradual.



Sheet 1 of 1

Client: sPower



Remarks:			
SAMPLE TYPES		WATER LEVELS (ft)	LEGEND
 SPLIT SPOON  3-inch Shelby Tube	 Wet Cave-in Depth 48.0  End of Drilling  Dry At Time of Drilling 20.0	MC Moisture Content γ Dry Unit Weight ϕ Friction Angle	Q_u Unconfined Compression Q_p Hand Penetrometer UC Gs Specific Gravity RQD Rock Quality Designation

The stratification lines represent approximate boundaries. The transition may be gradual.



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LOG OF BORING GEO-092

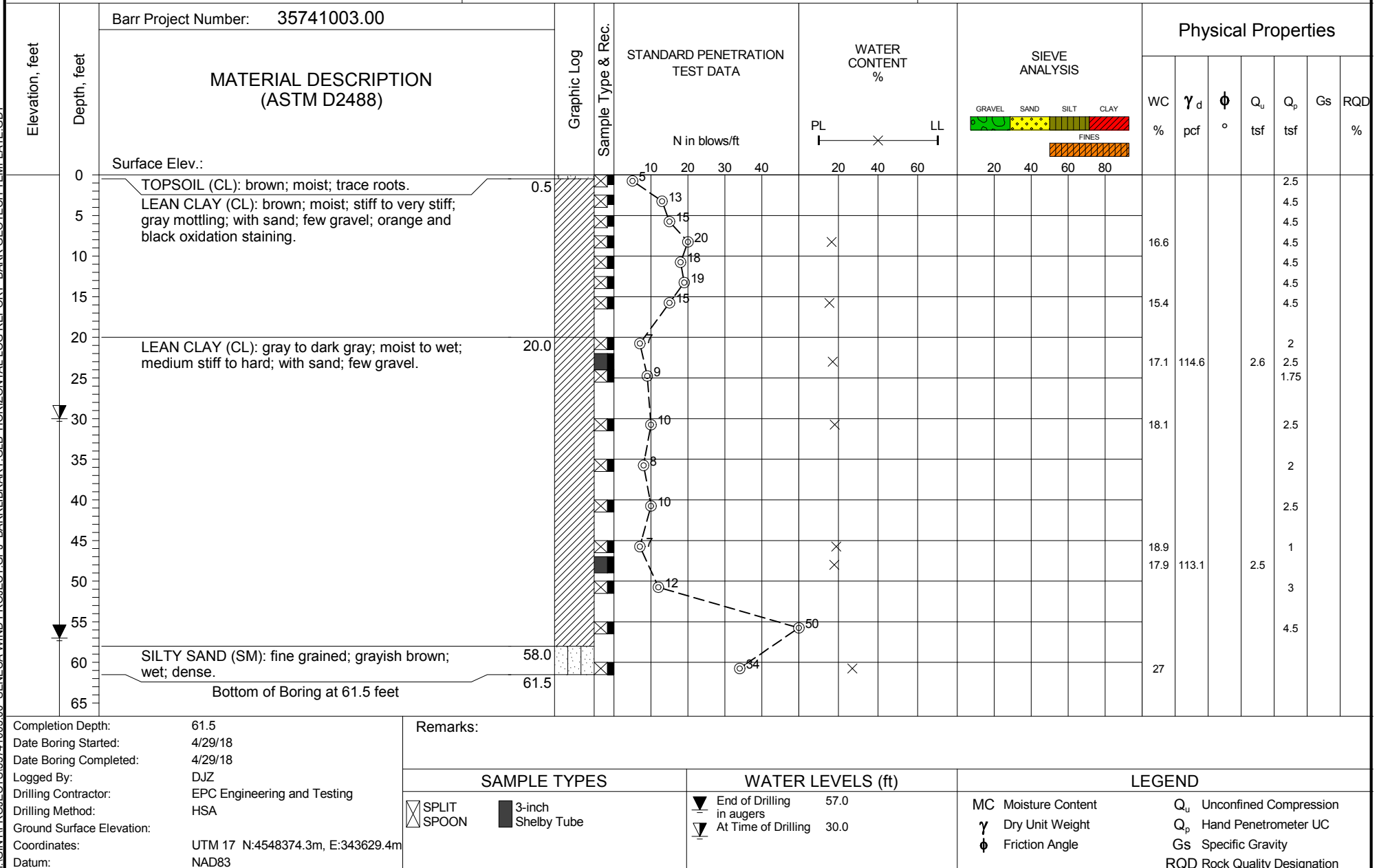
Sheet 1 of 1

Project: Seneca Wind Project

Location: Seneca County, Ohio

Client: sPower

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The stratification lines represent approximate boundaries. The transition may be gradual.



Sheet 1 of 1

Client: sPower



Remarks:					
SAMPLE TYPES		WATER LEVELS (ft)		LEGEND	
<div> <div></div> <div>SPLIT SPOON</div> </div> <div> <div></div> <div>3-inch Shelby Tube</div> </div>	<div> <div></div> <div>End of Drilling</div> <div>25.5</div> </div> <div> <div></div> <div>At Time of Drilling</div> <div>30.0</div> </div>	<div> <div>MC</div> <div>Moisture Content</div> </div> <div> <div>γ</div> <div>Dry Unit Weight</div> </div> <div> <div>ϕ</div> <div>Friction Angle</div> </div>			<div> <div>Q_u</div> <div>Unconfined Compression</div> </div> <div> <div>Q_p</div> <div>Hand Penetrometer UC</div> </div> <div> <div>Gs</div> <div>Specific Gravity</div> </div> <div> <div>RQD</div> <div>Rock Quality Designation</div> </div>

The stratification lines represent approximate boundaries. The transition may be gradual.



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Telephone: 952-832-2600

LOG OF BORING GEO-094

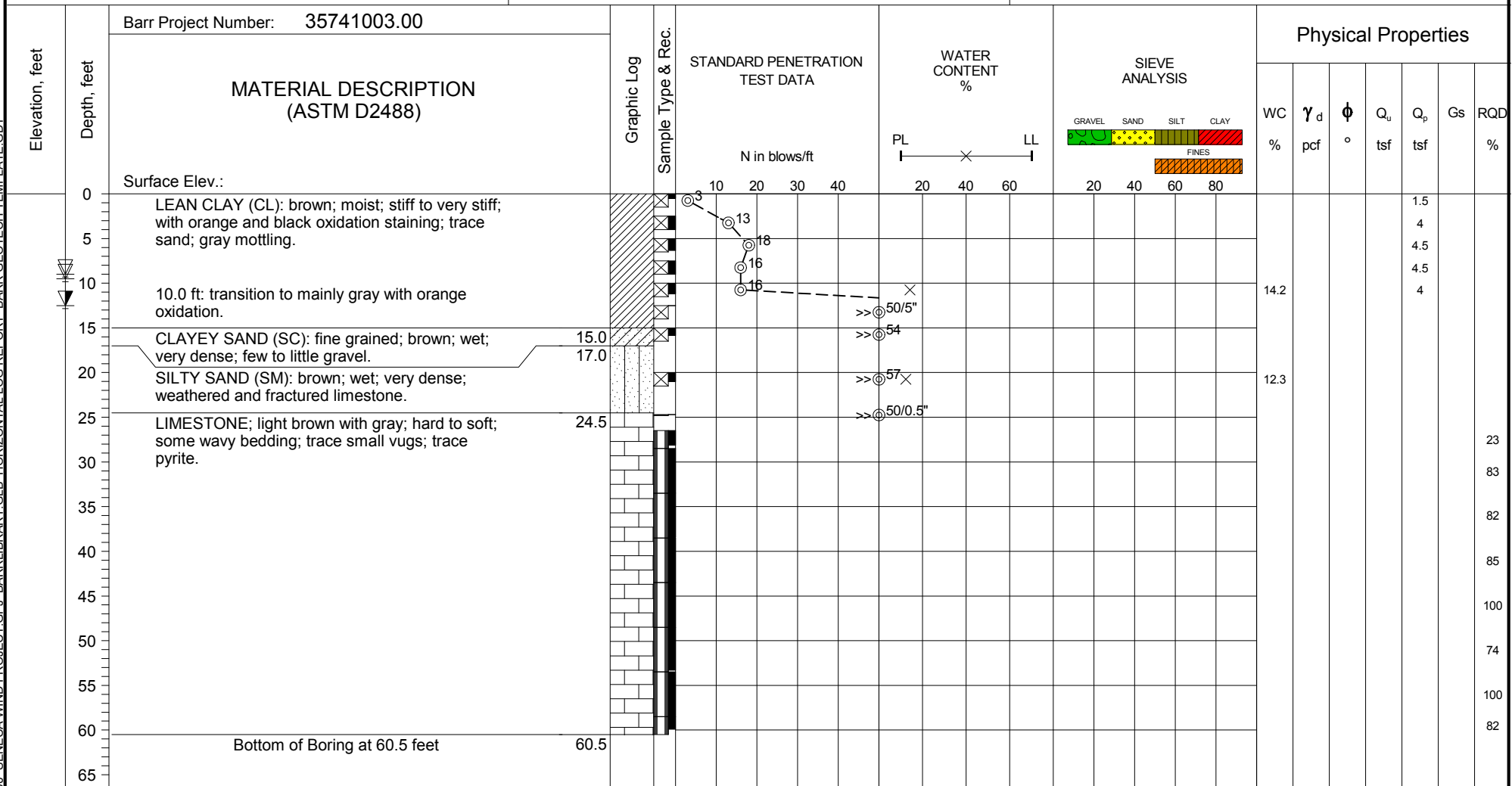
Sheet 1 of 1

Project: Seneca Wind Project

Location: Seneca County, Ohio

Client: sPower

M:\GINT\PROJECTS\35741003.00 - SENECA WIND PROJECT.GPJ - BARR\LIBRARY.GLB - HORIZONTAL LOG REPORT - BARR GEOTECH TEMPLATE.GDT



Completion Depth: 60.5
Date Boring Started: 4/8/18
Date Boring Completed: 4/9/18
Logged By: DJZ / BWL
Drilling Contractor: EPC Engineering and Testing
Drilling Method: HSA and NQ Core
Ground Surface Elevation: UTM 17 N:4542142.6m, E:325974.7m
Coordinates: UTM 17 N:4542142.6m, E:325974.7m
Datum: NAD83

Remarks:

SAMPLE TYPES

⊠ SPLIT SPOON █ ROCK CORE

WATER LEVELS (ft)

▼ At Time of Drilling 12.5
▼ After Drilling 9.5
▼ 12 hrs After Drilling 9.0

LEGEND

MC Moisture Content Q_u Unconfined Compression
 γ Dry Unit Weight Q_p Hand Penetrometer UC
 ϕ Friction Angle Gs Specific Gravity
RQD Rock Quality Designation

The stratification lines represent approximate boundaries. The transition may be gradual.



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Telephone: 952-832-2600

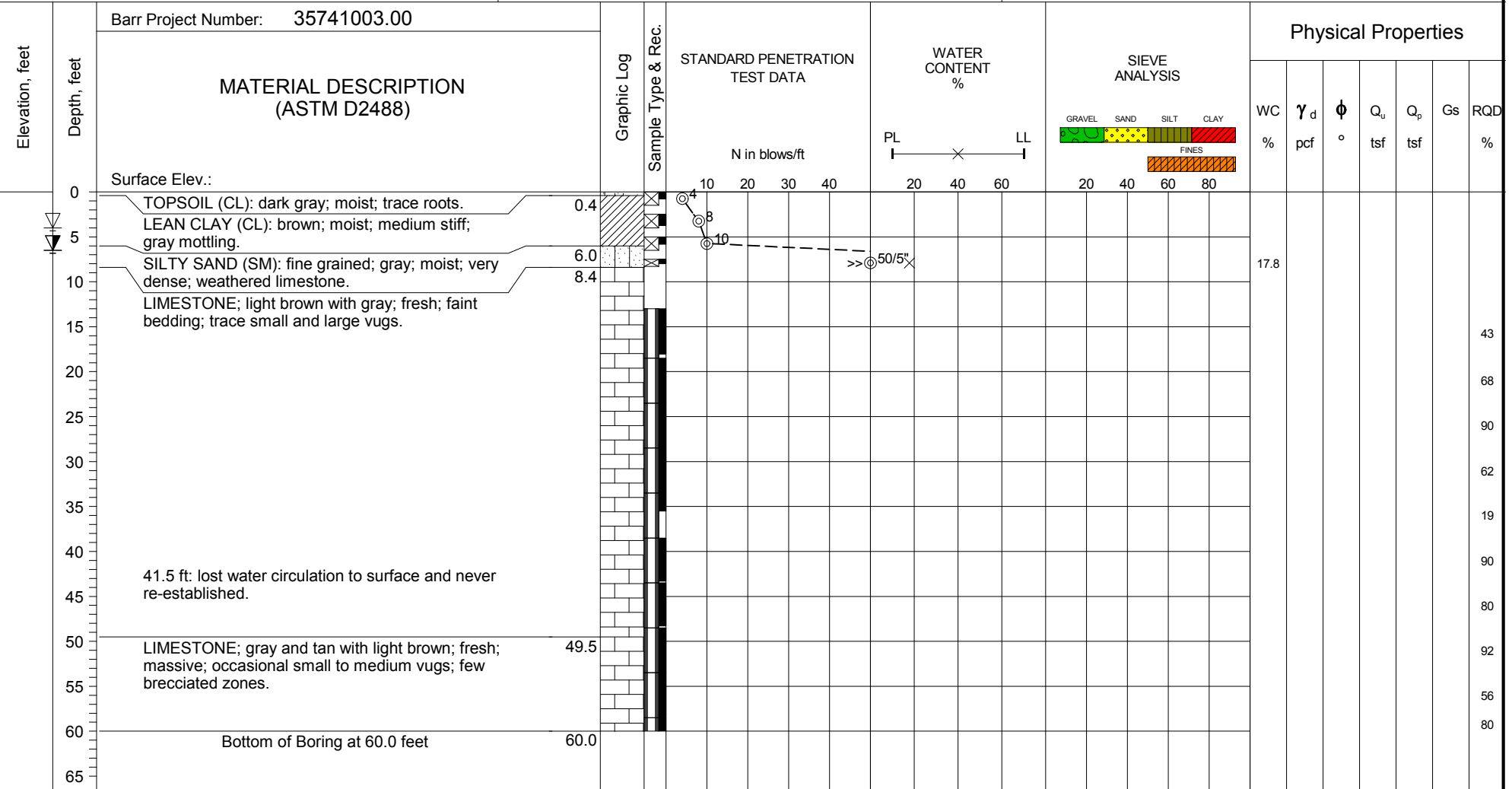
LOG OF BORING GEO-096

Sheet 1 of 1

Project: Seneca Wind Project

Location: Seneca County, Ohio

Client: sPower



Completion Depth:	60.0	<div>Remarks:</div> <table><tr><th colspan="2">SAMPLE TYPES</th><th colspan="2">WATER LEVELS (ft)</th><th colspan="2">LEGEND</th></tr><tr><td><div><div><div></div></div>SPLIT SPOON</div></td><td><div><div><div></div></div>ROCK CORE</div></td><td><div><div><div></div></div>After Drilling</div></td><td>6.5</td><td>MC Moisture Content</td><td>Q_u Unconfined Compression</td></tr><tr><td></td><td></td><td><div><div><div></div></div>At Time of Drilling</div></td><td>6.5</td><td>γ Dry Unit Weight</td><td>Q_p Hand Penetrometer UC</td></tr><tr><td></td><td></td><td><div><div><div></div></div>12 hrs After Drilling</div></td><td>4.0</td><td>ϕ Friction Angle</td><td>Gs Specific Gravity</td></tr><tr><td></td><td></td><td></td><td></td><td></td><td>RQD Rock Quality Designation</td></tr><tr><td></td><td></td><td></td><td></td><td></td><td></td></tr><tr><td></td><td></td><td></td><td></td><td></td><td></td></tr><tr><td></td><td></td><td></td><td></td><td></td><td></td></tr></table>	SAMPLE TYPES		WATER LEVELS (ft)		LEGEND		<div><div><div></div></div>SPLIT SPOON</div>	<div><div><div></div></div>ROCK CORE</div>	<div><div><div></div></div>After Drilling</div>	6.5	MC Moisture Content	Q_u Unconfined Compression			<div><div><div></div></div>At Time of Drilling</div>	6.5	γ Dry Unit Weight	Q_p Hand Penetrometer UC			<div><div><div></div></div>12 hrs After Drilling</div>	4.0	ϕ Friction Angle	Gs Specific Gravity						RQD Rock Quality Designation																		
SAMPLE TYPES			WATER LEVELS (ft)		LEGEND																																													
<div><div><div></div></div>SPLIT SPOON</div>	<div><div><div></div></div>ROCK CORE</div>		<div><div><div></div></div>After Drilling</div>	6.5	MC Moisture Content	Q_u Unconfined Compression																																												
			<div><div><div></div></div>At Time of Drilling</div>	6.5	γ Dry Unit Weight	Q_p Hand Penetrometer UC																																												
			<div><div><div></div></div>12 hrs After Drilling</div>	4.0	ϕ Friction Angle	Gs Specific Gravity																																												
						RQD Rock Quality Designation																																												
Date Boring Started:	4/10/18																																																	
Date Boring Completed:	4/11/18																																																	
Logged By:	BAA / BWL																																																	
Drilling Contractor:	EPC Engineering and Testing																																																	
Drilling Method:	HSA and NQ Core																																																	
Ground Surface Elevation:																																																		
Coordinates:	UTM 17 N:4543010.5m, E:326543.0m																																																	
Datum:	NAD83																																																	

The stratification lines represent approximate boundaries. The transition may be gradual.

Appendix C

Geophysical Survey Report

May 11, 2018

Robb J. Roy
BARR
Geotechnical Engineer
4700 West 77th Street
Minneapolis, MN 55435
(952) 842-3695



**RE: MASW and Seismic Refraction Survey
Seneca Wind Project
Seneca County, Ohio
THG Project No. 881-6820**

Dear Mr. Roy:

THG Geophysics, Ltd. (THG) collected seismic velocity data at the proposed Seneca Wind Project located in Seneca County, Ohio on April 30 - May 1, 2018 (Figure 1). The work scope included the collection of shear wave velocity data (s-wave) using multichannel analysis of surface wave methods (MASW) and compressional body-wave velocity data (p-wave) using a refraction method. THG conducted MASW shear-wave tests and seismic refraction tests at 6 turbine locations throughout the site.

MASW

Multichannel Analysis of Surface Waves (MASW) is a method of collecting shear-wave data using surface wave velocity analysis (Xia, et al., 2000). One-dimensional (MASW) data were collected at six locations (Figures 2-7). Elastic waves were initiated using a 16-pound sledge hammer striking a 10" by 10" aluminum plate. The velocity data were collected using a 24-channel, 4.5-Hz geophone spread with 32 geophones and a 5-foot array.

MASW theory holds that the penetration depth of a surface wave increases with wavelength. Further, propagation velocity (i.e., phase velocity) is determined mainly by shear-wave velocity of penetrated materials; consequently, surface waves have nearly the same velocity as shear wave at depth. Through the use of dispersion curves, or the change of propagation speed (i.e., phase velocity) with wavelength (or frequency), the shear wave velocity with depth can be derived. Four events (5 stacks each) were recorded and post-processed using SurfSeis 4.24. These data were then inverted to obtain the associated shear wave velocities (Figures 2-7).

REFRACTION

Seismic tomographic imaging is a refraction seismic method for showing p-wave velocity at depth in a profile format. A 24-channel (Geometrics Geode) seismograph using 10 Hz geophones collected p-wave velocity data with a 3-meter (10-ft) takeout distance. Five events on each seismic line were initiated with a 16-lb sledge hammer, collected and stacked (5 records) for post processing at each of the 6 locations (Figures 8-13).

Post-processing used of a proprietary processing program, RayFract®, a seismic refraction tomography software that provides reliable imaging of subsurface velocity. The "smoothin-version" algorithm tomographic method is based on physically meaningful modeling of seismic first break energy refraction, transmission and diffraction. Both p-wave and s-wave propagation is modeled with wave paths (also known as Fresnel volumes) instead of conventional seismic rays. This increases the numerical robustness of the method. A smooth initial 1-D gradient model is determined directly from the traveltimes data, by averaging Delta-t-V (improved Wiechert-Herglotz) method 1-D velocity-depth profiles along the seismic line. This procedure delivers reliable smoothed models even in case of velocity inversions. The initial model is then refined with true 2-D WET (Wavepath Eikonal Traveltime) tomography.

QUALITY ASSURANCE AND QUALITY CONTROL

The interpretation of geophysically-generated data is not an exact science since the responses to induced disturbance is affected by many phenomena including buried material, operator error, precipitation, wind and net changes in ground saturation conditions. Some sources of spurious data can be overcome through a QA/QC program and use of multiple geophysical methods. The quality control program employed with this study included frequent checks of the equipment and resurveys of lines and locations. The QA/QC program indicates that all geophysical equipment functioned as designed during the survey program.

ANALYSIS

Geophysical data collected at the Seneca Wind Project site was of good quality. S-wave velocities range from approximately 600 ft/s to 4,500 ft/s and p-wave velocities from 3,000 ft/s to 14,500 ft/s (Table 1). The resulting velocities can be grouped into two different categories at the site. Turbine locations GEO-013, GEO-033, GEO-061, and GEO-085 exhibit slower velocities at shallower depths and GEO-003 and GEO-066 exhibit much faster velocities at shallower depths indicating shallower bedrock at turbine locations GEO-003 and GEO-066.

MASW data at all six turbine test locations were used to perform seismic hazard classification (IBS, 2018). All six sites meet the specification of a Site Class C: Very dense soil and soft rock (Table 2).

Should you have any questions or require additional information, please contact our office at (724) 325-3996 or via e-mail hk@thggeophysics.com.

Respectfully,
THG Geophysics, Ltd.



Heather Krivos, G.I.T.
Senior Geophysicist

Enclosures

REFERENCES

IBS, (2018). International Building Code; International Code Council, Inc.

Xia, J.; Miller, R. D.; Park, C. B.; Hunter J. A.; Harris, J. B.; and Ivanov, J., (2002), Comparing shear-wave velocity profiles inverted from multi-channel surface wave with borehole measurements: *Soil Dynamics Earthquake Engineering*, Vol. 22, pp. 181-190.

Geophysical investigations are a non-invasive method of interpreting physical properties of the shallow earth using electrical, electromagnetic, or mechanical energy. This document contains geophysical interpretations of responses to induced or real-world phenomena. As such, the measured phenomenon may be impacted by variables not readily identified in the field that can result in a false-positive and/or false-negative interpretation. THG makes no representations or warranties as to the accuracy of the interpretations.

Table 1

Seneca Wind Project
Seneca County, Ohio

s-wave and p-wave Velocity Table

DEPTH (ft below grade)	GEO-003		GEO-013		GEO-033	
	s-wave velocity (ft/sec)	p-wave velocity (ft/sec)	s-wave velocity (ft/sec)	p-wave velocity (ft/sec)	s-wave velocity (ft/sec)	p-wave velocity (ft/sec)
-10	987	3,169	1,530	3,126	782	3,992
-20	955	6,125	1,357	3,499	673	5,344
-30	1,299	7,903	1,186	4,076	1,021	5,647
-40	1,629	10,121	1,413	4,579	1,021	5,865
-50	1,842	11,844	2,050	5,190	1,360	6,312
-60	2,163	13,365	2,050	6,113	1,564	
-70	2,163	14,644	2,403	7,450	1,564	
-80	2,691		2,403	8,671	1,651	
-90	2,691		2,297	9,705	1,651	
-100	4,529		2,297		2,070	
-110	4,529		2,981		2,070	
-120	4,529		2,981			

DEPTH (ft below grade)	GEO-061		GEO-066		GEO-085	
	s-wave velocity (ft/sec)	p-wave velocity (ft/sec)	s-wave velocity (ft/sec)	p-wave velocity (ft/sec)	s-wave velocity (ft/sec)	p-wave velocity (ft/sec)
-10	830	3,803	2,251	6,209	830	4,232
-20	617	4,519	2,299	8,340	643	5,431
-30	1,067	4,597	2,439		810	5,018
-40	1,762	5,007	2,492		1,353	5,444
-50	1,984	5,923	2,272		1,629	6,079
-60	1,746	6,618	1,833		1,576	6,861
-70	1,746	7,585	1,833		1,576	8,371
-80	1,584	8,489	1,784		1,573	9,252
-90	1,584		1,784		1,753	
-100	3,123		3,399		1,753	
-110	3,123		3,399		2,846	
-120	3,123		3,399		2,846	

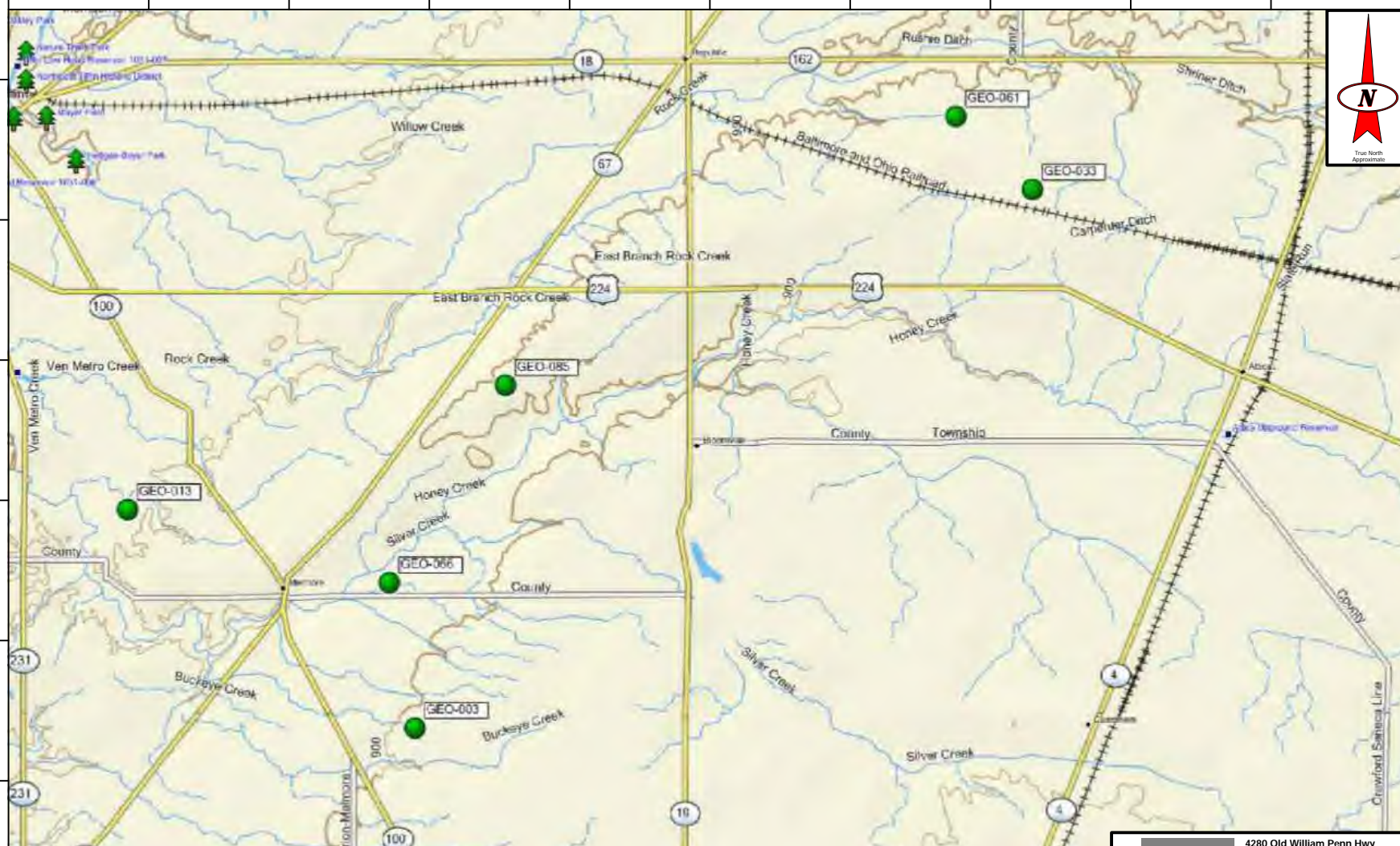
Table 2

**Seneca Wind Project
Seneca County, Ohio**

MASW Seismic Site Classification

GEO-003			GEO-013			GEO-033		
Depth (ft)	Velocity (ft/sec)	Weight	Depth (ft)	Velocity (ft/sec)	Weight	Depth (ft)	Velocity (ft/sec)	Weight
-3.8	1,059	0.04	-4.1	1,542	0.04	-3.6	807	0.04
-8.6	1,128	0.05	-9.2	1,585	0.05	-8.1	831	0.05
-14.6	987	0.06	-15.6	1,530	0.06	-13.8	782	0.06
-22.1	955	0.08	-23.6	1,357	0.08	-20.8	673	0.07
-31.4	1,299	0.09	-33.6	1,186	0.10	-29.7	685	0.09
-43.1	1,629	0.12	-46.1	1,413	0.13	-40.7	1,021	0.11
-57.7	1,842	0.15	-61.7	2,050	0.16	-54.5	1,360	0.14
-76.0	2,163	0.18	-81.2	2,403	0.20	-71.7	1,564	0.17
-98.8	2,691	0.23	-100.0	2,297	0.19	-93.2	1,651	0.22
-100.0	4,529	0.01				-100.0	2,070	0.07
Weighted Average (ft/sec)		1,869	Weighted Average (ft/sec)		1,866	Weighted Average (ft/sec)		1,284

GEO-061			GEO-066			GEO-085		
Depth (ft)	Velocity (ft/sec)	Weight	Depth (ft)	Velocity (ft/sec)	Weight	Depth (ft)	Velocity (ft/sec)	Weight
-3.7	894	0.04	-3.8	2,328	0.04	-3.9	944	0.04
-8.4	918	0.05	-8.5	2,303	0.05	-8.7	952	0.05
-14.2	830	0.06	-14.4	2,251	0.06	-14.8	830	0.06
-21.4	617	0.07	-21.8	2,299	0.07	-22.3	643	0.08
-30.5	1,067	0.09	-31.1	2,439	0.09	-31.8	810	0.10
-41.9	1,762	0.11	-42.7	2,492	0.12	-43.6	1,353	0.12
-56.1	1,984	0.14	-57.1	2,272	0.14	-58.4	1,629	0.15
-73.8	1,746	0.18	-75.2	1,833	0.18	-76.9	1,576	0.19
-96.0	1,584	0.22	-97.8	1,784	0.23	-100.0	1,573	0.23
-100.0	3,123	0.04	-100.0	3,399	0.02			
Weighted Average (ft/sec)		1,534	Weighted Average (ft/sec)		2,152	Weighted Average (ft/sec)		1,314



Legend

● Geophysical Test Location

0 mi 1.5 mi 3 mi 4.5 mi

Notes

Geophysical survey conducted April 30 - May 1, 2018.

MASW data collected using Geometrics Geode 24-channel seismograph, 4.5 Hz geophones, 5-ft stepout and SurfSeis 4.24 software.

Seismic refraction data collected using Geometrics Geode 24-channel seismograph, 10 Hz geophones, 10-foot stepout and RayFract 3.15 software.

DRN	HLK	5/8/18
DES	HLK	5/8/18
CHK	PJH	5/10/18
REV		
PROJ. MGR.	HLK	5/8/18

SCALE: As Noted

SOURCE:

PREPARED FOR:

BARR

PROJECT:

Geophysical Investigation
Seneca Wind Project
Seneca Co, Ohio

DRAWING NO.

**Figure 1
Site Layout**

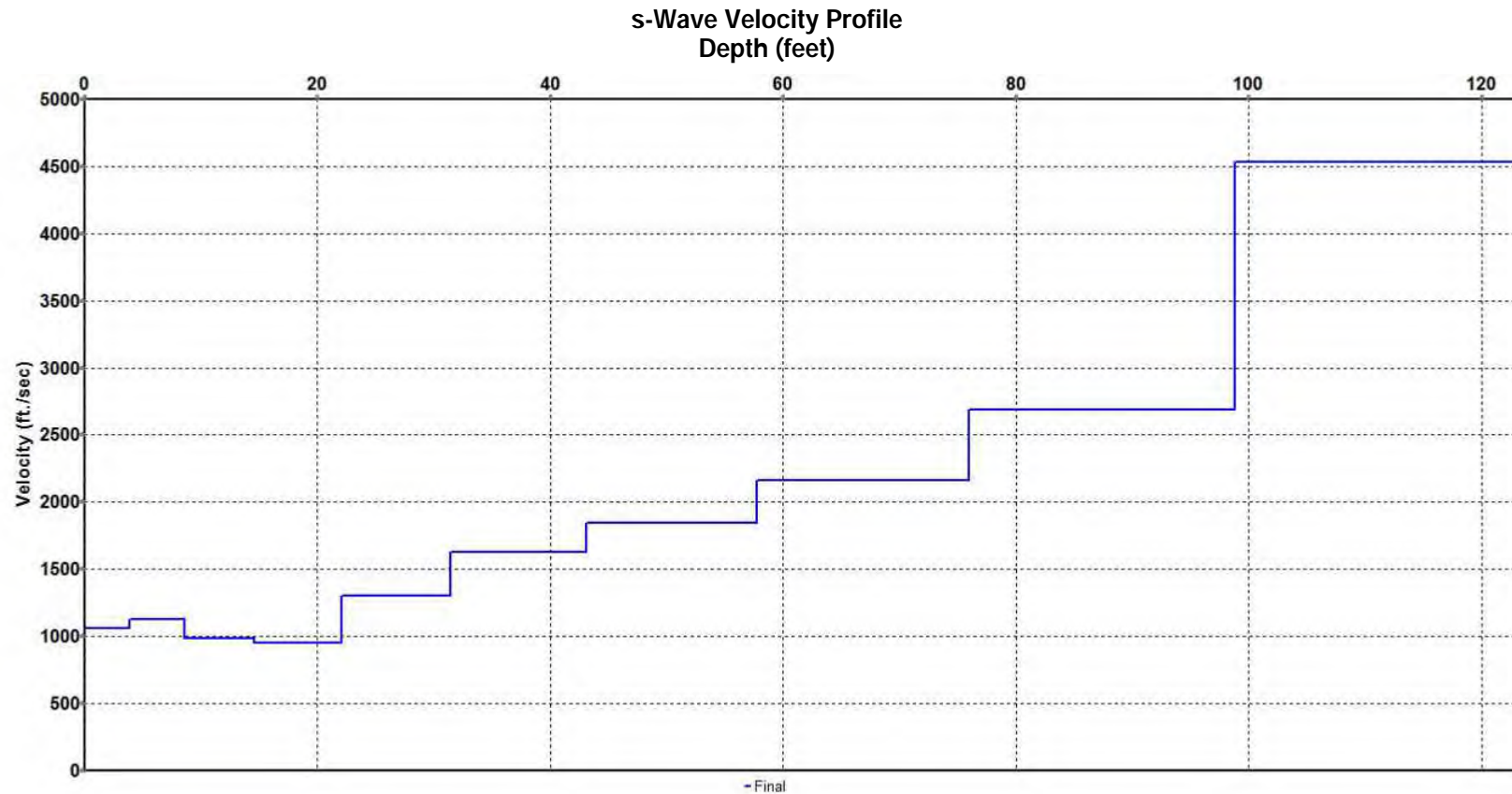
PROJECT NO.

881-6820

SHEET TITLE:

DWG6820F1

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Murrysville, Pennsylvania 15668
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s-WAVE VELOCITY

Depth (ft) Velocity (ft/s)

-3.8	1,059
-8.6	1,128
-14.6	987
-22.1	955
-31.4	1,299
-43.1	1,629
-57.7	1,842
-76.0	2,163
-98.8	2,691
-123.5	4,529

Notes

Geophysical survey conducted April 30 - May 1, 2018.

MASW data collected using Geometrics Geode 24-channel seismograph, 4.5 Hz geophones, 5-ft stepout and SurfSeis 4.24 software.

Seismic refraction data collected using Geometrics Geode 24-channel seismograph, 10 Hz geophones, 10-foot stepout and RayFract 3.15 software.



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DRN	HLK	5/8/18
DES	HLK	5/8/18
CHK	PJH	5/10/18
REV		
PROJ. MGR.	HLK	5/8/18

SCALE: As Noted

SOURCE:

PREPARED FOR:



PROJECT:

Geophysical Investigation
Seneca Wind Project
Seneca Co., Ohio

DRAWING NO:

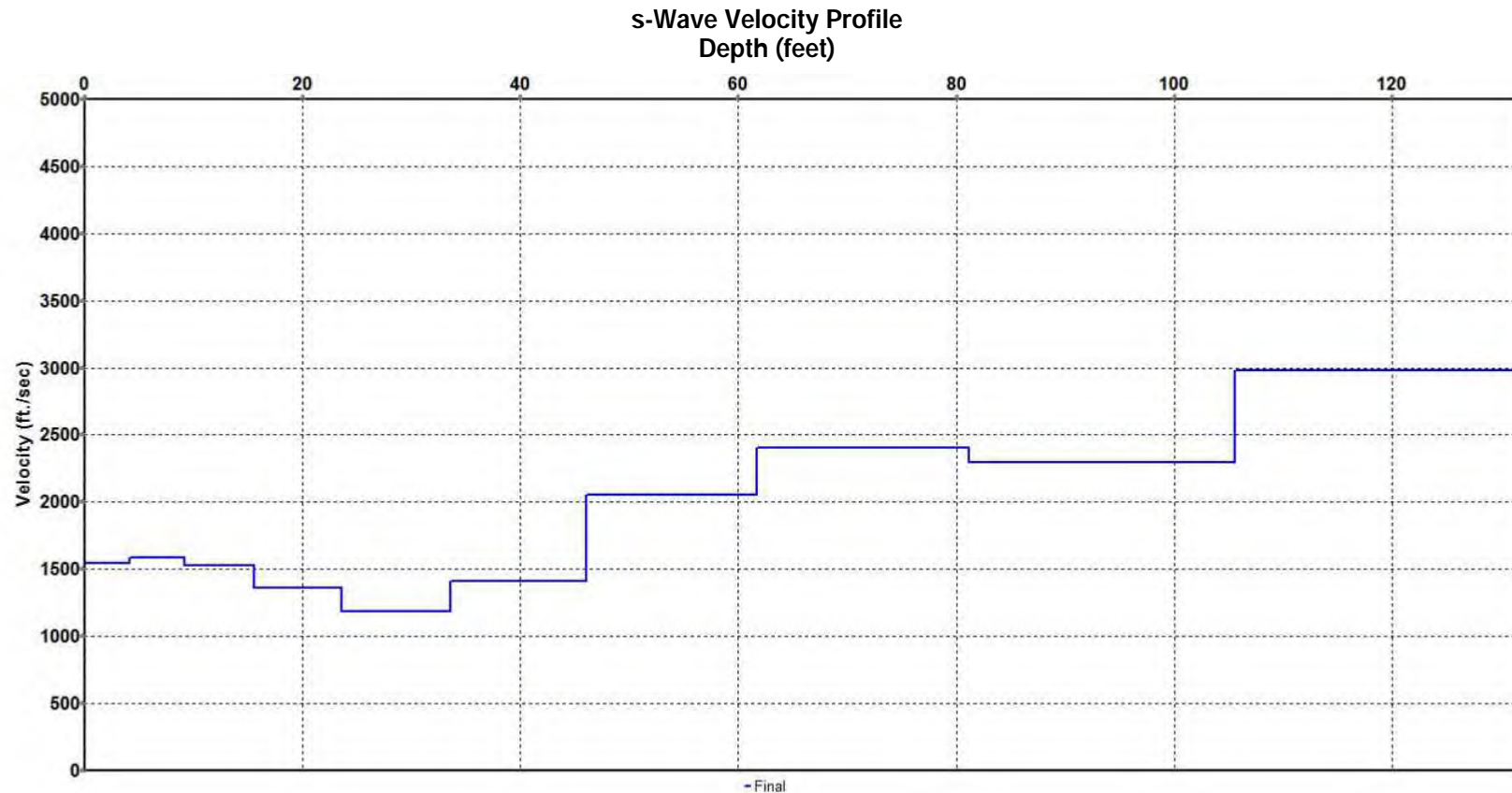
Figure 2
s-Wave Velocity Profile
GEO-003

PROJECT NO:

881-6820

SHEET TITLE:

DWG6820F2



s-WAVE VELOCITY

Depth (ft) Velocity (ft/s)

-4.1	1,542
-9.2	1,585
-15.6	1,530
-23.6	1,357
-33.6	1,186
-46.1	1,413
-61.7	2,050
-81.2	2,403
-105.6	2,297
-132.0	2,981

Notes

Geophysical survey conducted April 30 - May 1, 2018.

MASW data collected using Geometrics Geode 24-channel seismograph, 4.5 Hz geophones, 5-ft stepout and SurfSeis 4.24 software.

Seismic refraction data collected using Geometrics Geode 24-channel seismograph, 10 Hz geophones, 10-foot stepout and RayFract 3.15 software.

DRN	HLK	5/8/18
DES	HLK	5/8/18
CHK	PJH	5/10/18
REV		
PROJ. MGR.	HLK	5/8/18

SCALE: As Noted

SOURCE:

PREPARED FOR:



PROJECT:

Geophysical Investigation
Seneca Wind Project
Seneca Co., Ohio

DRAWING NO:

Figure 3
s-Wave Velocity Profile
GEO-013

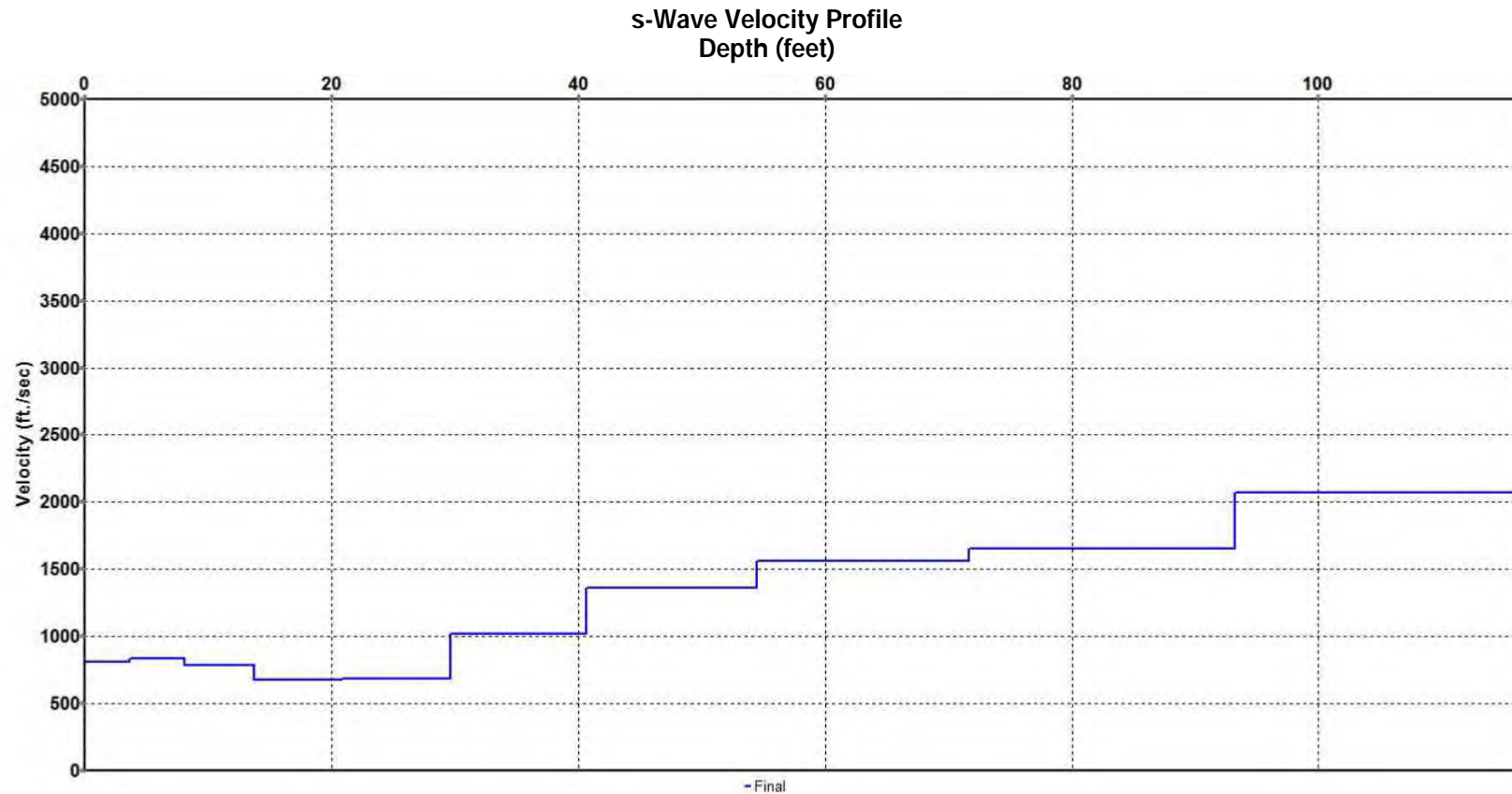
PROJECT NO:

881-6820

SHEET TITLE:

DWG6820F3

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s-WAVE VELOCITY

Depth (ft) Velocity (ft/s)

-3.6	807
-8.1	831
-13.8	782
-20.8	673
-29.7	685
-40.7	1,021
-54.5	1,360
-71.7	1,564
-93.2	1,651
-116.5	2,070

Notes

Geophysical survey conducted April 30 - May 1, 2018.

MASW data collected using Geometrics Geode 24-channel seismograph, 4.5 Hz geophones, 5-ft stepout and SurfSeis 4.24 software.

Seismic refraction data collected using Geometrics Geode 24-channel seismograph, 10 Hz geophones, 10-foot stepout and RayFract 3.15 software.

DRN	HLK	5/8/18
DES	HLK	5/8/18
CHK	PJH	5/10/18
REV		
PROJ. MGR.	HLK	5/8/18

SCALE: As Noted

SOURCE:

PREPARED FOR:



PROJECT:

Geophysical Investigation
Seneca Wind Project
Seneca Co., Ohio

DRAWING NO.:

Figure 4
s-Wave Velocity Profile
GEO-033

PROJECT NO.:

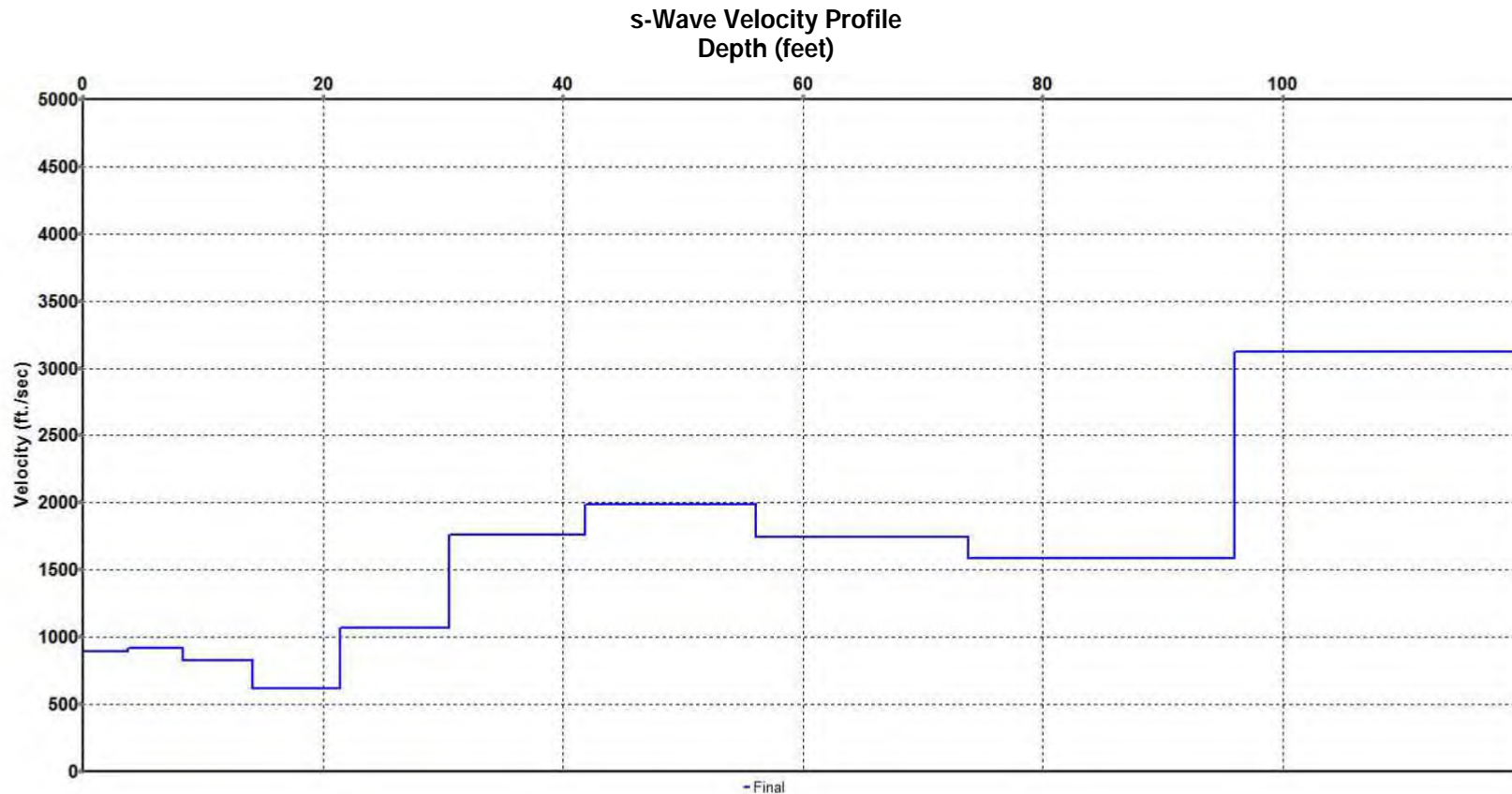
881-6820

SHEET TITLE:

DWG6820F4



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s-WAVE VELOCITY

Depth (ft) Velocity (ft/s)

-3.7	894
-8.4	918
-14.2	830
-21.4	617
-30.5	1,067
-41.9	1,762
-56.1	1,984
-73.8	1,746
-96.0	1,584
-120.0	3,123

Notes

Geophysical survey conducted April 30 - May 1, 2018.

MASW data collected using Geometrics Geode 24-channel seismograph, 4.5 Hz geophones, 5-ft stepout and SurfSeis 4.24 software.

Seismic refraction data collected using Geometrics Geode 24-channel seismograph, 10 Hz geophones, 10-foot stepout and RayFract 3.15 software.

DRN	HLK	5/8/18
DES	HLK	5/8/18
CHK	PJH	5/10/18
REV		
PROJ. MGR.	HLK	5/8/18

SCALE: As Noted

SOURCE:

PREPARED FOR:



PROJECT:

Geophysical Investigation
Seneca Wind Project
Seneca Co., Ohio

DRAWING NO.:

Figure 5
s-Wave Velocity Profile
GEO-061

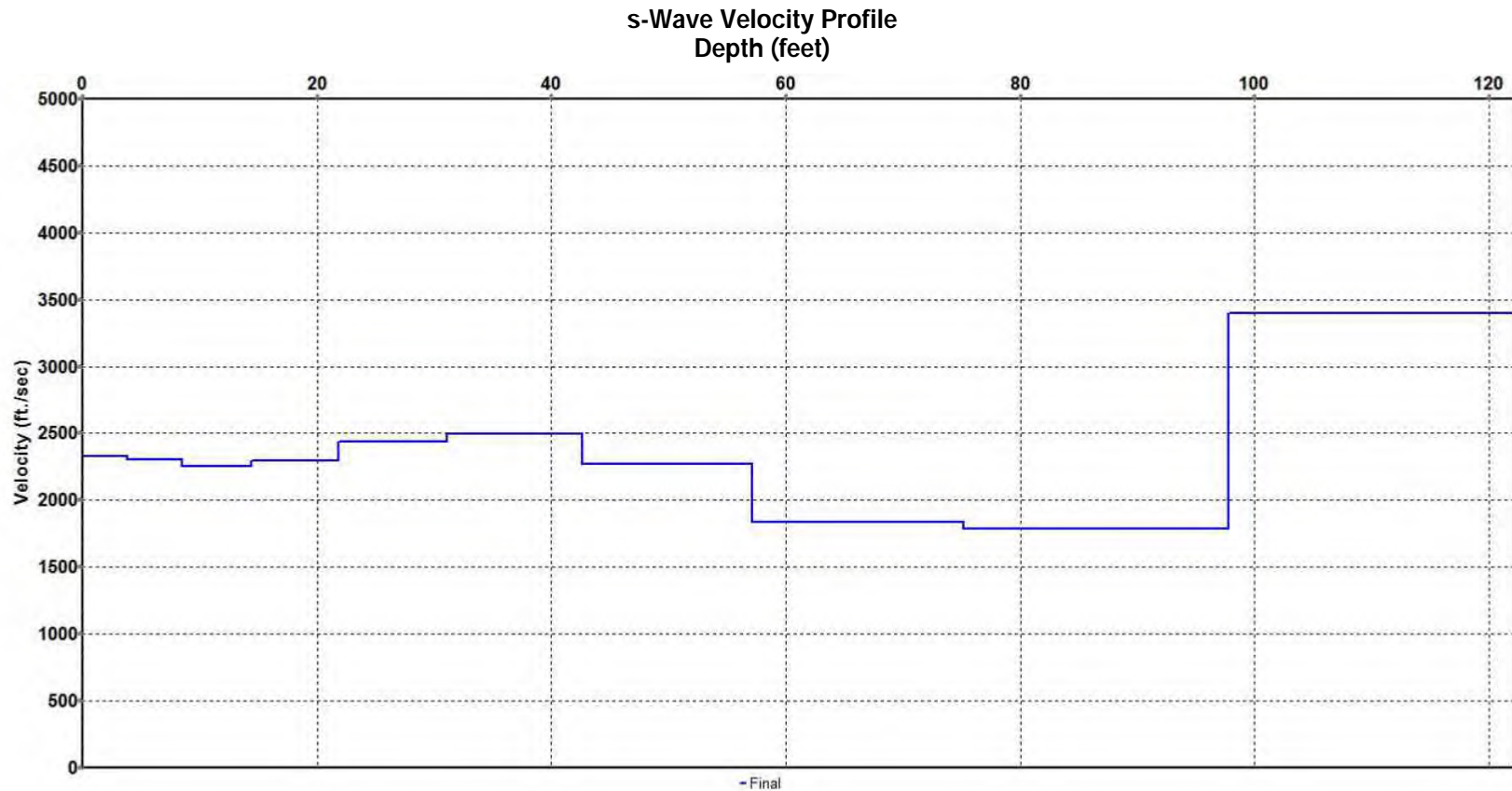
PROJECT NO.:

881-6820

SHEET TITLE:

DWG6820F5

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s-WAVE VELOCITY

Depth (ft) Velocity (ft/s)

-3.8	2,328
-8.5	2,303
-14.4	2,251
-21.8	2,299
-31.1	2,439
-42.7	2,492
-57.1	2,272
-75.2	1,833
-97.8	1,784
-122.2	3,399

Notes

Geophysical survey conducted April 30 - May 1, 2018.

MASW data collected using Geometrics Geode 24-channel seismograph, 4.5 Hz geophones, 5-ft stepout and SurfSeis 4.24 software.

Seismic refraction data collected using Geometrics Geode 24-channel seismograph, 10 Hz geophones, 10-foot stepout and RayFract 3.15 software.

DRN	HLK	5/8/18
DES	HLK	5/8/18
CHK	PJH	5/10/18
REV		
PROJ. MGR.	HLK	5/8/18

SCALE: As Noted

SOURCE:

PREPARED FOR:



PROJECT:

Geophysical Investigation
Seneca Wind Project
Seneca Co., Ohio

DRAWING NO:

Figure 6
s-Wave Velocity Profile
GEO-066

PROJECT NO:

881-6820

SHEET TITLE:

DWG6820F6



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s-WAVE VELOCITY

Depth (ft) Velocity (ft/s)

-3.9	944
-8.7	952
-14.8	830
-22.3	643
-31.8	810
-43.6	1,353
-58.4	1,629
-76.9	1,576
-100.0	1,573
-125.0	2,846

Notes

Geophysical survey conducted April 30 - May 1, 2018.

MASW data collected using Geometrics Geode 24-channel seismograph, 4.5 Hz geophones, 5-ft stepout and SurfSeis 4.24 software.

Seismic refraction data collected using Geometrics Geode 24-channel seismograph, 10 Hz geophones, 10-foot stepout and RayFract 3.15 software.

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DRN	HLK	5/8/18
DES	HLK	5/8/18
CHK	PJH	5/10/18
REV		
PROJ. MGR.	HLK	5/8/18

SCALE: As Noted

SOURCE:

PREPARED FOR:

PROJECT:

Geophysical Investigation
Seneca Wind Project
Seneca Co., Ohio

DRAWING NO:

Figure 7
s-Wave Velocity Profile
GEO-085

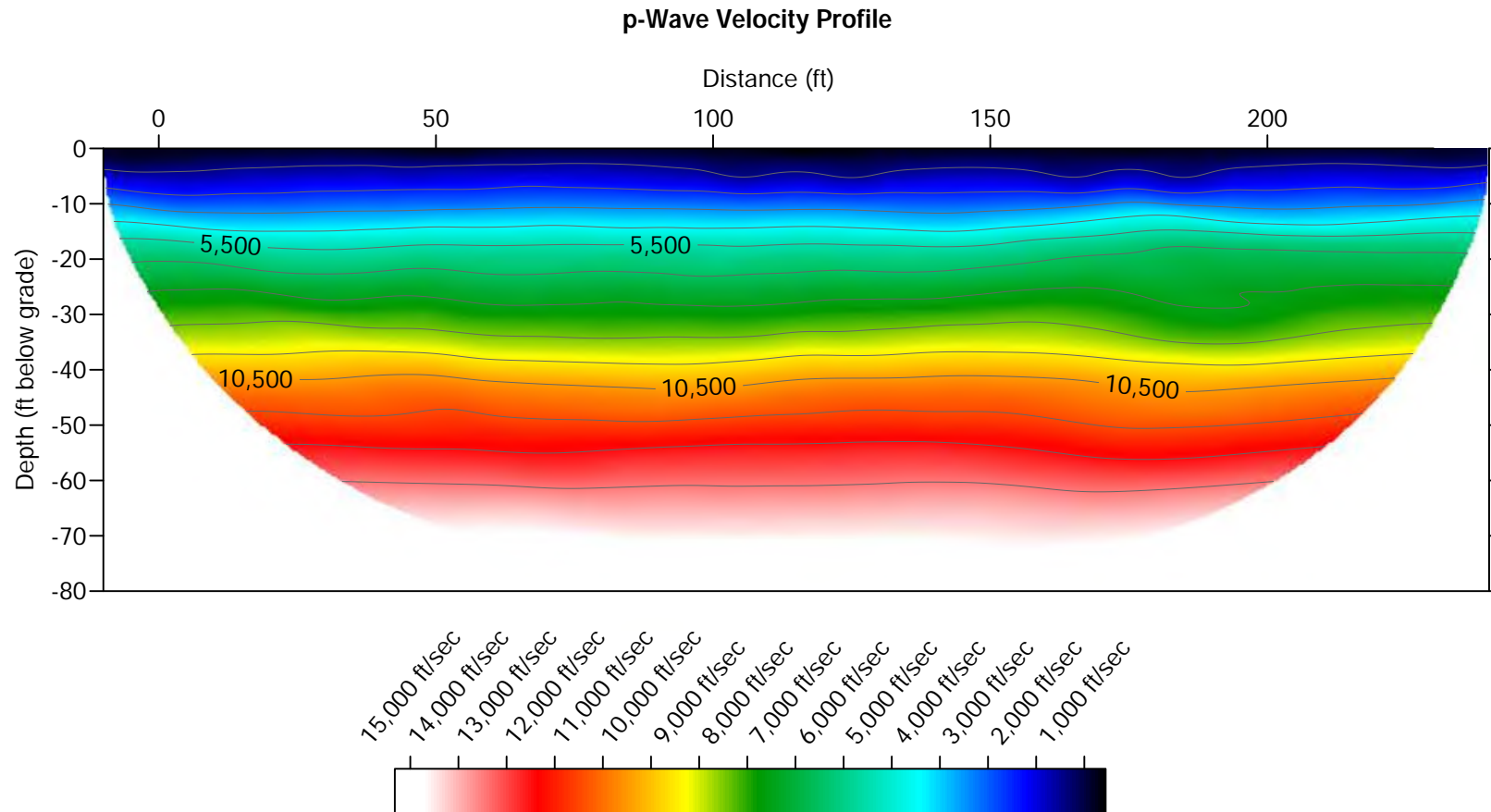
PROJECT NO:

881-6820

SHEET TITLE:

DWG6820F7





p-WAVE VELOCITY

Depth (ft) Velocity (ft/s)

-10	3,169
-20	6,125
-30	7,903
-40	10,121
-50	11,844
-60	13,365
-70	14,644

Notes

Geophysical survey conducted April 30 - May 1, 2018.

MASW data collected using Geometrics Geode 24-channel seismograph, 4.5 Hz geophones, 5-ft stepout and SurfSeis 4.24 software.

Seismic refraction data collected using Geometrics Geode 24-channel seismograph, 10 Hz geophones, 10-foot stepout and RayFract 3.15 software.

DRN	HLK	5/8/18
DES	HLK	5/8/18
CHK	PJH	5/10/18
REV		
PROJ. MGR.	HLK	5/8/18

SCALE: As Noted

SOURCE:

PREPARED FOR:



PROJECT:

Geophysical Investigation
Seneca Wind Project
Seneca Co, Ohio

DRAWING NO:

Figure 8
p-Wave Velocity Profile
GEO-003

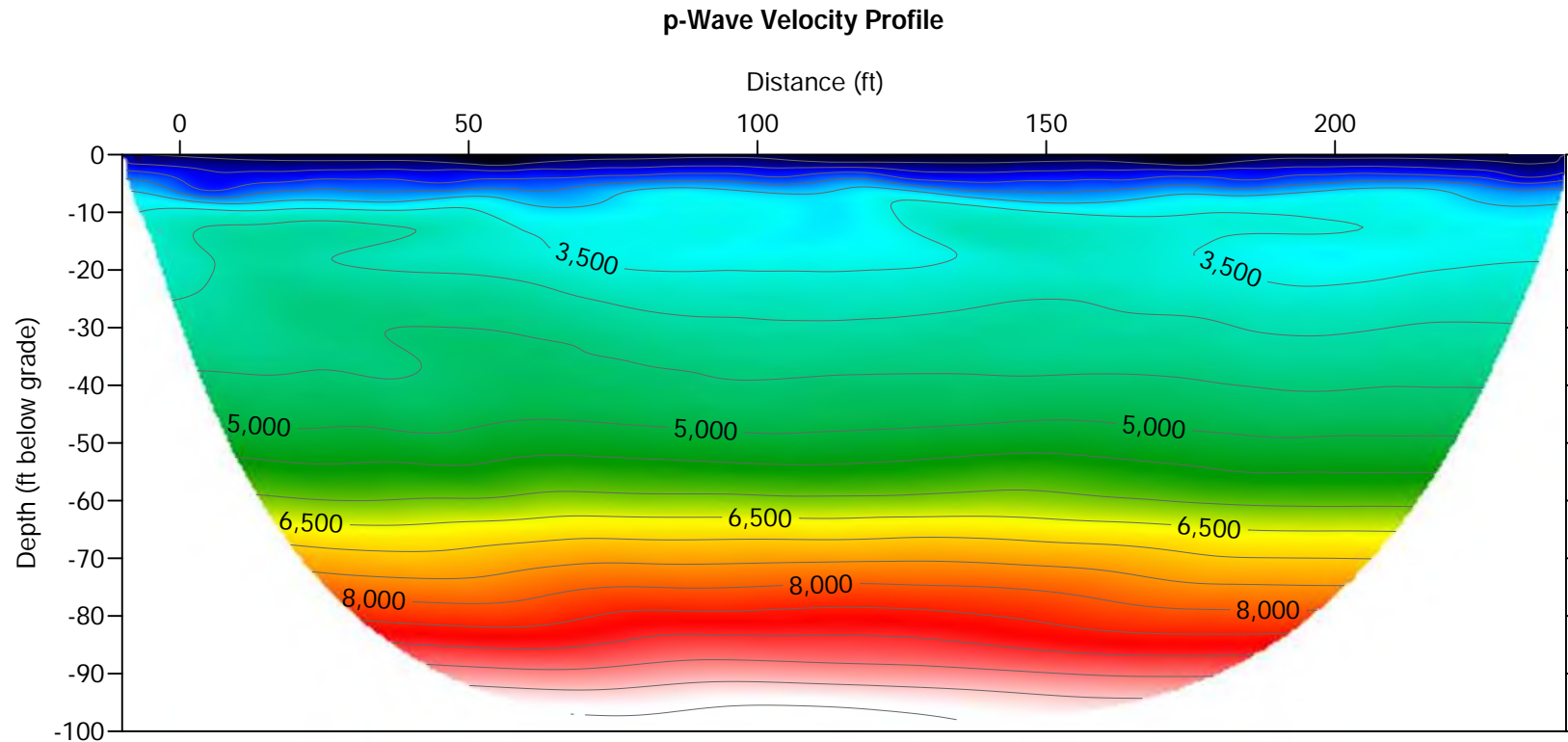
PROJECT NO:

881-6820

SHEET TITLE:

DWG6820F8

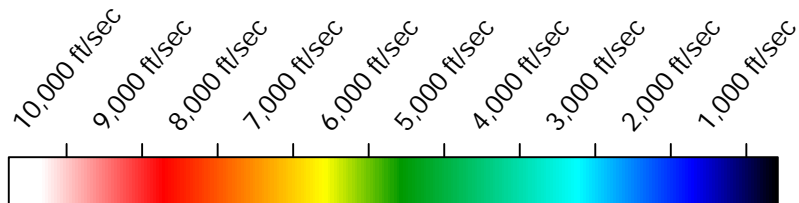
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p-WAVE VELOCITY

Depth (ft) Velocity (ft/s)

-10	3,126
-20	3,499
-30	4,076
-40	4,579
-50	5,190
-60	6,113
-70	7,450
-80	8,671
-90	9,705



Notes

Geophysical survey conducted April 30 - May 1, 2018.

MASW data collected using Geometrics Geode 24-channel seismograph, 4.5 Hz geophones, 5-ft stepout and SurfSeis 4.24 software.

Seismic refraction data collected using Geometrics Geode 24-channel seismograph, 10 Hz geophones, 10-foot stepout and RayFract 3.15 software.


DRN	HLK	5/8/18
DES	HLK	5/8/18
CHK	PJH	5/10/18
REV		
PROJ. MGR.	HLK	5/8/18

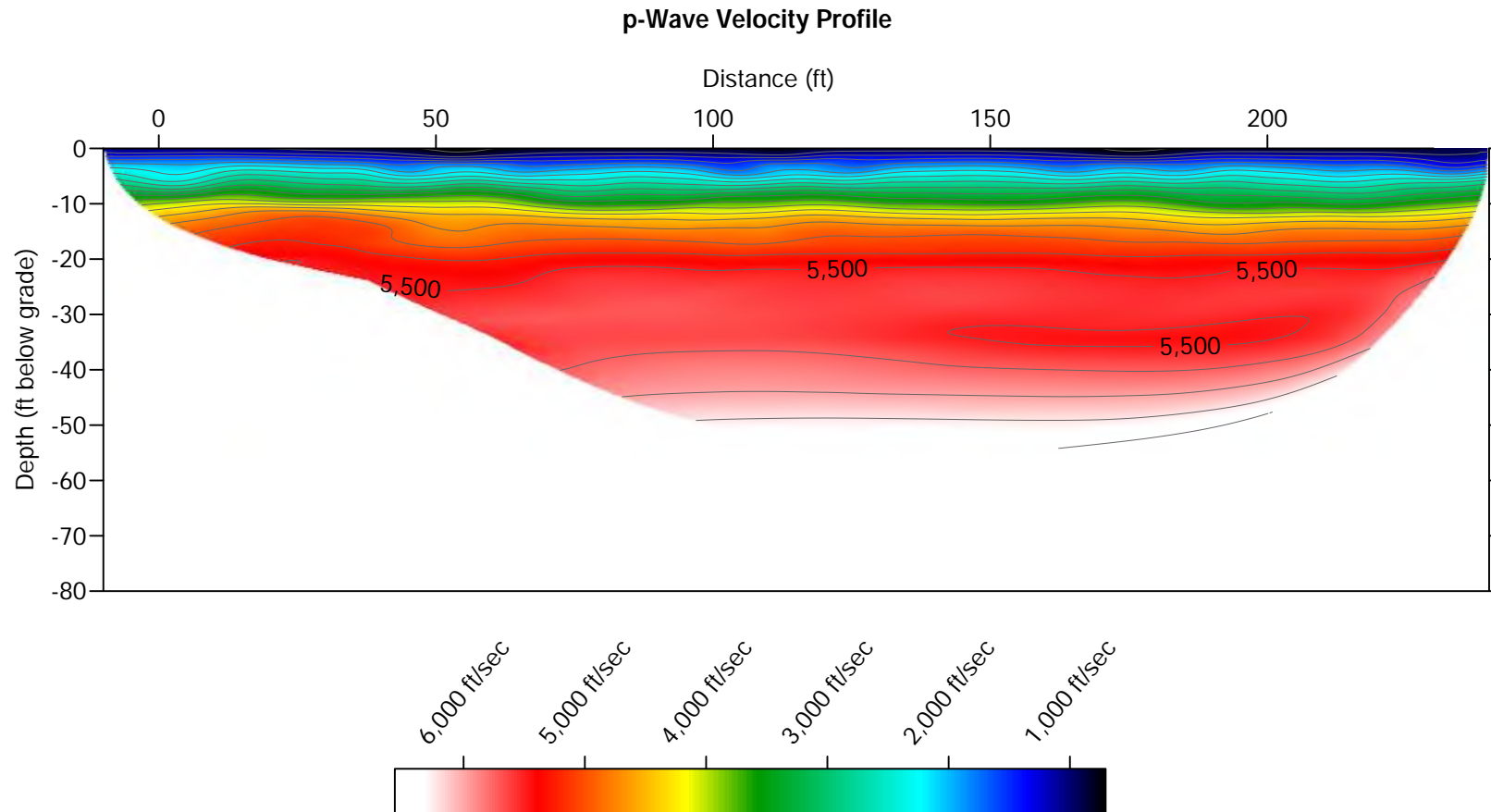
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SOURCE:

PREPARED FOR:



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PROJECT: Geophysical Investigation Seneca Wind Project Seneca Co, Ohio	
DRAWING NO: Figure 9 p-Wave Velocity Profile GEO-013	
PROJECT NO: 881-6820	SHEET TITLE: DWG6820F9



p-WAVE VELOCITY

Depth (ft) Velocity (ft/s)

-10	3,992
-20	5,344
-30	5,647
-40	5,865
-50	6,312

Notes

Geophysical survey conducted April 30 - May 1, 2018.

MASW data collected using Geometrics Geode 24-channel seismograph, 4.5 Hz geophones, 5-ft stepout and SurfSeis 4.24 software.

Seismic refraction data collected using Geometrics Geode 24-channel seismograph, 10 Hz geophones, 10-foot stepout and RayFract 3.15 software.

DRN	HLK	5/8/18
DES	HLK	5/8/18
CHK	PJH	5/10/18
REV		
PROJ. MGR.	HLK	5/8/18

SCALE: As Noted

SOURCE:

PREPARED FOR:



PROJECT:

Geophysical Investigation
Seneca Wind Project
Seneca Co, Ohio

DRAWING NO.

Figure 10
p-Wave Velocity Profile
GEO-033

PROJECT NO.

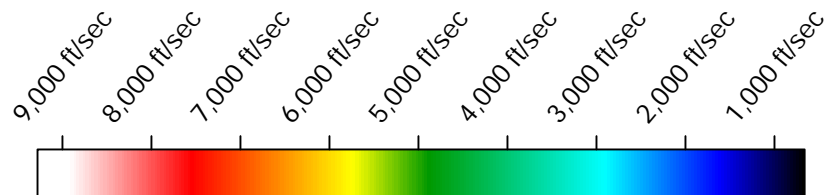
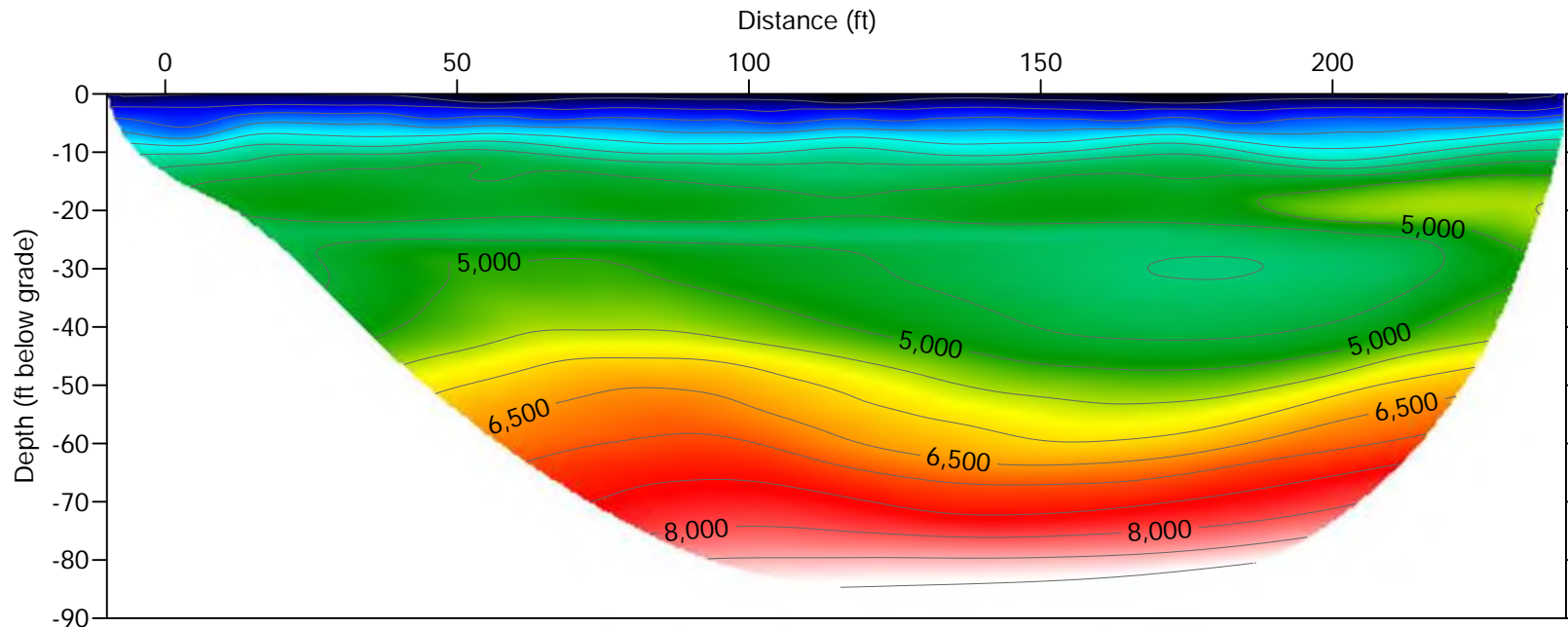
881-6820

SHEET TITLE:

DWG6820F10

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p-Wave Velocity Profile



p-WAVE VELOCITY

Depth (ft) Velocity (ft/s)

-10	3,803
-20	4,519
-30	4,597
-40	5,007
-50	5,923
-60	6,618
-70	7,585
-80	8,489

Notes

Geophysical survey conducted April 30 - May 1, 2018.

MASW data collected using Geometrics Geode 24-channel seismograph, 4.5 Hz geophones, 5-ft stepout and SurfSeis 4.24 software.

Seismic refraction data collected using Geometrics Geode 24-channel seismograph, 10 Hz geophones, 10-foot stepout and RayFract 3.15 software.


DRN	HLK	5/8/18
DES	HLK	5/8/18
CHK	PJH	5/10/18
REV		
PROJ. MGR.	HLK	5/8/18

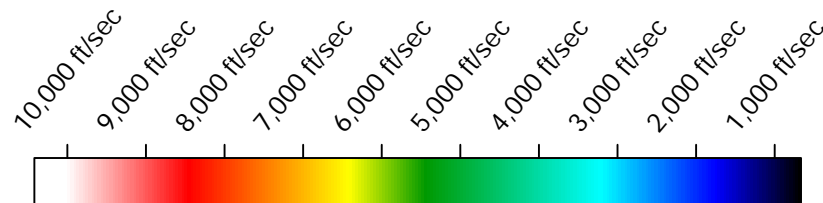
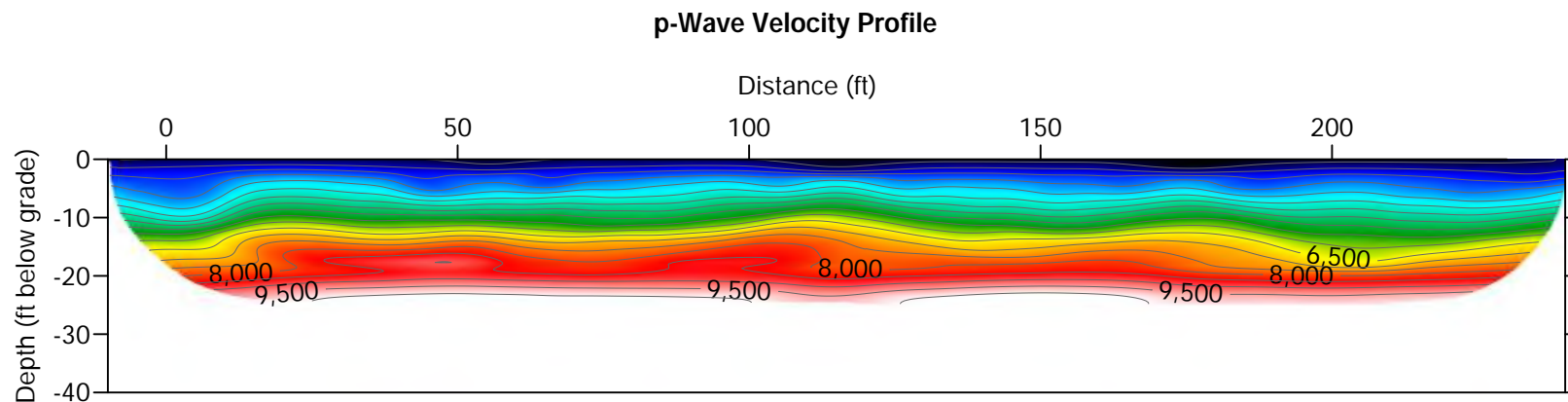
SCALE: As Noted

SOURCE:

PREPARED FOR:



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PROJECT: Geophysical Investigation Seneca Wind Project Seneca Co, Ohio	
DRAWING NO.: Figure 11 p-Wave Velocity Profile GEO-061	
PROJECT NO:	881-6820
SHEET TITLE:	DWG6820F11



p-WAVE VELOCITY

Depth (ft) Velocity (ft/s)

-10 6,209
-20 8,340

Notes

Geophysical survey conducted April 30 - May 1, 2018.

MASW data collected using Geometrics Geode 24-channel seismograph, 4.5 Hz geophones, 5-ft stepout and SurfSeis 4.24 software.

Seismic refraction data collected using Geometrics Geode 24-channel seismograph, 10 Hz geophones, 10-foot stepout and RayFract 3.15 software.

DRN	HLK	5/8/18
DES	HLK	5/8/18
CHK	PJH	5/10/18
REV		
PROJ. MGR.	HLK	5/8/18

SCALE: As Noted

SOURCE:

PREPARED FOR:



PROJECT:

Geophysical Investigation
Seneca Wind Project
Seneca Co, Ohio

DRAWING NO:

Figure 12
p-Wave Velocity Profile
GEO-066

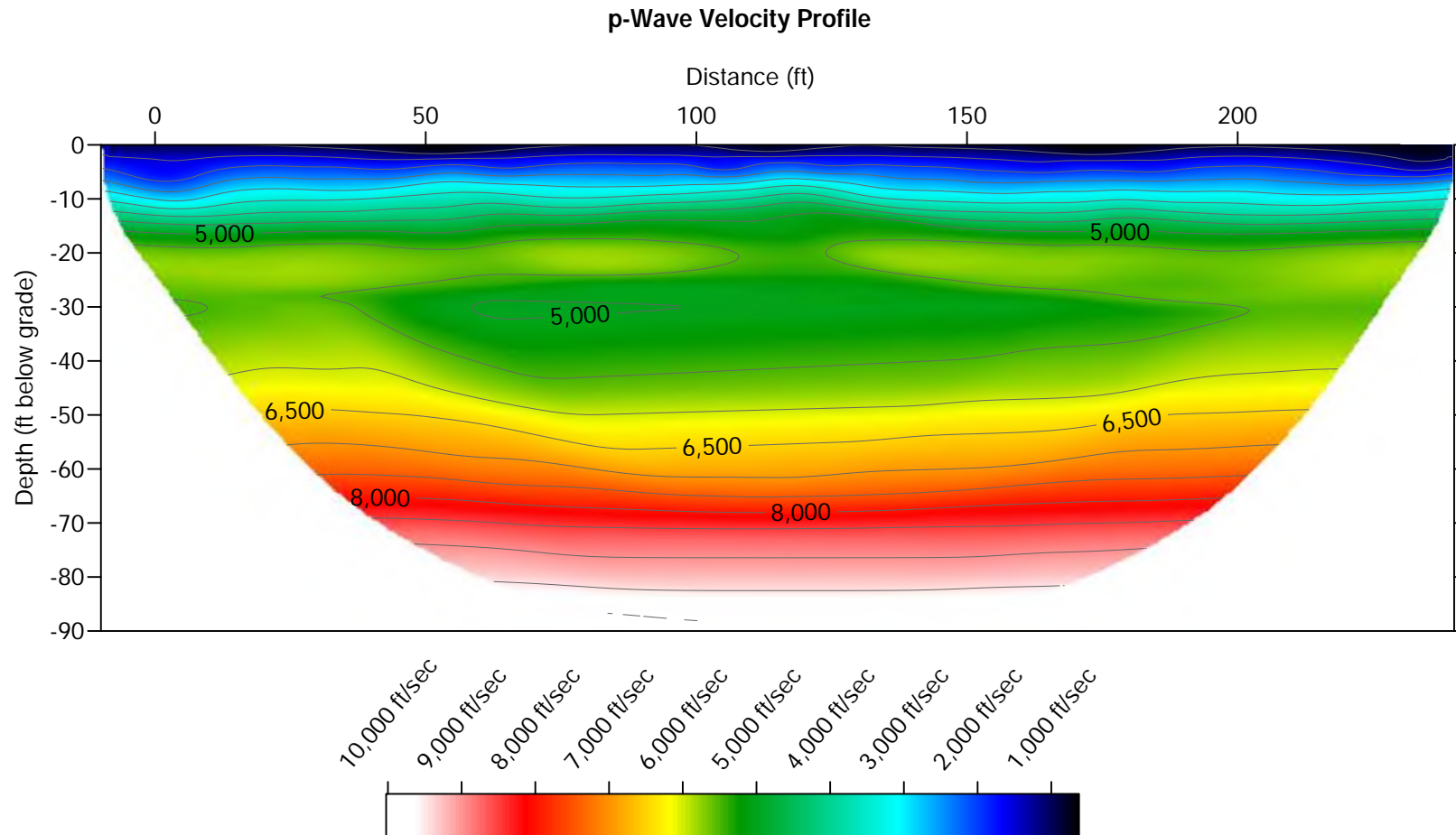
PROJECT NO:

881-6820

SHEET TITLE:

DWG6820F12

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p-WAVE VELOCITY

Depth (ft) Velocity (ft/s)

-10	4,232
-20	5,431
-30	5,018
-40	5,444
-50	6,079
-60	6,861
-70	8,371
-80	9,252

Notes

Geophysical survey conducted April 30 - May 1, 2018.

MASW data collected using Geometrics Geode 24-channel seismograph, 4.5 Hz geophones, 5-ft stepout and SurfSeis 4.24 software.

Seismic refraction data collected using Geometrics Geode 24-channel seismograph, 10 Hz geophones, 10-foot stepout and RayFract 3.15 software.

DRN	HLK	5/8/18
DES	HLK	5/8/18
CHK	PJH	5/10/18
REV		
PROJ. MGR.	HLK	5/8/18

SCALE: As Noted

SOURCE:

PREPARED FOR:



PROJECT:

Geophysical Investigation
Seneca Wind Project
Seneca Co, Ohio

DRAWING NO:

Figure 13
p-Wave Velocity Profile
GEO-085

PROJECT NO:

881-6820

SHEET TITLE:

DWG6820F13

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Appendix D

Physical Laboratory Test Results

Moisture Contents

Water Content Test Summary (ASTM:D2216)

Project: Seneca Wind

Job: 11376

Client Barr Engineering Company

Date: 4/18/2018

Sample Information & Classification

Boring #	Geo-001	Geo-001	Geo-003	Geo-014	Geo-014	Geo-018	Geo-019	Geo-020
Sample #								
Depth (ft)	24-25.5	35-36.5	15-16.5	10-11.5	35-36.5	7.5-9	7.5-9	7.5-9
Type	Bag	Bag	Bag	Bag	Bag	Bag	Bag	Bag
Material Classification	Lean Clay w/sand (CL)	Lean Clay w/sand (CL)	Lean Clay w/sand (CL)	Sandy Lean Clay (CL)	Sand w/gravel (SP/SP-SM)	Clayey Sand w/a little gravel (SC)	Sandy Lean Clay w/a little gravel (CL)	Lean Clay w/sand (CL)
Water Content (%)	17.3	16.7	12.3	16.6	13.6	17.3	19.6	17.1

Sample Information & Classification

Boring #	Geo-020	Geo-021	Geo-022	Geo-022	Geo-045	Geo-047	Geo-051	Geo-051
Sample #								
Depth (ft)	15-16.5	7.5-9	12.5-14	20-20.5	10-11.5	20-21.5	10-11.5	20-21.5
Type	Bag	Bag	Bag	Bag	Bag	Bag	Bag	Bag
Material Classification	Lean Clay w/sand (CL)	Sandy Lean Clay (CL)	Lean Clay w/sand (CL)	Gravel (GP)	Clayey Sand w/gravel (SC)	Sandy Lean Clay w/gravel (CL/SC)	Sandy Lean Clay (CL)	Clayey Sand w/gravel (SC/SP-SC)
Water Content (%)	15.3	16.2	12.0	13.0	19.6	13.8	16.3	14.7

Sample Information & Classification

Boring #	Geo-53	Geo-053	Geo-053	Geo-053	Geo-068	Geo-078	Geo-084	Geo-086
Sample #								
Depth (ft)	15-17.5	25-26.5	45-46.5	55-56.5	10-11.5	5-6.5	10-11.5	5-6.5
Type	Bag	Bag	Bag	Bag	Bag	Bag	Bag	Bag
Material Classification	Gravel w/sand and silt (GP-GM)	Sand (SP/SP-SM)	Sandy Lean Clay (CL)	Clayey Sand w/gravel (SC)	Sandy Silt w/gravel (ML)	Sandy Lean Clay (CL)	Lean Clay (CL)	Lean Clay w/sand (CL)
Water Content (%)	4.9	18.5	14.4	9.5	17.6	19.8	19.3	18.3

Sample Information & Classification

Boring #	Geo-089	Geo-094	Geo-094	Geo-096				
Sample #								
Depth (ft)	15-16.5	10-11.5	20-21.5	7.5-9				
Type	Bag	Bag	Bag	Bag				
Material Classification	Lean Clay w/sand and a trace of gravel (CL)	Lean Clay w/sand (CL)	Clayey Gravel (GC)	Sandy Silt (ML/CL-ML)				
Water Content (%)	13.7	14.2	12.3	17.8				

Water Content Test Summary (ASTM:D2216)

Project: Seneca Wind

Job: 11376

Client Barr Engineering Company

Date: 5/16/2018

Sample Information & Classification

Boring #	Geo-002	Geo-002	Geo-002	Geo-002	Geo-002	Geo-004	Geo-004	Geo-004
Sample #								
Depth (ft)	7.5-9	15-16.5	30-31.5	45-46.5	55-56.5	7.5-9	15-16.5	35-36.5
Type	Bag	Bag	Bag	Bag	Bag	Bag	Bag	Bag
Material Classification	Sandy Lean Clay (CL)	Sandy Lean Clay (CL)	Sandy Lean Clay (CL)	Silty Sand w/a little gravel (SM)	Sandy Lean Clay w/a little gravel (CL)	Sandy Lean Clay w/a little gravel (CL)	Sandy Lean Clay (CL)	Sandy Lean Clay (CL)
Water Content (%)	16.3	16.2	16.6	4.6	12.8	15.8	14.6	18.5

Sample Information & Classification

Boring #	Geo-005	Geo-005	Geo-005	Geo-006	Geo-006	Geo-006	Geo-006	Geo-007
Sample #								
Depth (ft)	12.5-14	20-21.5	35-36.5	10-11.5	30-31.5	40-41.5	55-57	7.5-9
Type	Bag	Bag	Bag	Bag	Bag	Bag	Bag	Bag
Material Classification	Sandy Lean Clay (CL)	Sandy Lean Clay w/a little gravel (CL)	Sandy Lean Clay w/a trace of gravel (CL)	Sandy Lean Clay w/a little gravel (CL)	Sandy Lean Clay w/a trace of gravel (CL)	Sandy Lean Clay (CL)	Sandy Lean Clay (CL)	Sandy Lean Clay w/a trace of gravel (CL)
Water Content (%)	15.9	16.7	16.4	16.7	16.2	15.7	16.9	16.6

Sample Information & Classification

Boring #	Geo-007	Geo-007	Geo-008	Geo-008	Geo-008	Geo-013	Geo-013	Geo-013
Sample #								
Depth (ft)	12.5-14	25-26.5	10-11.5	15-16.5	30-31.5	7.5-9	20-21.5	30-31.5
Type	Bag	Bag	Bag	Bag	Bag	Bag	Bag	Bag
Material Classification	Sandy Lean Clay (CL)	Sandy Lean Clay w/a trace of gravel (CL)	Sandy Lean Clay (CL)	Sandy Lean Clay w/a trace of gravel (CL)	Sandy Lean Clay w/a trace of gravel (CL)	Sandy Lean Clay w/gravel (CL)	Lean Clay w/sand (CL)	Gravel w/silt and sand (GP-GM)
Water Content (%)	15.3	15.0	18.2	17.5	14.2	15.8	14.5	11.3

Sample Information & Classification

Boring #	Geo-013	Geo-013	Geo-015	Geo-015	Geo-015	Geo-015	Geo-015	Geo-016
Sample #								
Depth (ft)	40-41.5	50-51.5	7.5-9	15-16.5	25-26.5	40-41.5	48-48.5	10-11.5
Type	Bag	Bag	Bag	Bag	Bag	Bag	Bag	Bag
Material Classification	Sandy Lean Clay (CL)	Clayey Sand w/gravel (SC)	Lean Clay w/sand (CL)	Sandy Lean Clay w/a trace of gravel (CL)	Sandy Lean Clay w/gravel (CL)	Clayey Gravel (GC)	Clayey Sand w/gravel (SC)	Sandy Lean Clay w/a trace of gravel (CL)
Water Content (%)	24.2	11.6	16.8	18.1	16.3	8.9	12.6	15.0

Water Content Test Summary (ASTM:D2216)

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Sample Information & Classification

Boring #	Geo-016	Geo-016	Geo-016	Geo-024	Geo-024	Geo-024	Geo-024	Geo-024
Sample #								
Depth (ft)	20-21.5	30-31.5	45-46.5	7.5-9	12.5-14	20-21.5	30-31.5	40-41.5
Type	Bag	Bag	Bag	Bag	Bag	Bag	Bag	Bag
Material Classification	Sandy Lean Clay (CL)	Lean Clay w/sand (CL)	Gravel w/sand and silt (GP-GM)	Lean Clay w/sand (CL)	Lean Clay w/sand (CL)	Lean Clay w/sand (CL)	Sandy Lean Clay w/a trace of gravel (CL)	Sand w/silt (SP-SM)
Water Content (%)	17.1	18.3	5.4	18.5	18.0	20.9	13.9	17.7

Sample Information & Classification

Boring #	Geo-025	Geo-025	Geo-025	Geo-030	Geo-030	Geo-030	Geo-030	Geo-031
Sample #								
Depth (ft)	7.5-9	15-16.5	30-31.5	10-11.5	20-21.5	30-31.5	40-41.5	7.5-9
Type	Bag	Bag	Bag	Bag	Bag	Bag	Bag	Bag
Material Classification	Lean Clay w/sand (CL)	Lean Clay w/sand (CL)	Silty Sand (SM)	Sandy Lean Clay (CL)	Lean Clay w/sand (CL)	Lean Clay w/sand (CL)	Silty Sand (SM)	Sandy Lean Clay (CL)
Water Content (%)	17.2	17.1	14.5	10.3	18.0	15.1	15.0	16.6

Sample Information & Classification

Boring #	Geo-031	Geo-031	Geo-033	Geo-033	Geo-033	Geo-033	Geo-033	Geo-034
Sample #								
Depth (ft)	15-16.5	25-26.5	10-11.5	20-21.5	35-36.5	45-46.5	60-61.5	7.5-9
Type	Bag	Bag	Bag	Bag	Bag	Bag	Bag	Bag
Material Classification	Lean Clay w/sand (CL)	Silty Sand w/gravel (SM)	Sandy Lean Clay w/a little gravel (CL)	Lean Clay w/sand and a trace of gravel (CL)	Lean Clay w/sand and a trace of gravel (CL)	Lean Clay w/sand (CL)	Sandy Silt (ML)	Sandy Lean Clay w/a little gravel (CL)
Water Content (%)	16.3	23.1	16.2	16.8	16.4	16.0	23.7	15.5

Sample Information & Classification

Boring #	Geo-034	Geo-034	Geo-034	Geo-034	Geo-035	Geo-035	Geo-035	Geo-035
Sample #								
Depth (ft)	12.5-14	20-21.5	35-36.5	50-51.5	10-11.5	20-21.5	30-31.5	50-51.5
Type	Bag	Bag	Bag	Bag	Bag	Bag	Bag	Bag
Material Classification	Lean Clay w/sand (CL)	Lean Clay w/sand (CL)	Lean Clay w/sand (CL)	Clayey Sand (SC)	Sandy Lean Clay (CL)	Sandy Lean Clay w/a trace of gravel (CL)	Sandy Lean Clay w/a trace of gravel (CL)	Sandy Lean Clay w/a trace of gravel (CL)
Water Content (%)	16.2	16.4	17.6	13.9	15.7	17.6	17.0	16.4

Water Content Test Summary (ASTM:D2216)

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Sample Information & Classification

Boring #	Geo-035	Geo-036	Geo-036	Geo-036	Geo-036	Geo-037	Geo-037	Geo-037
Sample #								
Depth (ft)	60-61.5	10-11.5	25-26.5	40	55	10-11.5	25-26.5	40-41.5
Type	Bag	Bag	Bag	Bag	Bag	Bag	Bag	Bag
Material Classification	Sandy Lean Clay w/a trace of gravel (CL)	Sandy Lean Clay w/a trace of gravel (CL)	Lean Clay w/sand (CL)	Lean Clay w/sand (CL)	Silty Sand w/gravel (SM)	Sandy Lean Clay (CL)	Lean Clay w/sand (CL)	Lean Clay w/sand (CL)
Water Content (%)	16.5	16.3	17.9	17.0	7.2	17.3	17.0	17.0

Sample Information & Classification

Boring #	Geo-037	Geo-038	Geo-038	Geo-038	Geo-038	Geo-038	Geo-039	Geo-039
Sample #								
Depth (ft)	55-56.5	10-11.5	20-21.5	35-36.5	45-46.5	60-61.5	7.5-9	15-16.5
Type	Bag	Bag	Bag	Bag	Bag	Bag	Bag	Bag
Material Classification	Sand w/silt (SP-SM)	Sandy Lean Clay (CL)	Sandy Lean Clay w/a trace of gravel (CL)	Sandy Lean Clay (CL)	Sandy Lean Clay (CL)	Clayey Gravel (GC)	Sandy Lean Clay (CL)	Sandy Lean Clay (CL)
Water Content (%)	18.3	19.3	17.0	16.6	11.1	7.0	16.9	14.8

Sample Information & Classification

Boring #	Geo-039	Geo-039	Geo-040	Geo-040	Geo-040	Geo-040	Geo-040	Geo-040
Sample #								
Depth (ft)	25-26.5	40-41.5	10-11.5	15-16.5	25-26.5	35-36.5	50-51.5	60-61.5
Type	Bag	Bag	Bag	Bag	Bag	Bag	Bag	Bag
Material Classification	Sandy Lean Clay (CL)	Sandy Lean Clay (CL)	Sandy Lean Clay (CL)	Sandy Lean Clay (CL)	Sandy Lean Clay (CL)	Sandy Lean Clay (CL)	Lean Clay w/sand (CL)	Gravel w/clay and silt (GC-GM)
Water Content (%)	14.9	13.0	16.5	16.0	16.3	17.1	17.8	11.2

Sample Information & Classification

Boring #	Geo-041	Geo-041	Geo-041	Geo-041	Geo-041	Geo-043	Geo-043	Geo-043
Sample #								
Depth (ft)	7.5-9	20-21.5	30-31.5	40-41.5	55-56.5	7.5-9	15-16.5	25-26.5
Type	Bag	Bag	Bag	Bag	Bag	Bag	Bag	Bag
Material Classification	Sandy Lean Clay w/a little gravel (CL)	Lean Clay w/sand (CL)	Lean Clay w/sand (CL)	Sandy Lean Clay (CL)	Sandy Lean Clay w/a trace of gravel (CL)	Sandy Lean Clay w/a trace of gravel (CL)	Lean Clay w/sand and a trace of gravel (CL)	Lean Clay w/sand (CL)
Water Content (%)	13.1	17.4	17.1	15.8	14.6	15.4	13.0	17.9

Water Content Test Summary (ASTM:D2216)

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Sample Information & Classification

Boring #	Geo-043	Geo-043	Geo-044	Geo-044	Geo-044	Geo-044	Geo-044	Geo-044
Sample #								
Depth (ft)	35-36.5	50-51.5	7.5-9	12.5-14	20-21.5	35-36.5	45-46.5	60-61.5
Type	Bag	Bag	Bag	Bag	Bag	Bag	Bag	Bag
Material Classification	Lean Clay w/sand (CL)	Sandy Lean Clay (CL)	Sandy Lean Clay (CL)	Sandy Lean Clay w/a trace of gravel (CL)	Sandy Lean Clay w/a trace of gravel (CL)	Lean Clay w/sand and a trace of gravel (CL)	Lean Clay w/sand (CL)	Sandy Lean Clay (CL)
Water Content (%)	18.8	17.3	15.1	15.9	17.7	18.5	16.7	10.5

Sample Information & Classification

Boring #	Geo-056	Geo-056	Geo-056	Geo-056	Geo-059	Geo-059	Geo-059	Geo-059
Sample #								
Depth (ft)	7.5-9	15-16.5	25-26.5	45-46.5	7.5-9	12.5-14	20-21.5	30-31.5
Type	Bag	Bag	Bag	Bag	Bag	Bag	Bag	Bag
Material Classification	Lean Clay w/sand (CL)	Sandy Lean Clay w/a trace of gravel (CL)	Sandy Lean Clay w/a trace of gravel (CL)	Sandy Lean Clay (CL)	Lean Clay w/sand (CL)	Lean Clay w/sand (CL)	Sandy Lean Clay w/a trace of gravel (CL)	Sandy Lean Clay w/a trace of gravel (CL)
Water Content (%)	16.6	16.5	17.4	16.2	16.4	17.2	12.6	13.5

Sample Information & Classification

Boring #	Geo-059	Geo-059	Geo-061	Geo-061	Geo-061	Geo-061	Geo-062	Geo-062
Sample #								
Depth (ft)	45-46.5	55-56.5	10-11.5	20-21.5	30-31.5	45-46.5	7.5-9	15-16.5
Type	Bag	Bag	Bag	Bag	Bag	Bag	Bag	Bag
Material Classification	Sandy Lean Clay w/a trace of gravel (CL)	Clayey Sand w/a little gravel (SC)	Sandy Lean Clay (CL)	Sandy Lean Clay (CL)	Sandy Lean Clay (CL)	Sandy Lean Clay (CL)	Sandy Lean Clay w/a trace of gravel (CL)	Sandy Lean Clay w/a trace of gravel (CL)
Water Content (%)	18.0	12.7	15.9	14.8	14.1	12.4	15.0	15.2

Sample Information & Classification

Boring #	Geo-062	Geo-062	Geo-062	Geo-063	Geo-063	Geo-063	Geo-063	Geo-069
Sample #								
Depth (ft)	30-31.5	45-46.5	60-61.5	10-11.5	20-21.5	35-36.5	50-51.5	10-11.5
Type	Bag	Bag	Bag	Bag	Bag	Bag	Bag	Bag
Material Classification	Sandy Lean Clay w/a trace of gravel (CL)	Sandy Lean Clay w/a trace of gravel (CL)	Lean Clay w/sand and a trace of gravel (CL)	Sandy Lean Clay (CL)	Lean Clay w/sand (CL)	Lean Clay w/sand (CL)	Sandy Lean Clay (CL)	Lean Clay w/sand (CL)
Water Content (%)	16.8	14.6	15.6	17.4	17.3	15.8	13.4	18.8

Water Content Test Summary (ASTM:D2216)

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Sample Information & Classification

Boring #	Geo-069	Geo-070	Geo-070	Geo-070	Geo-071	Geo-071	Geo-071	Geo-071
Sample #								
Depth (ft)	20-21.5	7.5-9	15-16.5	30-31.5	10-11.5	20-21.5	35-36.5	45-46.5
Type	Bag	Bag	Bag	Bag	Bag	Bag	Bag	Bag
Material Classification	Silty Sand (SM)	Sandy Lean Clay (CL)	Lean Clay (CL)	Lean Clay w/sand (CL)	Sandy Lean Clay (CL)	Sandy Lean Clay (CL)	Clayey Sand (SC)	Sandy Lean Clay w/a little gravel (CL)
Water Content (%)	14.5	16.4	19.9	17.7	16.8	17.1	14.0	14.1

Sample Information & Classification

Boring #	Geo-071	Geo-072	Geo-072	Geo-072	Geo-072	Geo-074	Geo-074	Geo-074
Sample #								
Depth (ft)	60-61.5	10-11.5	20-21.5	40-41.5	50-51.5	7.5-9	15-16.5	30-31.5
Type	Bag	Bag	Bag	Bag	Bag	Bag	Bag	Bag
Material Classification	Sandy Lean Clay w/a little gravel (CL)	Sandy Lean Clay w/a trace of gravel (CL)	Sandy Lean Clay w/a trace of gravel (CL)	Sandy Lean Clay w/a trace of gravel (CL)	Sand w/silt (SP-SM)	Sandy Lean Clay w/a trace of gravel (CL)	Lean Clay w/sand (CL)	Lean Clay w/sand (CL)
Water Content (%)	14.0	15.9	16.5	13.5	14.3	14.9	17.2	18.2

Sample Information & Classification

Boring #	Geo-074	Geo-074	Geo-079	Geo-079	Geo-079	Geo-079	Geo-079	Geo-081
Sample #								
Depth (ft)	45-46.5	60-61.5	7.5-9	15-16.5	30-31.5	45-46.5	60-61.5	10-11.5
Type	Bag	Bag	Bag	Bag	Bag	Bag	Bag	Bag
Material Classification	Sandy Lean Clay w/a little gravel (CL)	Sandy Lean Clay w/a little gravel (CL)	Lean Clay w/sand (CL)	Lean Clay w/sand (CL)	Lean Clay w/sand (CL)	Lean Clay w/sand and a trace of gravel (CL)	Sand (SP)	Lean Clay w/sand (CL)
Water Content (%)	13.4	12.8	17.2	17.7	17.1	18.3	13.1	16.3

Sample Information & Classification

Boring #	Geo-081	Geo-081	Geo-081	Geo-082	Geo-082	Geo-082	Geo-082	Geo-082
Sample #								
Depth (ft)	24-25.5	40-41.5	60-61.5	10-11.5	20-21.5	30-31.5	40-41.5	55-55.9
Type	Bag	Bag	Bag	Bag	Bag	Bag	Bag	Bag
Material Classification	Lean Clay w/sand (CL)	Sandy Lean Clay w/a trace of gravel (CL)	Sand w/silt and gravel (SP-SM)	Sandy Lean Clay w/a trace of gravel (CL)	Lean Clay w/sand (CL)	Lean Clay w/sand (CL)	Lean Clay w/sand and pockets of clayey sand (CL)	Lean Clay w/sand (CL)
Water Content (%)	18.6	17.4	10.3	17.7	18.6	19.3	15.7	17.2

Water Content Test Summary (ASTM:D2216)

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Date: 5/16/2018

Sample Information & Classification

Boring #	Geo-083	Geo-083	Geo-083	Geo-085	Geo-085	Geo-085	Geo-085	Geo-087
Sample #								
Depth (ft)	10-11.5	20-21.5	30-31.5	7.5-9	15-16.5	30-31.5	50-51.5	10-11.5
Type	Bag	Bag	Bag	Bag	Bag	Bag	Bag	Bag
Material Classification	Lean Clay w/sand (CL)	Sandy Lean Clay w/a trace of gravel (CL)	Silt w/sand (ML)	Lean Clay w/sand and a trace of gravel (CL)	Lean Clay w/sand and a trace of gravel (CL)	Lean Clay w/sand and a trace of gravel (CL)	Lean Clay w/sand and a trace of gravel (CL)	Lean Clay w/sand and a trace of gravel (CL)
Water Content (%)	15.7	15.9	16.8	15.7	15.4	16.9	14.2	16.3

Sample Information & Classification

Boring #	Geo-087	Geo-087	Geo-087	Geo-088	Geo-088	Geo-091	Geo-091	Geo-091
Sample #								
Depth (ft)	20-21.5	30-31.5	45-46.5	7.5-9	15-16.5	10-11.5	20-21.5	30-31.5
Type	Bag	Bag	Bag	Bag	Bag	Bag	Bag	Bag
Material Classification	Lean Clay w/sand (CL)	Sandy Lean Clay (CL)	Sand w/silt and a little gravel (SP-SM)	Sandy Lean Clay (CL)	Lean Clay w/sand (CL)	Lean Clay w/sand (CL)	Sandy Lean Clay w/a trace of gravel (CL)	Lean Clay w/sand (CL)
Water Content (%)	16.4	15.4	14.8	18.0	16.7	16.3	14.5	16.9

Sample Information & Classification

Boring #	Geo-091	Geo-091	Geo-092	Geo-092	Geo-092	Geo-092	Geo-092	Geo-093
Sample #								
Depth (ft)	45-46.5	60-61.5	7.5-9	15-16.5	30-31.5	45-46.5	60-61.5	10-11.5
Type	Bag	Bag	Bag	Bag	Bag	Bag	Bag	Bag
Material Classification	Lean Clay w/sand (CL)	Sandy Lean Clay (CL)	Lean Clay w/sand (CL)	Lean Clay w/sand (CL)	Lean Clay w/sand (CL)	Lean Clay w/sand and a trace of gravel (CL)	Silt (ML)	Lean Clay w/sand and a trace of gravel (CL)
Water Content (%)	18.2	12.3	16.6	15.4	18.1	18.9	27.0	16.1

Sample Information & Classification

Boring #	Geo-093	Geo-093	Geo-093					
Sample #								
Depth (ft)	20-21.5	40-41.5	55-56.5					
Type	Bag	Bag	Bag					
Material Classification	Lean Clay w/sand and a trace of gravel (CL)	Lean Clay w/sand and a trace of gravel (CL)	Silty Sand w/occasional pieces of clay and a trace of gravel (SM)					
Water Content (%)	17.0	18.3	14.3					

Water Content Test Summary (ASTM:D2216)

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Sample Information & Classification

Boring #	Geo-009	Geo-009	Geo-009	Geo-042	Geo-042	Geo-052	Geo-052	Geo-052
Sample #								
Depth (ft)	7.5-9	35-36.5	55-56.5	7.5-9	55-56.5	10-11.5	35-36.5	55-56.5
Type	Bag	Bag	Bag	Bag	Bag	Bag	Bag	Bag
Material Classification	Lean Clay w/sand (CL)	Lean Clay w/sand (CL)	Lean Clay w/sand and a trace of gravel (CL)	Lean Clay (CL)	Lean Clay (CL)	Lean Clay w/sand (CL)	Lean Clay w/sand (CL)	Lean Clay w/sand (CL)
Water Content (%)	16.6	16.3	14.5	20.3	17.2	14.4	19.3	19.0

Sample Information & Classification

Boring #	Geo-075	Geo-075	Geo-075	Geo-075	Geo-076	Geo-076	Geo-076	Geo-076
Sample #								
Depth (ft)	10-11.5	20-21.5	35-36.5	55-56.5	7.5-9	25-26.5	40-41.5	55-56.5
Type	Bag	Bag	Bag	Bag	Bag	Bag	Bag	Bag
Material Classification	Lean Clay w/sand (CL)	Lean Clay w/sand (CL)	Lean Clay (CL)	Lean Clay w/sand and a trace of gravel (CL)	Lean Clay w/sand and pockets of sand (CL)	Silty Clayey (CL-ML/ML)	Lean Clay w/sand (CL)	Lean Clay w/a trace of gravel (CL)
Water Content (%)	15.3	16.9	19.8	14.4	12.3	16.8	18.0	20.0

Sample Information & Classification

Boring #	Geo-090	Geo-090	Geo-090	Geo-090				
Sample #								
Depth (ft)	10-11.5	20-21.5	40-41.5	60-61.5				
Type	Bag	Bag	Bag	Bag				
Material Classification	Lean Clay w/sand (CL)	Lean Clay w/sand (CL)	Silty Sand (SM)	Lean Clay w/sand (CL)				
Water Content (%)	17.9	16.5	14.4	15.9				

Sample Information & Classification

Boring #								
Sample #								
Depth (ft)								
Type								
Material Classification								
Water Content (%)								

Atterberg Limits

Laboratory Test Summary

Project: _____ Seneca Wind

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Client: _____ Barr Engineering Company

Date: 4/19/2018

Sample Information & Classification

Boring #	Geo-001	Geo-003	Geo-089					
Sample #								
Depth (ft)	12.5-14	15-16.5	10-11.5					
Sample Type	Bag	Bag	Bag					
Material Classification	Lean Clay w/sand (CL)	Lean Clay w/sand (CL)	Sandy Lean Clay (CL)					

Atterberg Limits (ASTM:D4318)

Liquid Limit	31	27	36					
Plastic Limit	16	15	19					
Plasticity Index	15	12	17					

Sample Information & Classification

Boring #								
Sample #								
Depth (ft)								
Sample Type								
Material Classification								

Atterberg Limits (ASTM:D4318)

Liquid Limit								
Plastic Limit								
Plasticity Index								

Laboratory Test Summary

Project: Seneca Wind

Job: 11376

Client: Barr Engineering Company

Date: 5/18/2018

Sample Information & Classification

Boring #	Geo-024	Geo-025	Geo-031	Geo-035	Geo-043	Geo-056	Geo-081	Geo-082
Sample #								
Depth (ft)	20-21.5	15-16.5	15-16.5	7.5-9	10-11.5	55-56.5	24-25.5	10-11.5
Sample Type	Bag	Bag	Bag	Bag	Bag	Bag	Bag	Bag
Material Classification	Lean Clay w/sand (CL)	Lean Clay w/sand (CL)	Lean Clay w/sand (CL)	Sandy Lean Clay (CL)	Sandy Lean Clay (CL)	Lean Clay w/sand (CL)	Lean Clay w/sand (CL)	Sandy Lean Clay w/a trace of gravel (CL)

Atterberg Limits (ASTM:D4318)

Liquid Limit	31	32	29	36	32	26	30	35
Plastic Limit	16	16	16	18	16	16	15	16
Plasticity Index	15	16	13	18	16	10	15	19

Sample Information & Classification

Boring #								
Sample #								
Depth (ft)								
Sample Type								
Material Classification								

Atterberg Limits (ASTM:D4318)

Liquid Limit								
Plastic Limit								
Plasticity Index								

Laboratory Test Summary

Project: _____ Seneca Wind

Job: 11376

Client: Barr Engineering Company

Date: 5/25/2018

Sample Information & Classification

Boring #	Geo-076							
Sample #								
Depth (ft)	25-26.5							
Sample Type	Bag							
Material Classification	Silty Clay (CL-ML/ML)							

Atterberg Limits (ASTM:D4318)

Liquid Limit	18							
Plastic Limit	14							
Plasticity Index	4							

Sample Information & Classification

Boring #								
Sample #								
Depth (ft)								
Sample Type								
Material Classification								

Atterberg Limits (ASTM:D4318)

Liquid Limit								
Plastic Limit								
Plasticity Index								

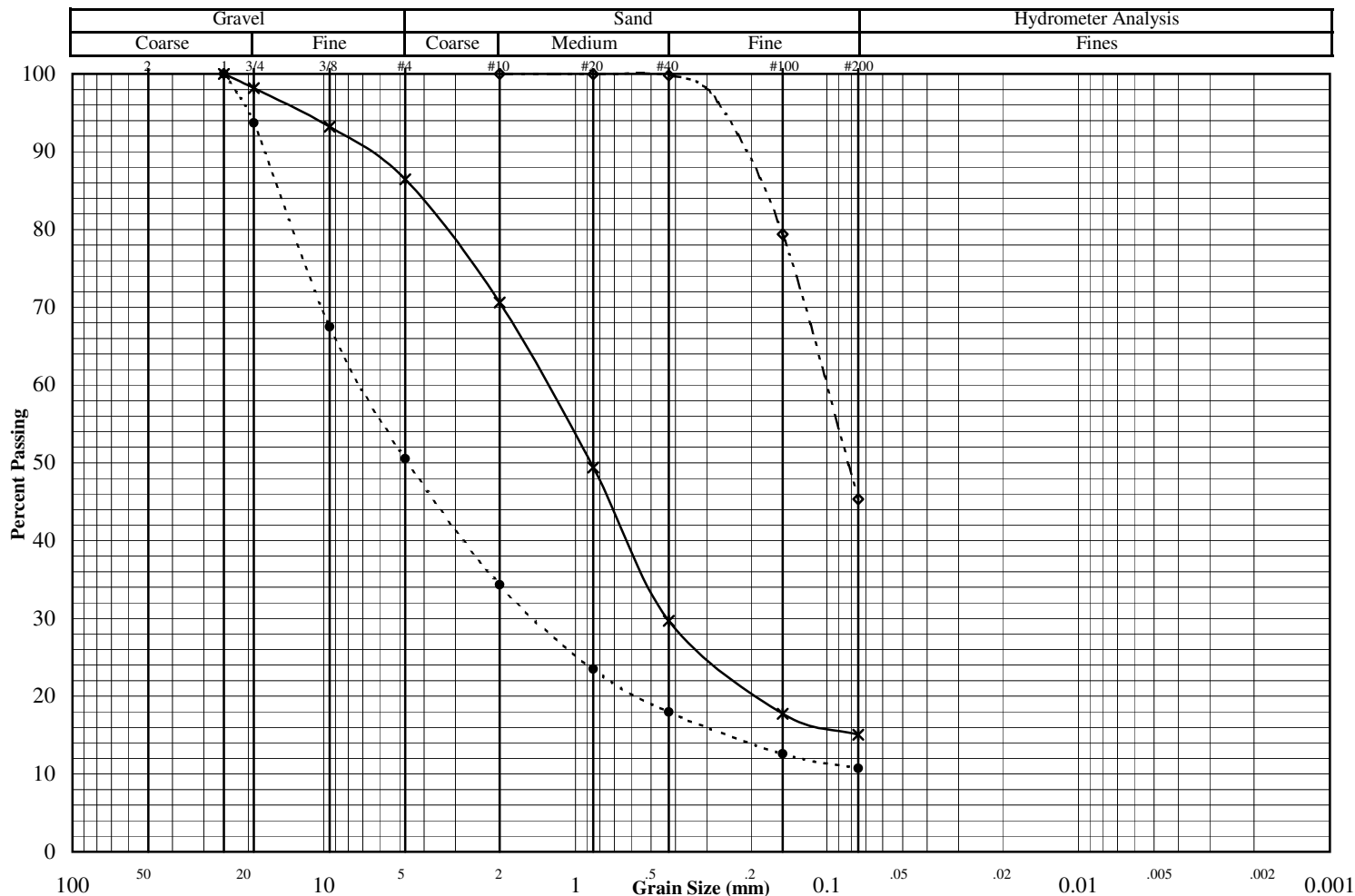
Grain Size

Grain Size Distribution ASTM D422

Job No. : **11376**

Project:	Seneca Wind	Test Date:	5/7/18
Reported To:	Barr Engineering Company	Report Date:	5/14/18

	Location / Boring No.	Sample No.	Depth (ft)	Sample Type	Soil Classification
*	Geo-002		45-46.5	Bag	Silty Sand w/ a little gravel (SM)
●	Geo-013		30-31.5	Bag	Gravel w/ silt and sand (GP-GM)
◇	Geo-025		30-31.5	Bag	Silty Sand (SM)

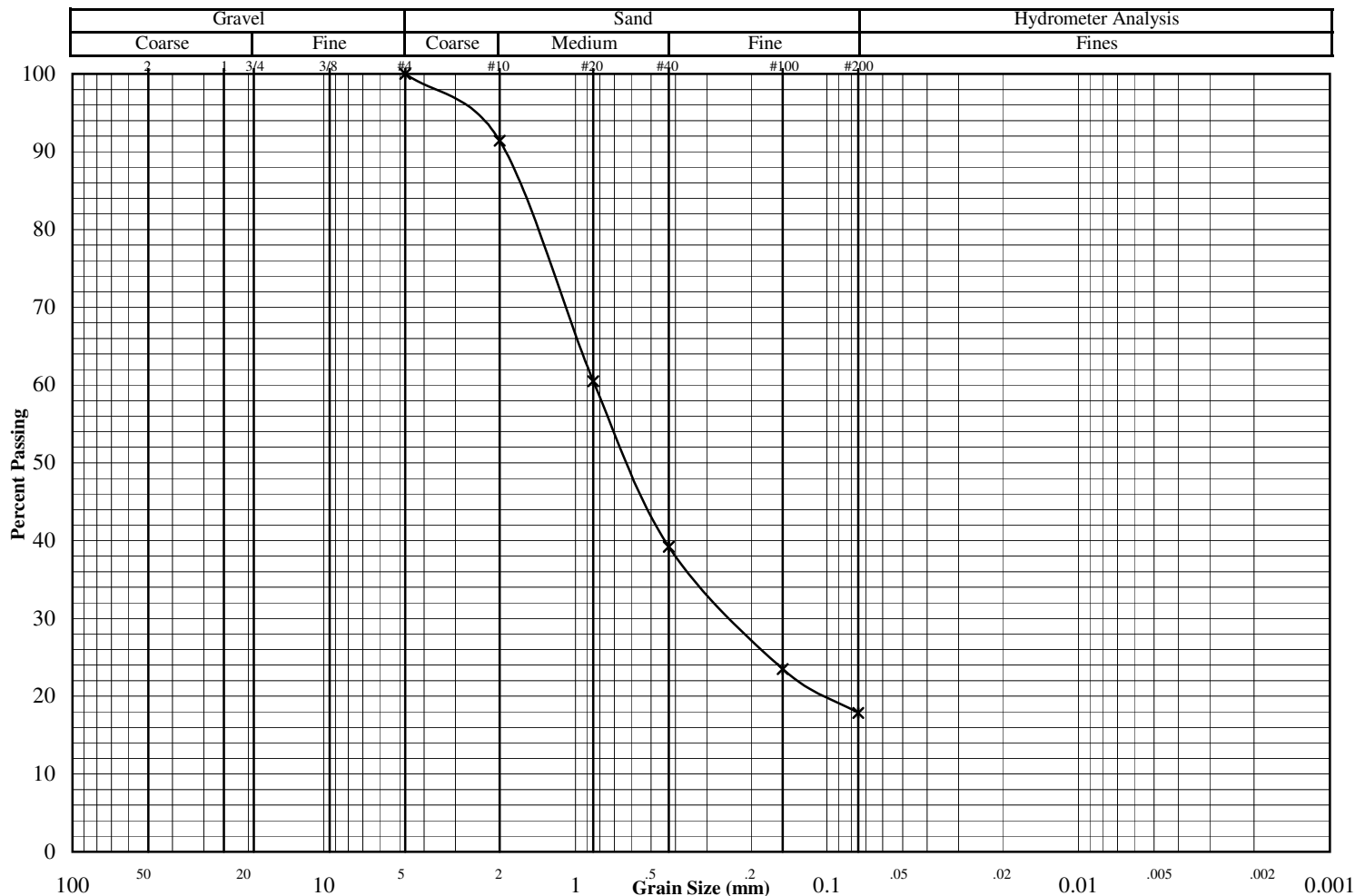


Grain Size Distribution ASTM D422

Job No. : **11376**

Project:	Seneca Wind	Test Date:	4/17/18
Reported To:	Barr Engineering Company	Report Date:	4/19/18

	Location / Boring No.	Sample No.	Depth (ft)	Sample Type	Soil Classification
*	Geo-014		45-46.5	Bag	Silty Sand (SM)
•					
◇					



Other Tests	*	•	◇
Liquid Limit			
Plastic Limit			
Plasticity Index			
Water Content			
Dry Density (pcf)			
Specific Gravity			
Porosity			
Organic Content			
pH			
Shrinkage Limit			
Penetrometer			
Qu (psf)			
(* = assumed)			

Percent Passing	*	•	◇
Mass (g)	265.4		
2"			
1.5"			
1"			
3/4"			
3/8"			
#4	100.0		
#10	91.4		
#20	60.5		
#40	39.2		
#100	23.5		
#200	17.9		

	*	•	◇
D ₆₀			
D ₃₀			
D ₁₀			
C _u			
C _c			

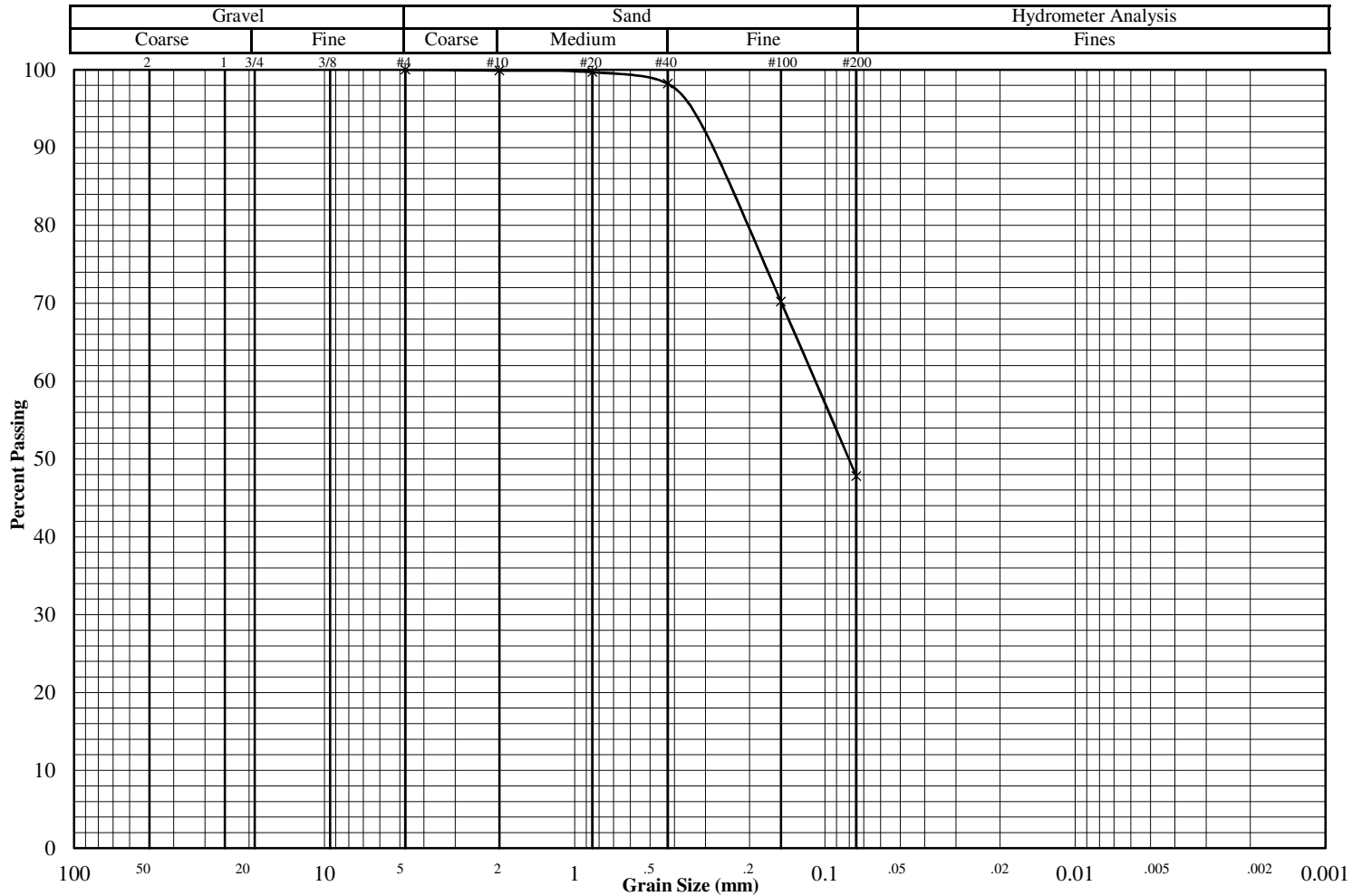
Remarks:

Grain Size Distribution ASTM D422

Job No. : **11376**

Project:	Seneca Wind	Test Date:	5/22/18
Reported To:	Barr Engineering Company	Report Date:	5/25/18

	Location / Boring No.	Sample No.	Depth (ft)	Sample Type	Soil Classification
*	Geo-090		40-41.5	Bag	Silty Sand (SM)
•					
◇					



Other Tests			Percent Passing					
	*	•	◇		*	•	◇	
Liquid Limit				Mass (g)	158.8			
Plastic Limit				2"				
Plasticity Index				1.5"				
Water Content	14.4			1"				
Dry Density (pcf)				3/4"				
Specific Gravity				3/8"				
Porosity				#4	100.0			
Organic Content				#10	99.9			
pH				#20	99.7			
Shrinkage Limit				#40	98.2			
Penetrometer				#100	70.2			
Qu (psf)				#200	47.8			
(* = assumed)								

	*	•	◇
D ₆₀			
D ₃₀			
D ₁₀			
C _u			
C _c			

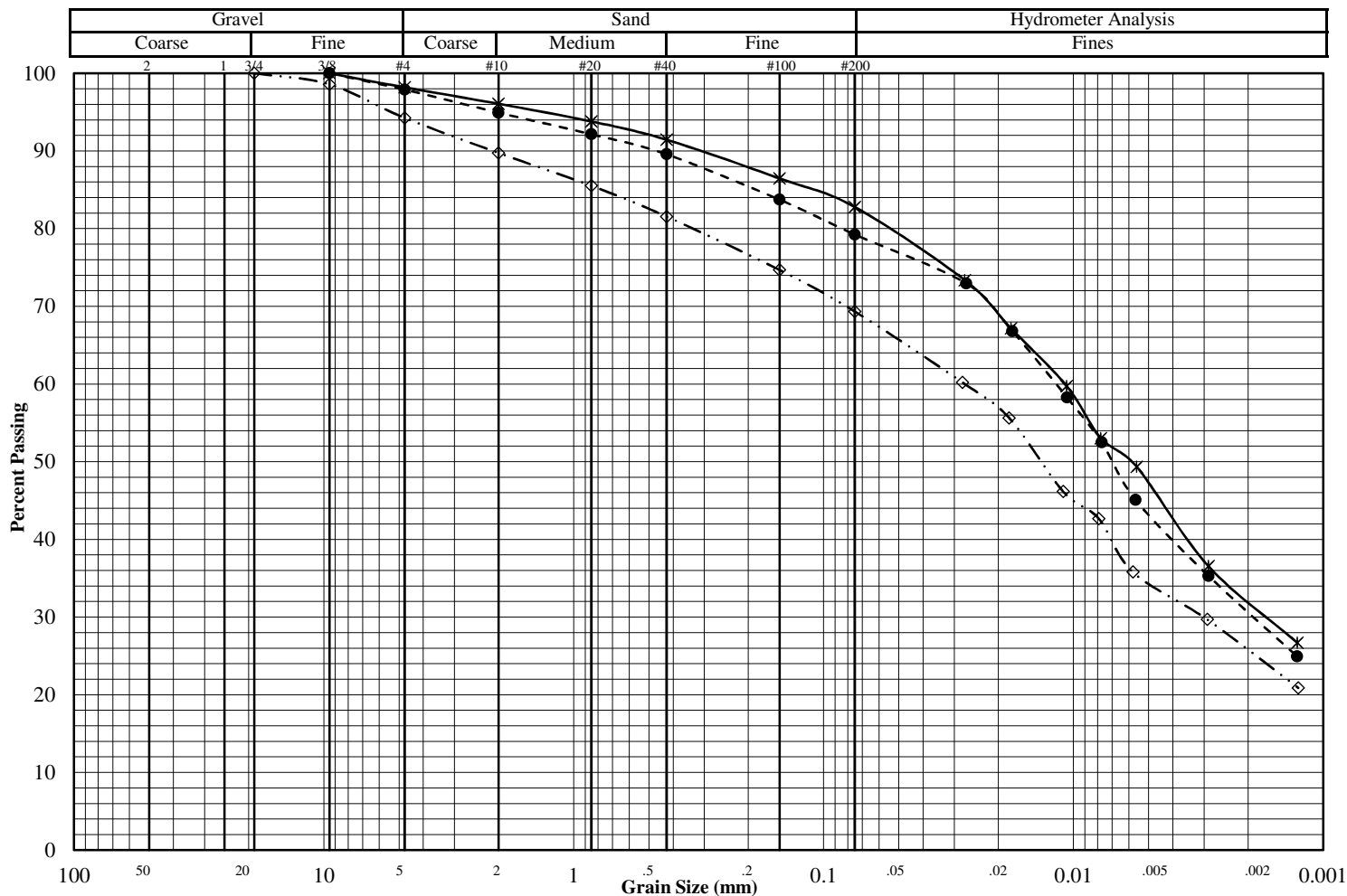
Remarks:

Grain Size Distribution ASTM D422

Job No. : **11376**

Project:	Seneca Wind	Test Date:	5/11/18
Reported To:	Barr Engineering Company	Report Date:	5/17/18

	Location / Boring No.	Sample No.	Depth (ft)	Sample Type	Soil Classification
*	Geo-036		25-26.5	Bag	Lean Clay w/sand (CL)
●	Geo-043		15-16.5	Bag	Lean Clay w/sand and a trace of gravel (CL)
◇	Geo-056		60-61.5	Bag	Sandy Lean Clay w/a little gravel (CL)



Other Tests

	*	●	◇
Liquid Limit			
Plastic Limit			
Plasticity Index			
Water Content	17.9	13.0	
Dry Density (pcf)			
Specific Gravity	2.68*	2.68*	2.68*
Porosity			
Organic Content			
pH			
Shrinkage Limit			
Penetrometer			
Qu (psf)			
(* = assumed)			

Percent Passing

	*	●	◇
Mass (g)	154.4	199.5	277.4
2"			
1.5"			
1"			
3/4"			100.0
3/8"	100.0	100.0	98.6
#4	98.2	97.9	94.3
#10	96.1	94.9	89.7
#20	93.8	92.1	85.5
#40	91.5	89.6	81.6
#100	86.5	83.7	74.7
#200	82.8	79.3	69.3

	*	●	◇
D ₆₀			
D ₃₀			
D ₁₀			
C _u			
C _c			

Remarks:

Grain Size Distribution ASTM D422

Job No. : **11376**

Project:	Seneca Wind	Test Date: 5/11/18
Reported To:	Barr Engineering Company	Report Date: 5/17/18

	Location / Boring No.	Sample No.	Depth (ft)	Sample Type	Soil Classification
Spec 1	Geo-036		25-26.5	Bag	Lean Clay w/sand (CL)
Spec 2	Geo-043		15-16.5	Bag	Lean Clay w/sand and a trace of gravel (CL)
Spec 3	Geo-056		60-61.5	Bag	Sandy Lean Clay w/a little gravel (CL)

Sieve Data

Specimen 1		Specimen 2		Specimen 3	
Sieve	% Passing	Sieve	% Passing	Sieve	% Passing
2"		2"		2"	
1.5"		1.5"		1.5"	
1"		1"		1"	
3/4"		3/4"		3/4"	100.0
3/8"	100.0	3/8"	100.0	3/8"	98.6
#4	98.2	#4	97.9	#4	94.3
#10	96.1	#10	94.9	#10	89.7
#20	93.8	#20	92.1	#20	85.5
#40	91.5	#40	89.6	#40	81.6
#100	86.5	#100	83.7	#100	74.7
#200	82.8	#200	79.3	#200	69.3

Hydrometer Data

Specimen 1		Specimen 2		Specimen 3	
Diameter (mm)	% Passing	Diameter	% Passing	Diameter	% Passing
0.027	73.4	0.027	72.9	0.028	60.2
0.018	67.2	0.018	66.8	0.018	55.6
0.011	59.7	0.011	58.3	0.011	46.2
0.008	53.0	0.008	52.5	0.008	42.7
0.006	49.3	0.006	45.1	0.006	35.8
0.003	36.5	0.003	35.3	0.003	29.7
0.001	26.7	0.001	24.9	0.001	20.9

Remarks

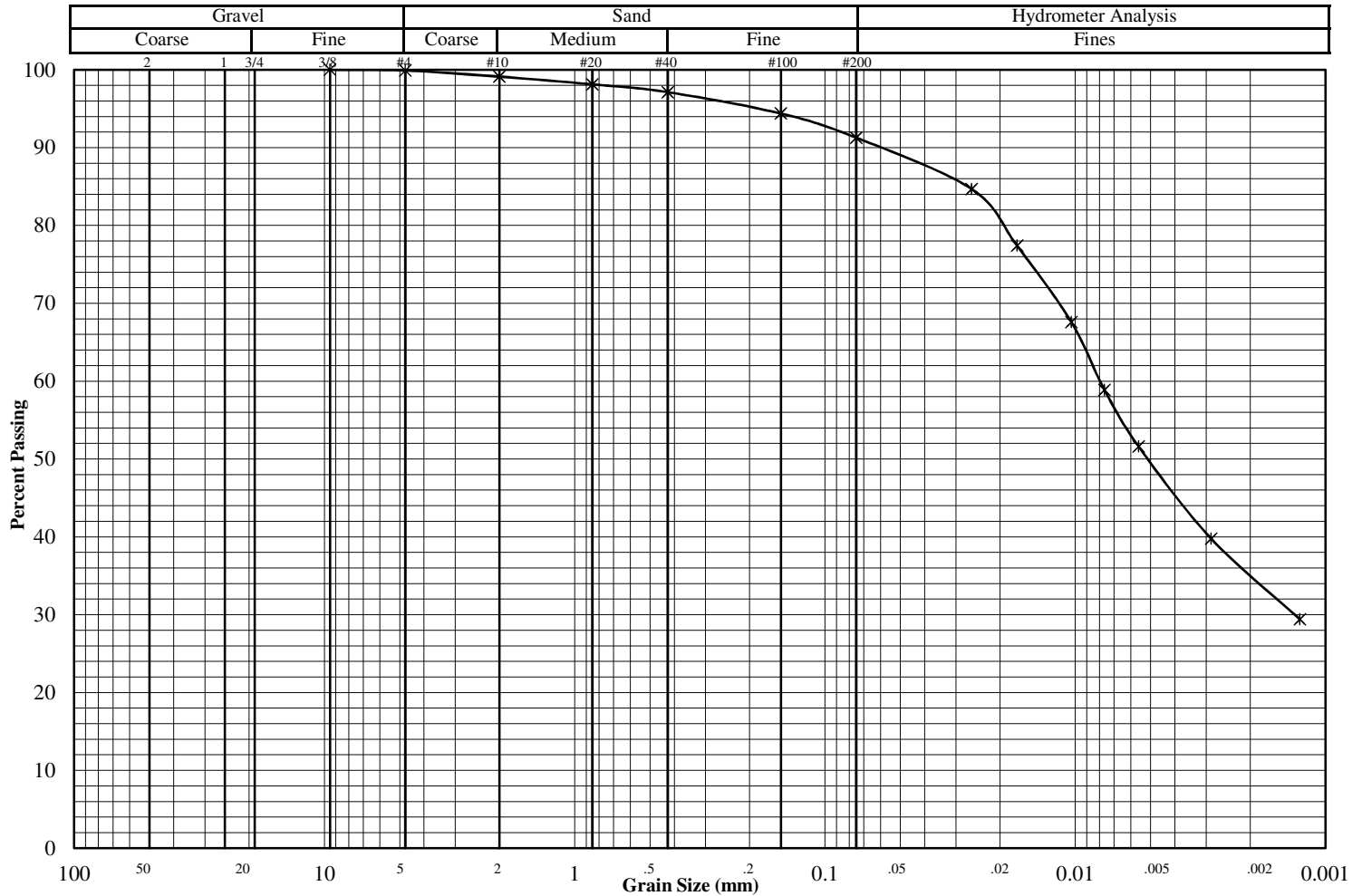
Specimen 1	Specimen 2	Specimen 3

Grain Size Distribution ASTM D422

Job No. : **11376**

Project:	Seneca Wind	Test Date:	5/11/18
Reported To:	Barr Engineering Company	Report Date:	5/17/18

	Location / Boring No.	Sample No.	Depth (ft)	Sample Type	Soil Classification
*	Geo-091		7.5-9	Bag	Lean Clay (CL)
•					
◇					



Grain Size Distribution ASTM D422

Job No. : **11376**

Project: Seneca Wind

Test Date: 5/11/18

Reported To: Barr Engineering Company

Report Date: 5/17/18

	Location / Boring No.	Sample No.	Depth (ft)	Sample Type	Soil Classification
Spec 1	Geo-091		7.5-9	Bag	Lean Clay (CL)
Spec 2					
Spec 3					

Sieve Data

Specimen 1		Specimen 2		Specimen 3	
Sieve	% Passing	Sieve	% Passing	Sieve	% Passing
2"		2"		2"	
1.5"		1.5"		1.5"	
1"		1"		1"	
3/4"		3/4"		3/4"	
3/8"	100.0	3/8"		3/8"	
#4	99.9	#4		#4	
#10	99.1	#10		#10	
#20	98.1	#20		#20	
#40	97.1	#40		#40	
#100	94.4	#100		#100	
#200	91.3	#200		#200	

Hydrometer Data

Specimen 1		Specimen 2		Specimen 3	
Diameter (mm)	% Passing	Diameter	% Passing	Diameter	% Passing
0.026	84.7				
0.017	77.4				
0.010	67.6				
0.008	58.9				
0.006	51.6				
0.003	39.8				
0.001	29.4				

Remarks

Specimen 1	Specimen 2	Specimen 3

Dry Unit Weight

Laboratory Test Summary

Project: Seneca Wind
Client: Barr Engineering Company

Job: 11376
Date: 5/22/18

Sample Information & Classification

Boring #	Geo-034						
Sample #							
Depth (ft)	24-25						
Type or BPF	TWT						
Classification	Sandy Lean Clay w/a trace of gravel (CL)						

Water Content, Dry Density (ASTM:D7263)

Water Content (%)	16.1						
Dry Density (pcf)	111.3						

Sample Information & Classification

Boring #							
Sample #							
Depth (ft)							
Type or BPF							
Classification							

Water Content, Dry Density (ASTM:D7263)

Water Content (%)							
Dry Density (pcf)							

Sample Information & Classification

Boring #							
Sample #							
Depth (ft)							
Type or BPF							
Classification							

Water Content, Dry Density (ASTM:D7263)

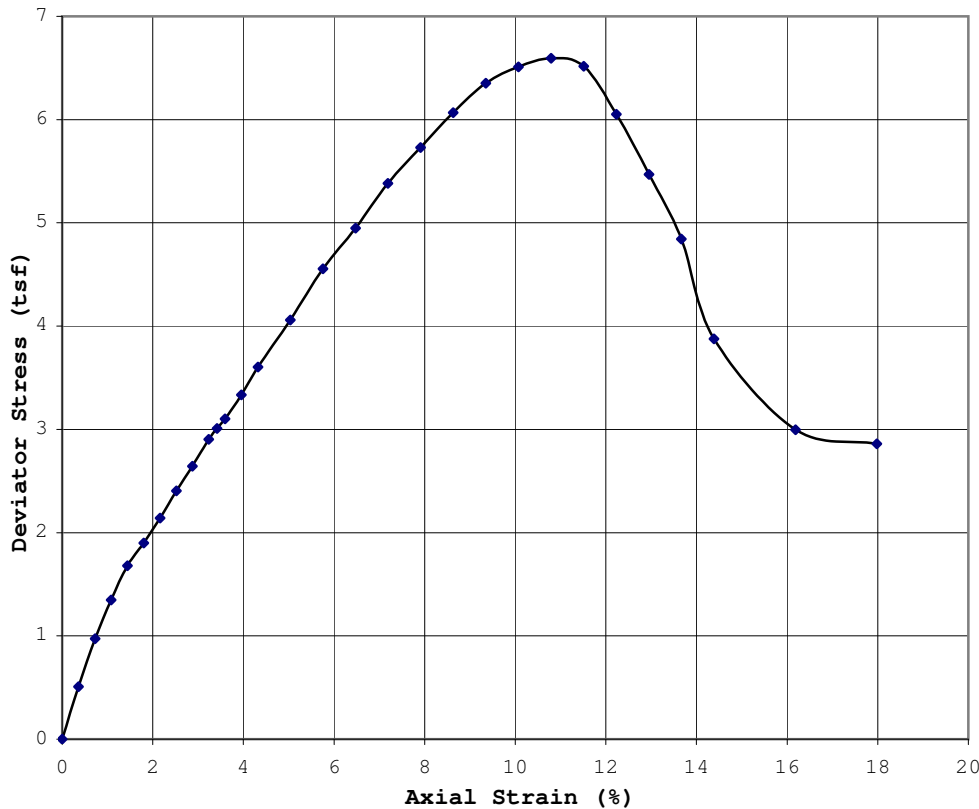
Water Content (%)							
Dry Density (pcf)							

Unconfined Compressive Shear Strength

Unconfined Stress/Strain Curves

ASTM: D2166

Project: Seneca Wind Job: 11376
 Client: Barr Engineering Company Date: 4/23/18
 Remarks: _____



Boring: Geo-003 Depth: 7.5-9

Sample #:

Soil Type: Sandy Lean Clay (CL)

Strain Rate (in/min): 0.030

Sample Type: SB

Dia. (in) 1.39 Ht. (in) 2.78

Height to Diameter Ratio: 2.0

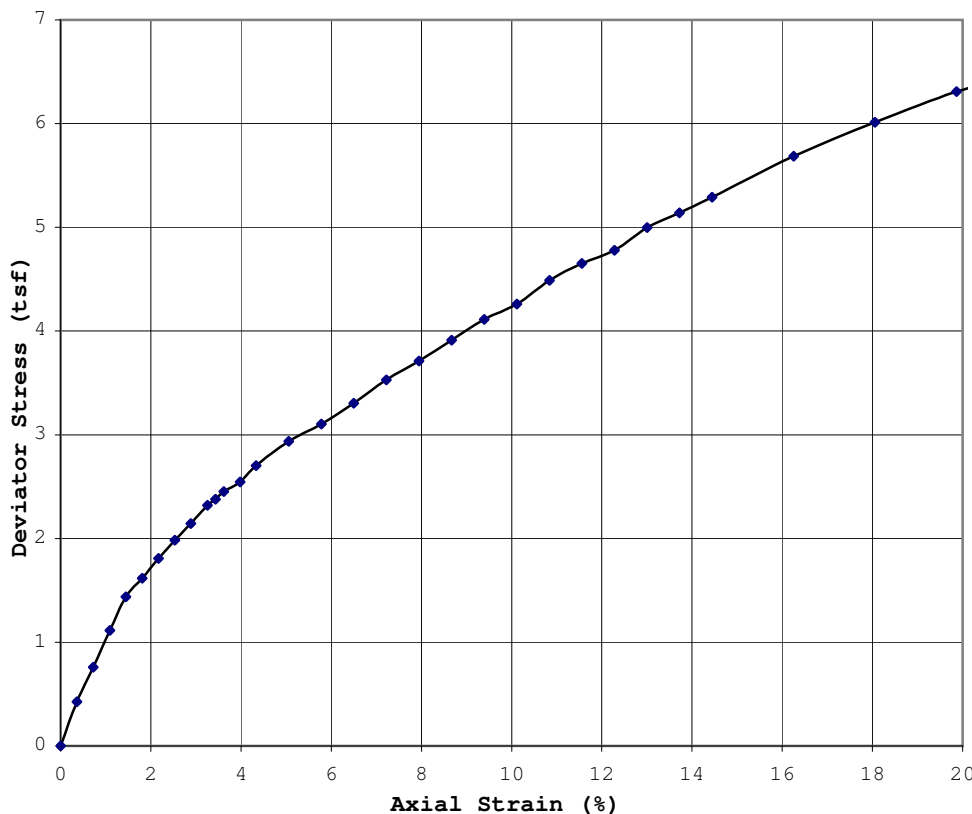
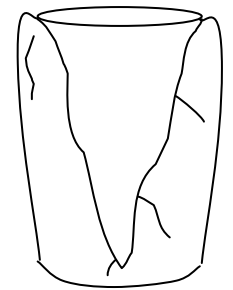
Unconfined Comp. Strength: 6.59 tsf

Strain at Failure (%): 10.8

W.C. (%): 14.4

Yd (pcf): 120.4

Sketch of Specimen After Failure



Boring: Geo-014 Depth: 20-21.5

Sample #:

Soil Type: Lean Clay w/sand (CL)

Strain Rate (in/min): 0.030

Sample Type: SB

Dia. (in) 1.35 Ht. (in) 2.77

Height to Diameter Ratio: 2.1

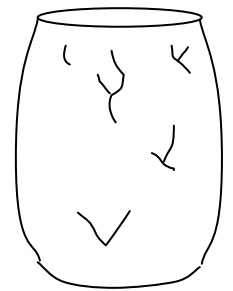
Unconfined Comp. Strength: 6.32 tsf

Strain at Failure (%): 20.0

W.C. (%): 14.9

Yd (pcf): 120.5

Sketch of Specimen After Failure

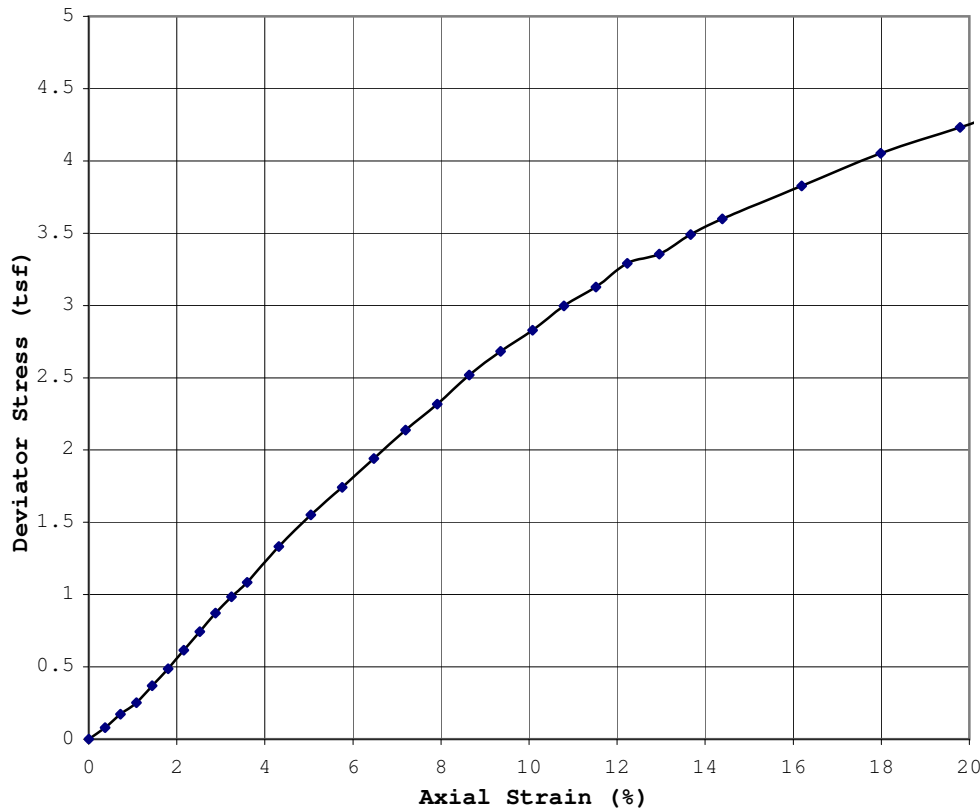


Unconfined Stress/Strain Curves

ASTM: D2166

Project: Seneca Wind
 Client: Barr Engineering Company
 Remarks:

Job: 11376
 Date: 4/23/18



Boring: Geo-021 Depth: 12.5-14
 Sample #:

Soil Type: Lean Clay w/sand (CL)

Strain Rate (in/min): 0.030

Sample Type: SB

Dia. (in) 1.35 Ht. (in) 2.78

Height to Diameter Ratio: 2.1

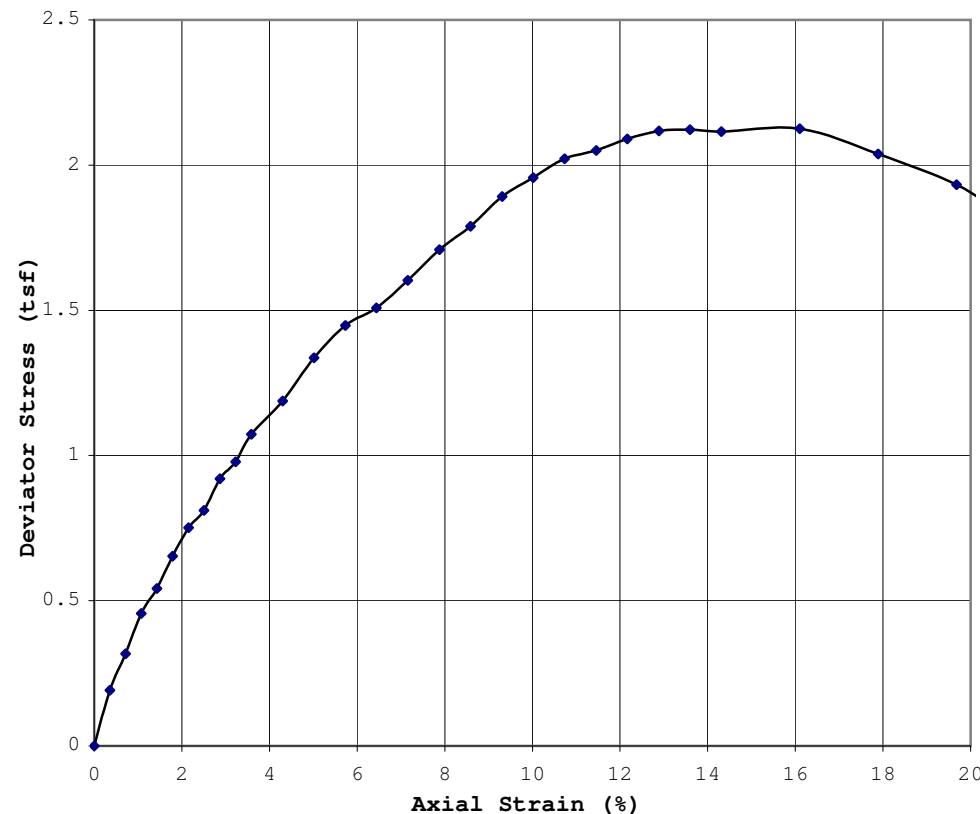
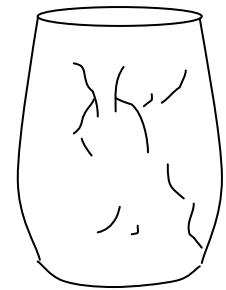
Unconfined Comp. Strength: 4.25 tsf

Strain at Failure (%): 20.0

W.C. (%): 13.0

Yd (pcf): 125.2

Sketch of Specimen After Failure



Boring: Geo-026 Depth: 10-11.5

Sample #:

Soil Type: Lean Clay w/sand (CL)

Strain Rate (in/min): 0.030

Sample Type: SB

Dia. (in) 1.38 Ht. (in) 2.80

Height to Diameter Ratio: 2.0

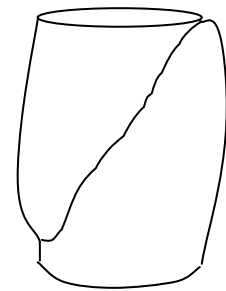
Unconfined Comp. Strength: 2.13 tsf

Strain at Failure (%): 16.1

W.C. (%): 22.5

Yd (pcf): 105.2

Sketch of Specimen After Failure

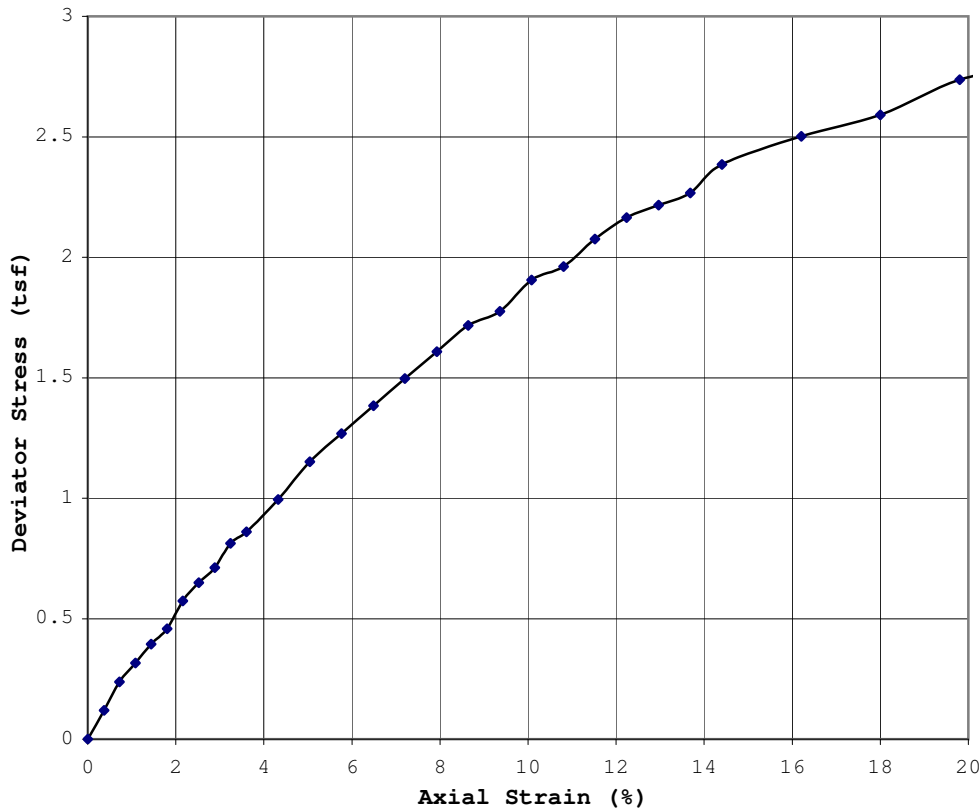


Unconfined Stress/Strain Curves

ASTM: D2166

Project: Seneca Wind
 Client: Barr Engineering Company
 Remarks:

Job: 11376
 Date: 4/23/18



Boring: Geo-050 Depth: 10-11.5
 Sample #:

Soil Type: Lean Clay w/sand (CL)

Strain Rate (in/min): 0.030

Sample Type: SB

Dia. (in) 1.35 Ht. (in) 2.78

Height to Diameter Ratio: 2.1

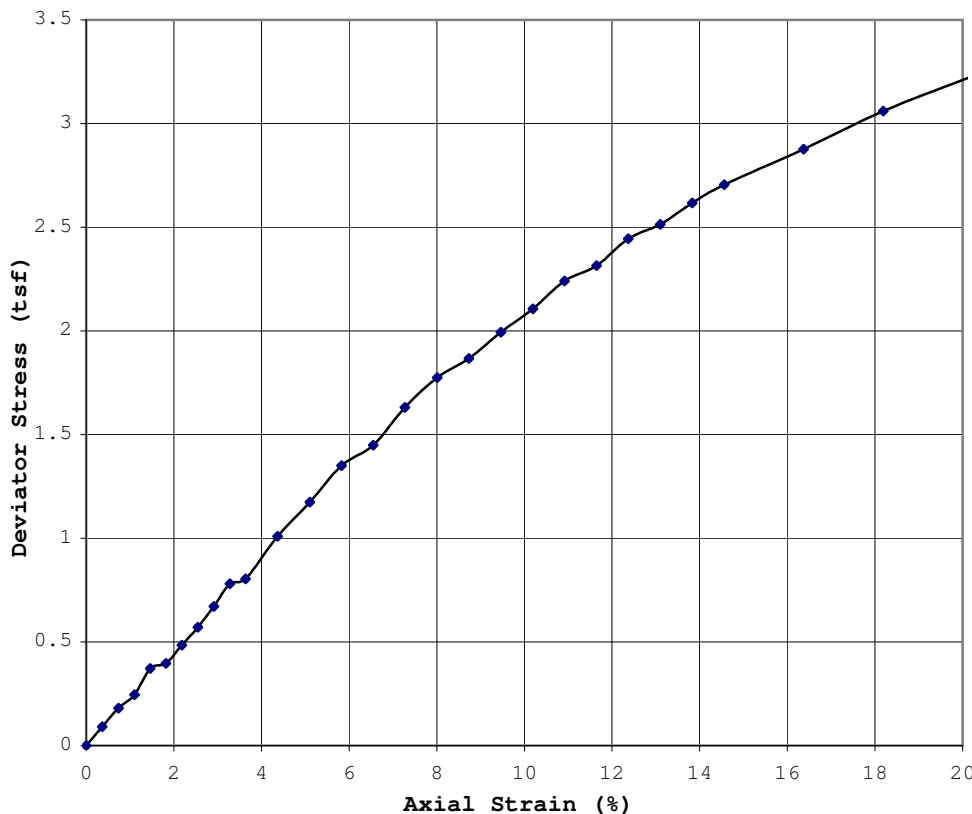
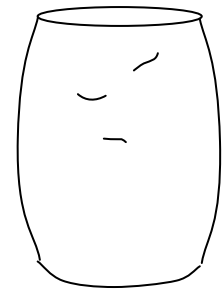
Unconfined Comp. Strength: 2.74 tsf

Strain at Failure (%): 20.0

W.C. (%): 17.8

Yd (pcf): 115.6

Sketch of Specimen After Failure



Boring: Geo-053 Depth: 35-36.5

Sample #:

Soil Type: Lean Clay w/sand (CL)

Strain Rate (in/min): 0.030

Sample Type: SB

Dia. (in) 1.37 Ht. (in) 2.75

Height to Diameter Ratio: 2.0

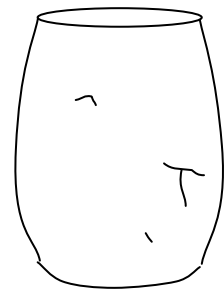
Unconfined Comp. Strength: 3.21 tsf

Strain at Failure (%): 20.0

W.C. (%): 14.0

Yd (pcf): 122.3

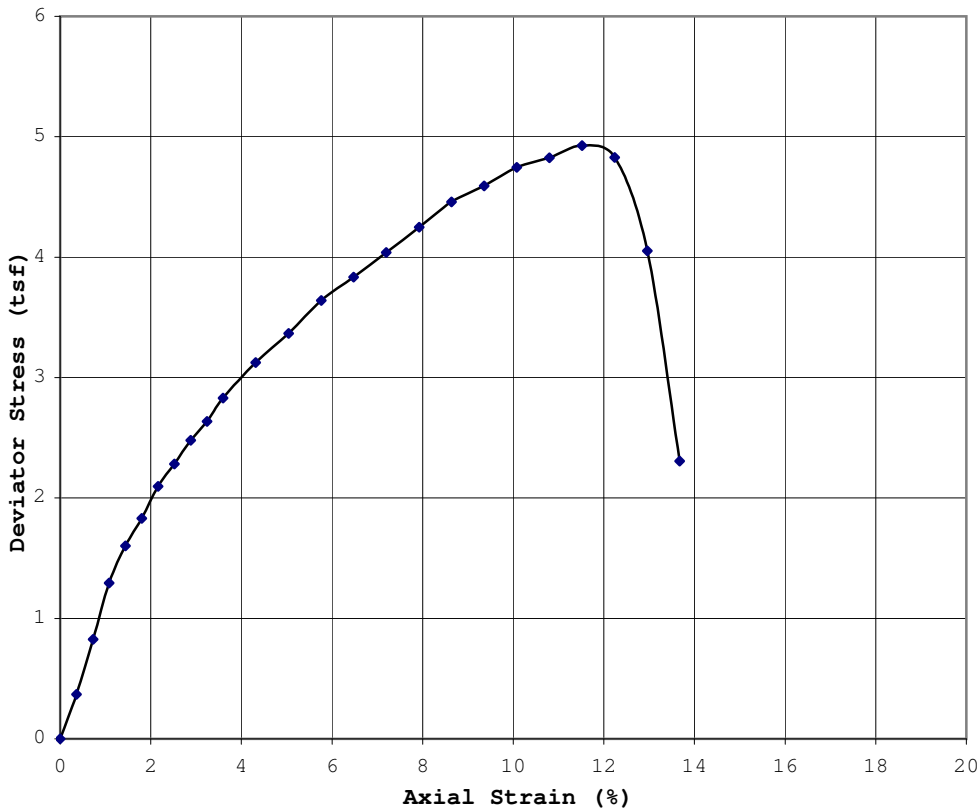
Sketch of Specimen After Failure



Unconfined Stress/Strain Curves

ASTM: D2166

Project: Seneca Wind Job: 11376
 Client: Barr Engineering Company Date: 4/23/18
 Remarks: _____



Boring: Geo-089 Depth: 10-11.5
 Sample #: _____

Soil Type: Sandy Lean Clay (CL)

Strain Rate (in/min): 0.030

Sample Type: SB

Dia. (in) 1.36 Ht. (in) 2.78

Height to Diameter Ratio: 2.0

Unconfined Comp. Strength: 4.93 tsf

Strain at Failure (%): 11.5

W.C. (%): 18.9

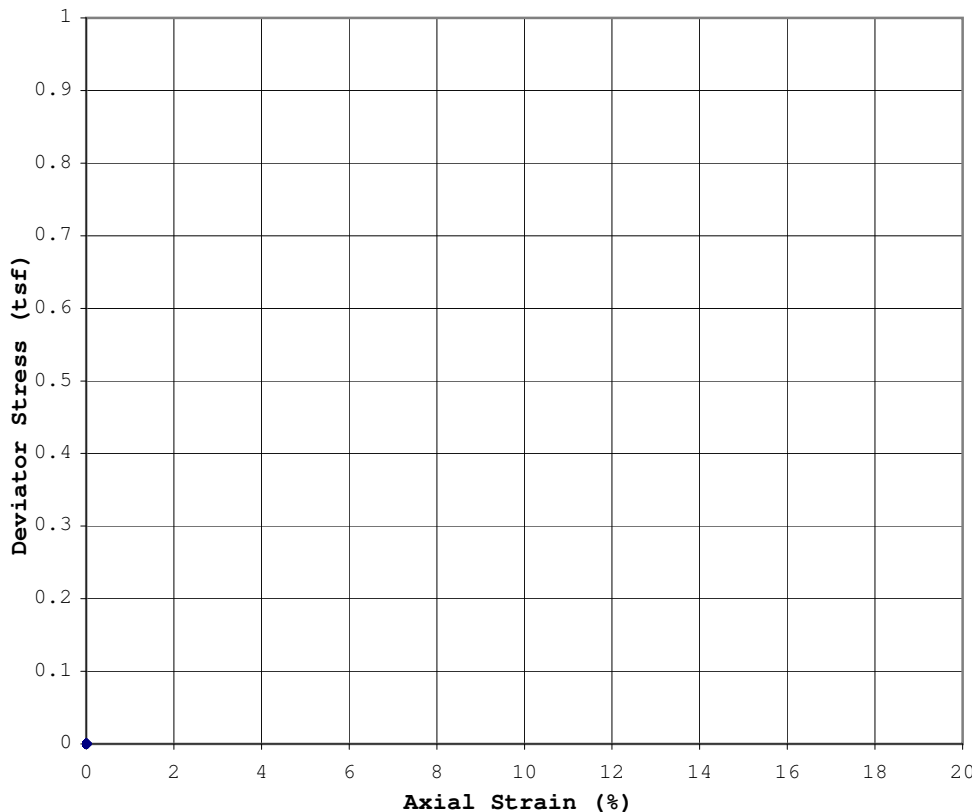
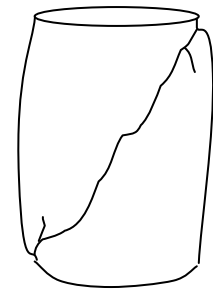
Yd (pcf): 110.7

LL: 36

PL: 19

PI: 17

Sketch of Specimen After Failure



Boring: _____ Depth: _____

Sample #: _____

Soil Type: _____

Strain Rate (in/min): _____

Sample Type: _____

Dia. (in): _____ Ht. (in): _____

Height to Diameter Ratio: _____

Unconfined Comp. Strength: _____ tsf

Strain at Failure (%): _____

W.C. (%): _____

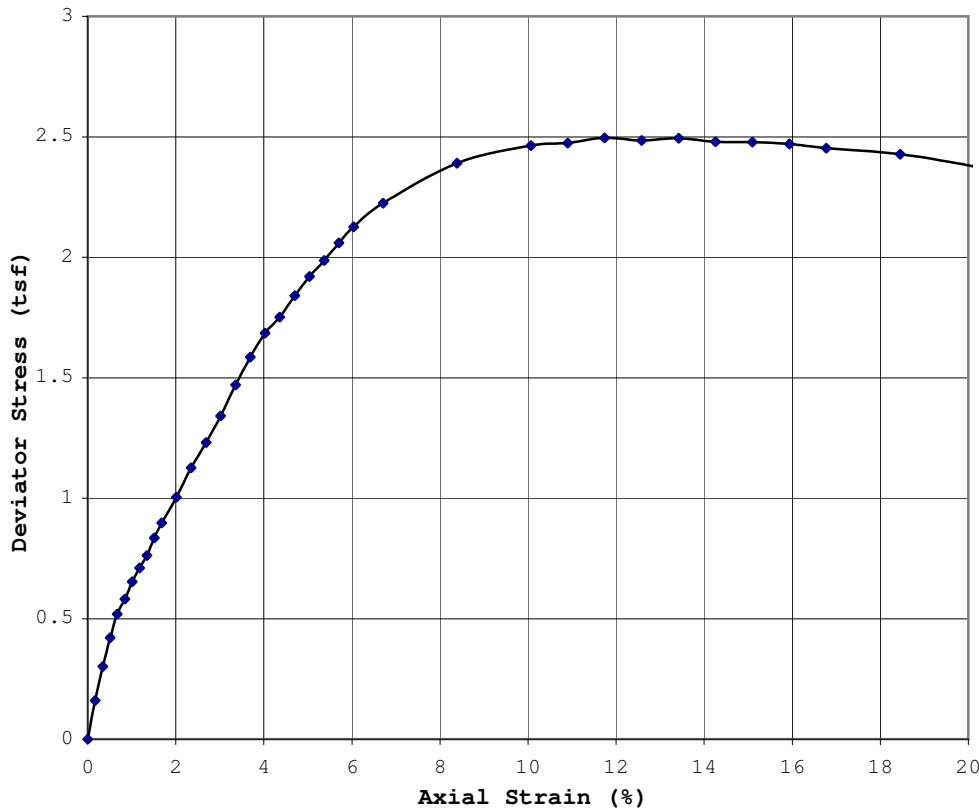
Yd (pcf): _____

Sketch of Specimen After Failure

Unconfined Stress/Strain Curves

ASTM: D2166

Project: Seneca Wind
 Client: Barr Engineering Company
 Job: 11376
 Date: 5/22/18
 Remarks:



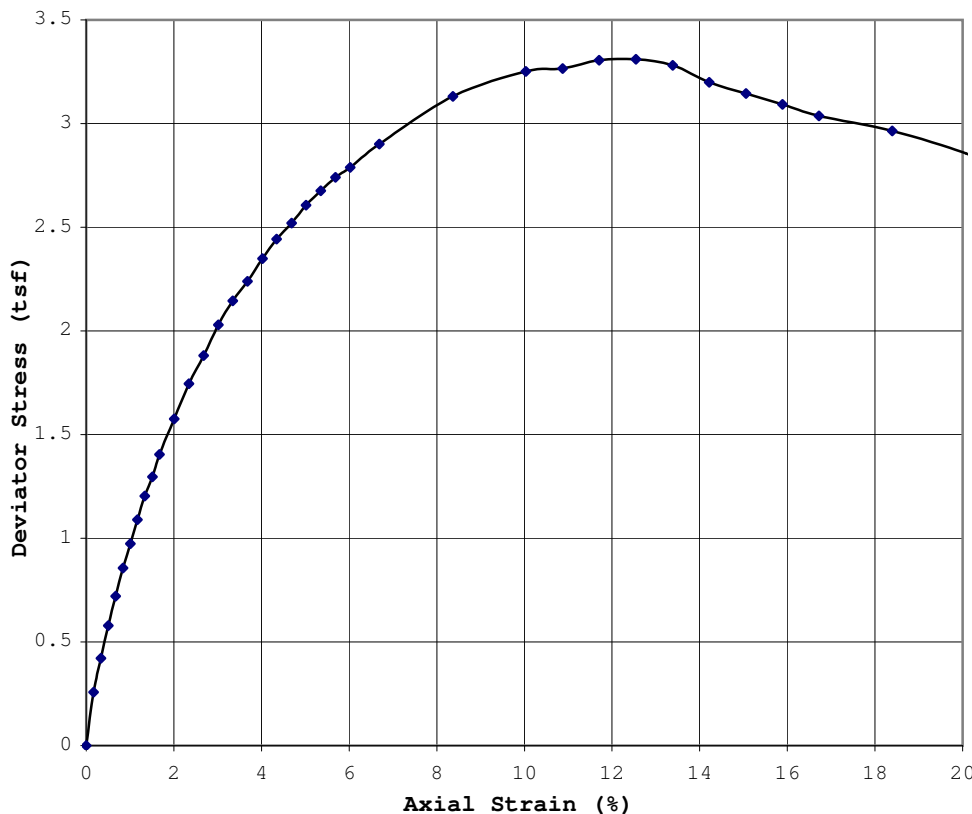
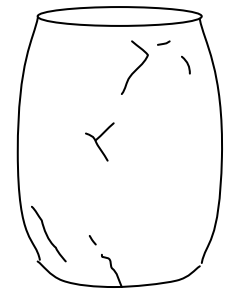
Boring: Geo-005 Depth: 25-26
 Sample #:
 Soil Type: Lean Clay w/sand and a trace of gravel (CL)

Strain Rate (in/min): 0.060
 Sample Type: 3T
 Dia. (in): 2.88 Ht. (in): 5.96
 Height to Diameter Ratio: 2.1

Unconfined Comp. Strength: 2.50 tsf
 Strain at Failure (%): 11.7

W.C. (%): 16.8
 Yd (pcf): 115.0

Sketch of Specimen After Failure



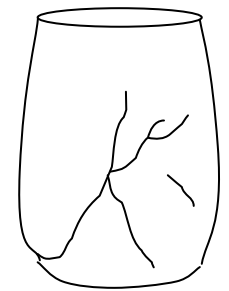
Boring: Geo-006 Depth: 20-21.7
 Sample #:
 Soil Type: Lean Clay w/sand and a little gravel (CL)

Strain Rate (in/min): 0.060
 Sample Type: 3T
 Dia. (in): 2.88 Ht. (in): 5.98
 Height to Diameter Ratio: 2.1

Unconfined Comp. Strength: 3.31 tsf
 Strain at Failure (%): 12.5

W.C. (%): 16.6
 Yd (pcf): 115.9

Sketch of Specimen After Failure

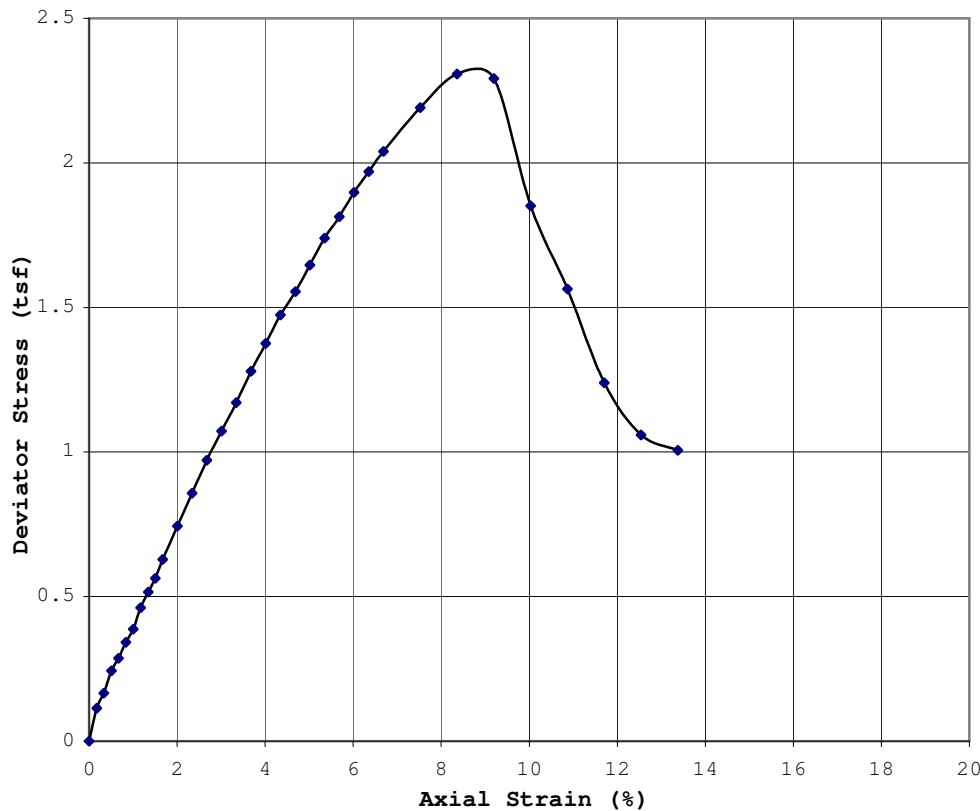


Unconfined Stress/Strain Curves

ASTM: D2166

Project: Seneca Wind
 Client: Barr Engineering Company
 Remarks:

Job: 11376
 Date: 5/22/18



Boring: Geo-006 Depth: 45-47
 Sample #:

Soil Type: Lean Clay w/sand, a little gravel, and a patch of sand (CL)

Strain Rate (in/min): 0.060

Sample Type: 3T

Dia. (in) 2.89 Ht. (in) 5.98

Height to Diameter Ratio: 2.1

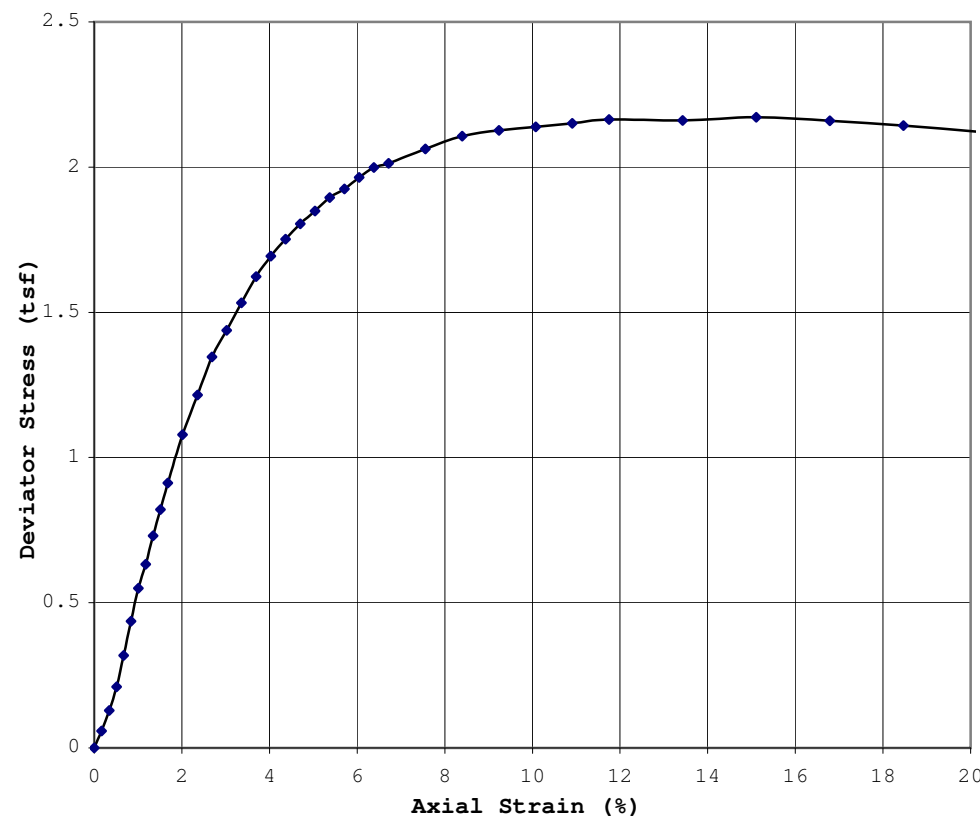
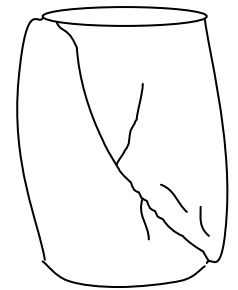
Unconfined Comp. Strength: 2.31 tsf

Strain at Failure (%): 8.4

W.C. (%): 16.7

Yd (pcf): 114.8

Sketch of Specimen After Failure



Boring: Geo-008 Depth: 20-22

Sample #:

Soil Type: Lean Clay w/sand and a trace of gravel (CL)

Strain Rate (in/min): 0.060

Sample Type: 3T

Dia. (in) 2.88 Ht. (in) 5.96

Height to Diameter Ratio: 2.1

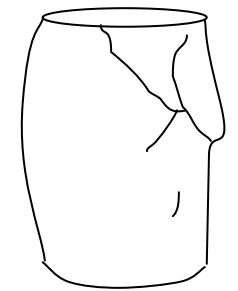
Unconfined Comp. Strength: 2.17 tsf

Strain at Failure (%): 15.1

W.C. (%): 18.5

Yd (pcf): 112.2

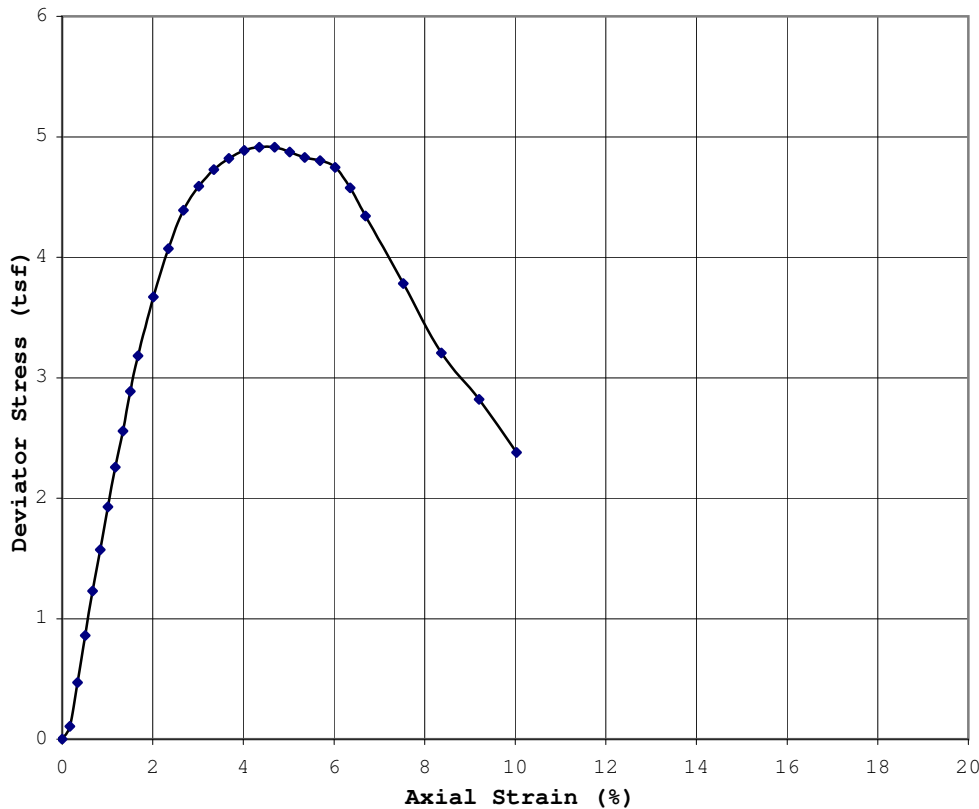
Sketch of Specimen After Failure



Unconfined Stress/Strain Curves

ASTM: D2166

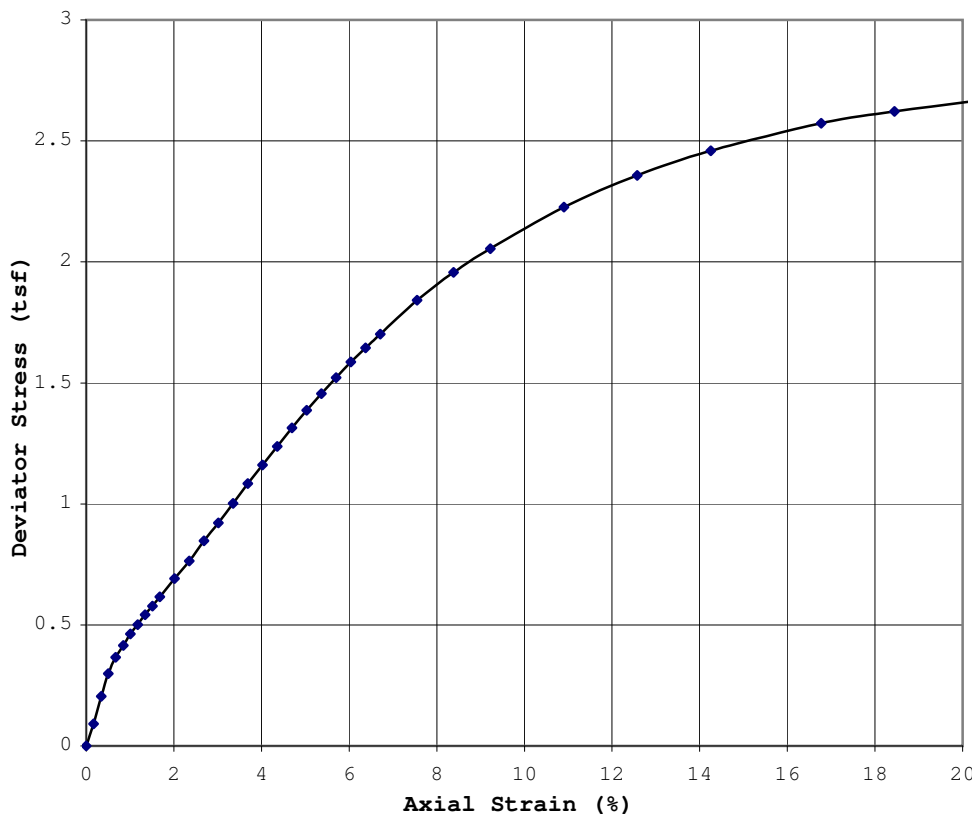
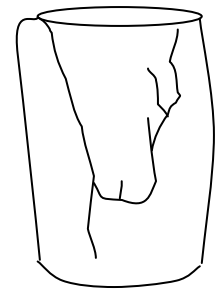
Project: Seneca Wind Job: 11376
 Client: Barr Engineering Company Date: 5/22/18
 Remarks: _____



Boring: Geo-013 Depth: 15-16.5
 Sample #: _____
 Soil Type: Lean Clay w/sand and a little gravel (CL)
 Strain Rate (in/min): 0.060
 Sample Type: 3T
 Dia. (in): 2.88 Ht. (in): 5.98
 Height to Diameter Ratio: 2.1
Unconfined Comp. Strength: 4.91 tsf
Strain at Failure (%): 4.7

W.C. (%): 16.6
 Yd (pcf): 115.2

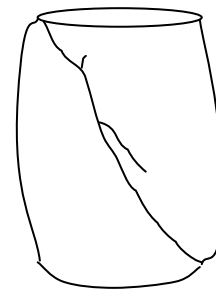
Sketch of Specimen After Failure



Boring: Geo-016 Depth: 28-30
 Sample #: _____
 Soil Type: Lean Clay w/sand and a trace of gravel (CL)
 Strain Rate (in/min): 0.060
 Sample Type: 3T
 Dia. (in): 2.89 Ht. (in): 5.97
 Height to Diameter Ratio: 2.1
Unconfined Comp. Strength: 2.66 tsf
Strain at Failure (%): 20.0

W.C. (%): 16.9
 Yd (pcf): 115.5

Sketch of Specimen After Failure

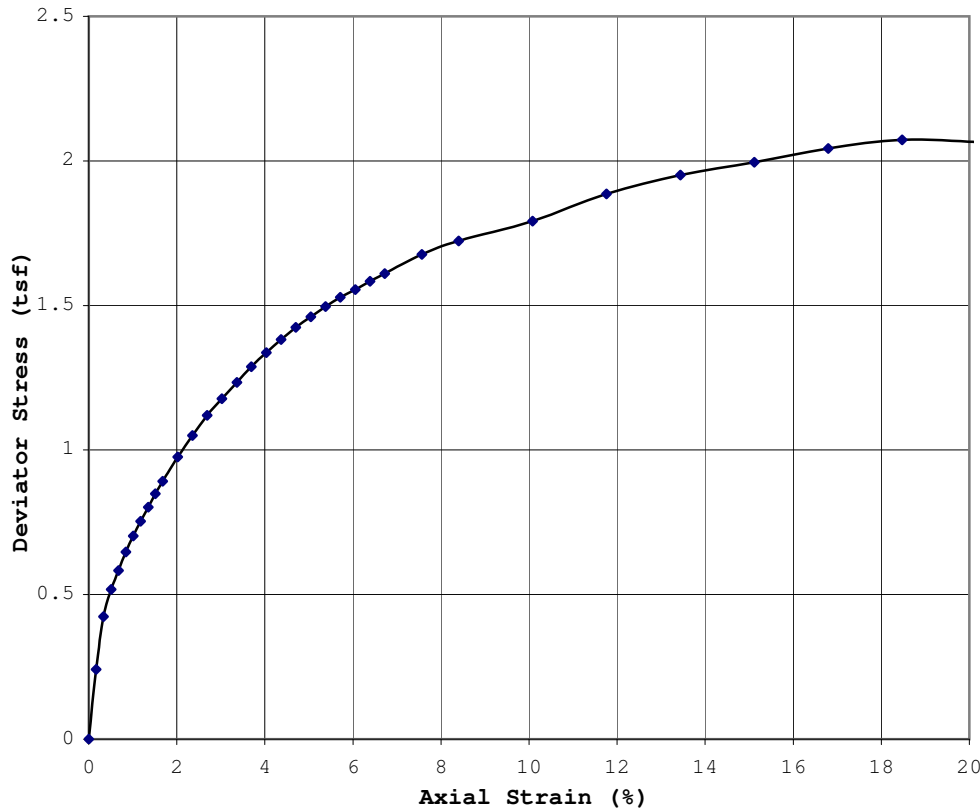


Unconfined Stress/Strain Curves

ASTM: D2166

Project: Seneca Wind
 Client: Barr Engineering Company
 Remarks:

Job: 11376
 Date: 5/22/18



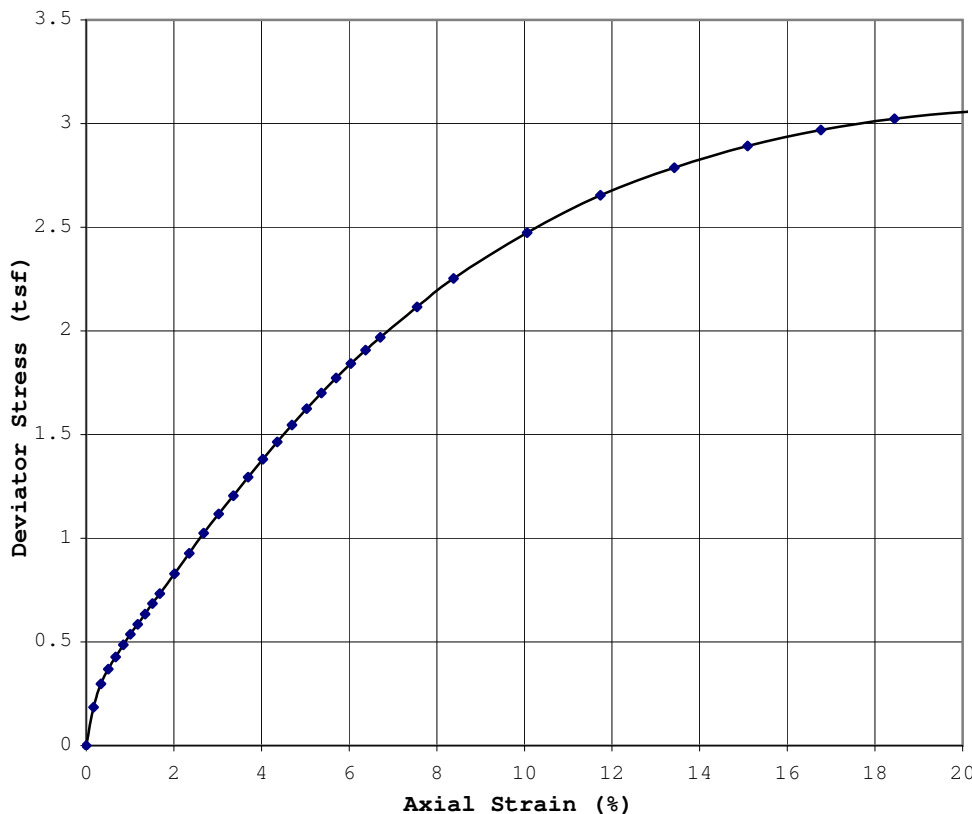
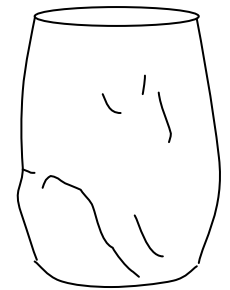
Boring: Geo-033 Depth: 23-25
 Sample #:
 Soil Type: Lean Clay w/sand and a trace of gravel (CL)

Strain Rate (in/min): 0.060
 Sample Type: 3T
 Dia. (in): 2.90 Ht. (in): 5.95
 Height to Diameter Ratio: 2.1

Unconfined Comp. Strength: 2.07 tsf
 Strain at Failure (%): 18.5

W.C. (%): 16.9
 Yd (pcf): 114.9

Sketch of Specimen After Failure



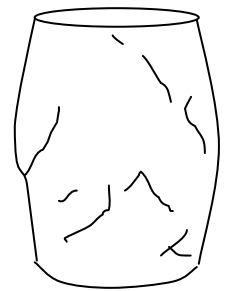
Boring: Geo-034 Depth: 38-40
 Sample #:
 Soil Type: Lean Clay w/sand and a trace of gravel (CL)

Strain Rate (in/min): 0.060
 Sample Type: 3T
 Dia. (in): 2.89 Ht. (in): 5.96
 Height to Diameter Ratio: 2.1

Unconfined Comp. Strength: 3.04 tsf
 Strain at Failure (%): 20.0

W.C. (%): 15.8
 Yd (pcf): 117.3

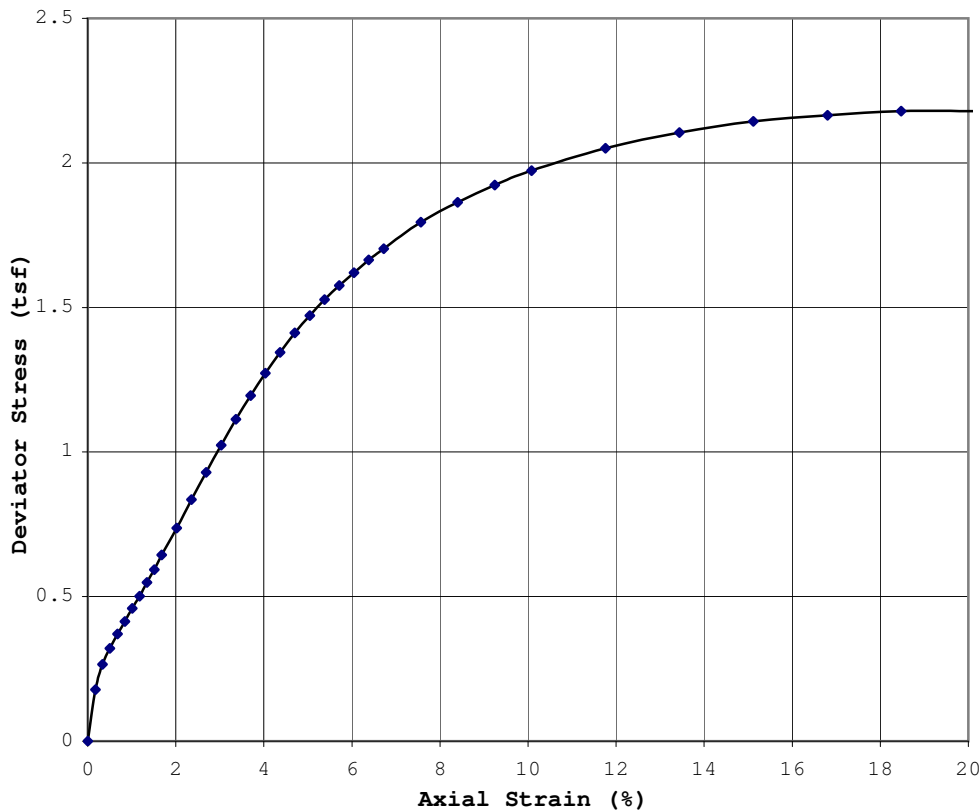
Sketch of Specimen After Failure



Unconfined Stress/Strain Curves

ASTM: D2166

Project: Seneca Wind
 Client: Barr Engineering Company
 Remarks: Job: 11376
 Date: 5/22/18



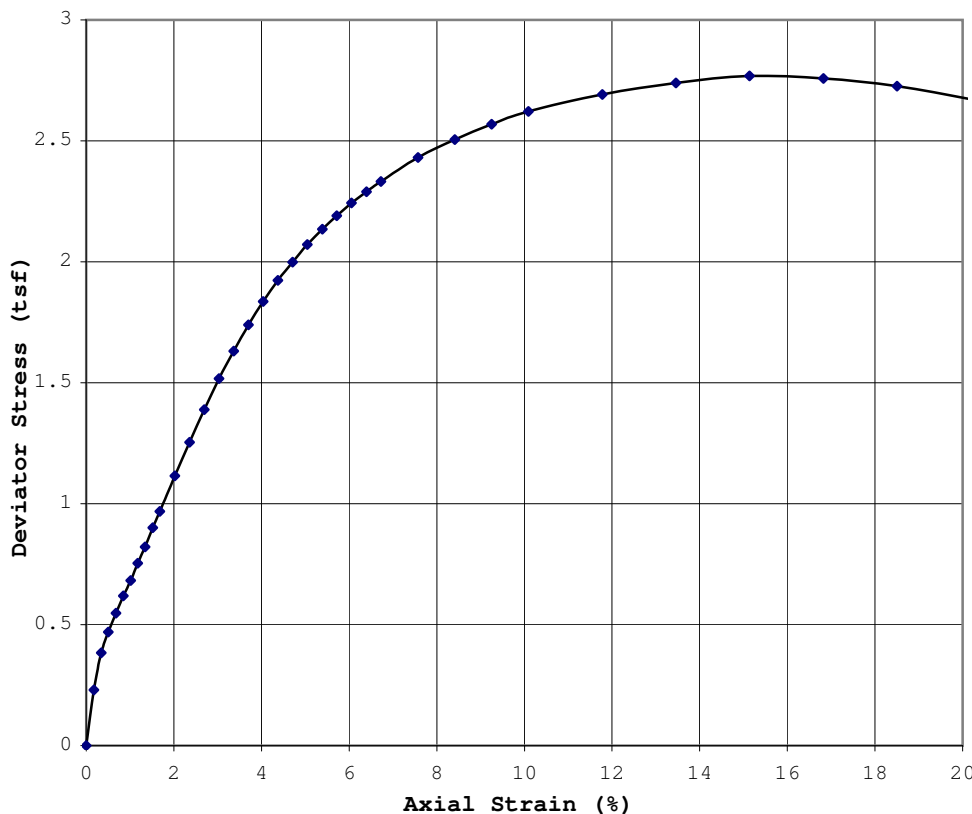
Boring: Geo-035 Depth: 18-20
 Sample #:
 Soil Type: Lean Clay w/sand and a trace of gravel (CL)

Strain Rate (in/min): 0.060
 Sample Type: 3T
 Dia. (in): 2.90 Ht. (in): 5.95
 Height to Diameter Ratio: 2.1

Unconfined Comp. Strength: 2.18 tsf
 Strain at Failure (%): 20.0

W.C. (%): 17.8
 Yd (pcf): 112.8

Sketch of Specimen After Failure



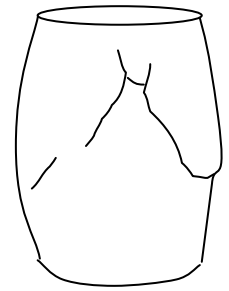
Boring: Geo-035 Depth: 22-24
 Sample #:
 Soil Type: Lean Clay w/sand and a little gravel (CL)

Strain Rate (in/min): 0.060
 Sample Type: 3T
 Dia. (in): 2.89 Ht. (in): 5.95
 Height to Diameter Ratio: 2.1

Unconfined Comp. Strength: 2.77 tsf
 Strain at Failure (%): 15.1

W.C. (%): 17.0
 Yd (pcf): 114.9

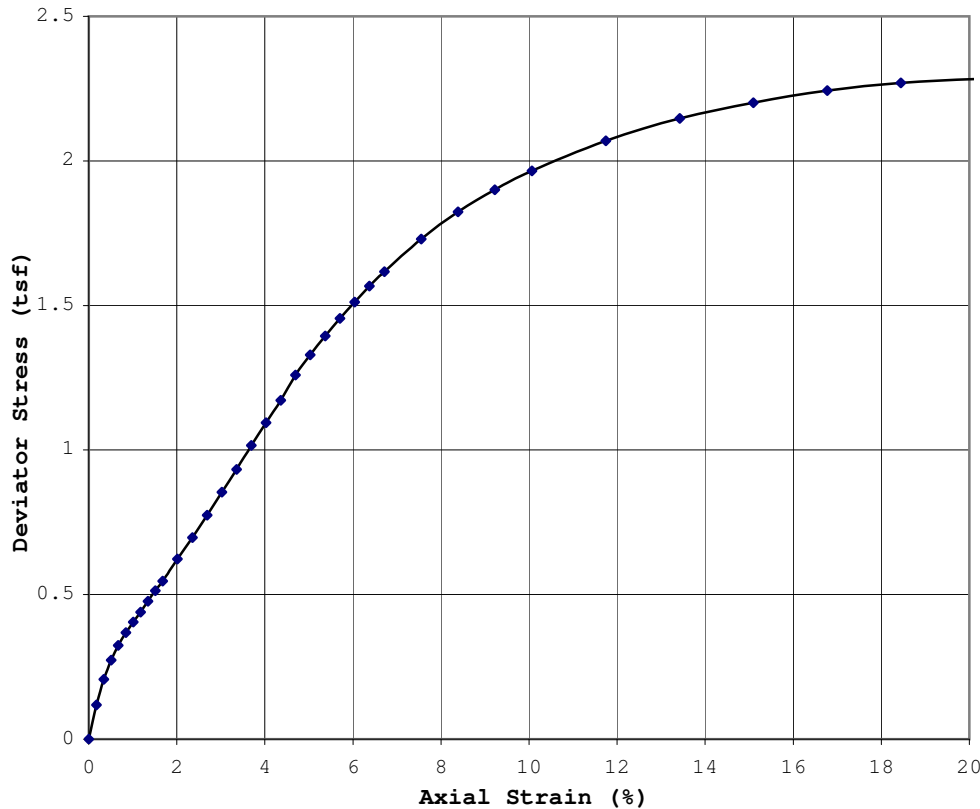
Sketch of Specimen After Failure



Unconfined Stress/Strain Curves

ASTM: D2166

Project: Seneca Wind
 Client: Barr Engineering Company
 Remarks: Job: 11376
 Date: 5/22/18



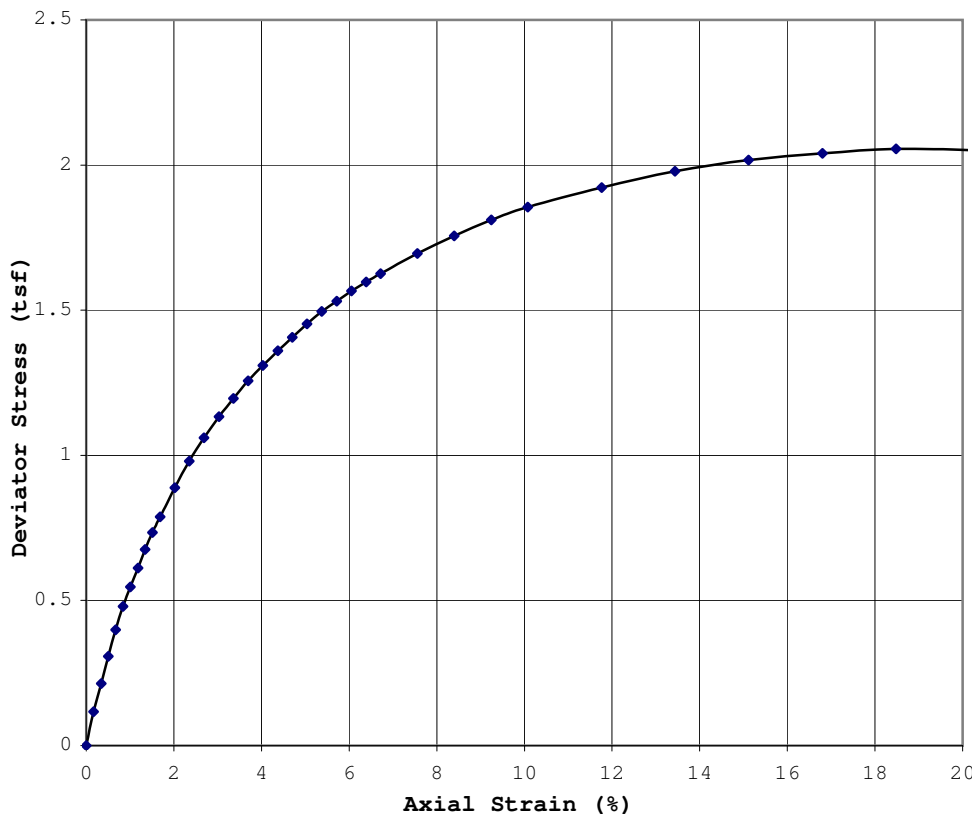
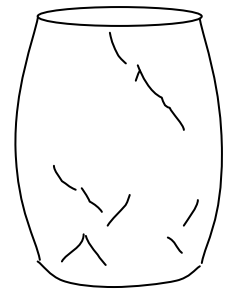
Boring: Geo-035 Depth: 37-39
 Sample #:
 Soil Type: Lean Clay w/sand and a trace of gravel (CL)

Strain Rate (in/min): 0.060
 Sample Type: 3T
 Dia. (in): 2.90 Ht. (in): 5.96
 Height to Diameter Ratio: 2.1

Unconfined Comp. Strength: 2.28 tsf
 Strain at Failure (%): 20.0

W.C. (%): 17.5
 Yd (pcf): 114.0

Sketch of Specimen After Failure



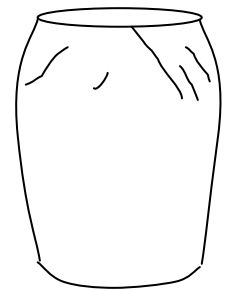
Boring: Geo-036 Depth: 33-35
 Sample #:
 Soil Type: Lean Clay w/sand and a trace of gravel (CL)

Strain Rate (in/min): 0.060
 Sample Type: 3T
 Dia. (in): 2.88 Ht. (in): 5.95
 Height to Diameter Ratio: 2.1

Unconfined Comp. Strength: 2.06 tsf
 Strain at Failure (%): 18.5

W.C. (%): 17.3
 Yd (pcf): 113.8

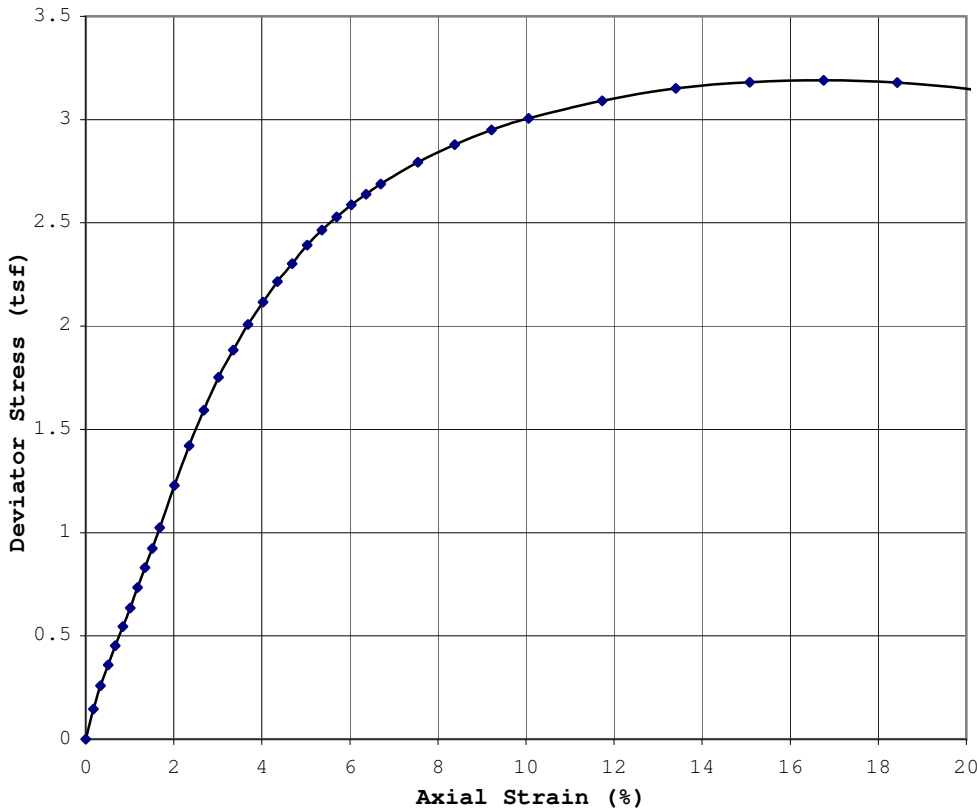
Sketch of Specimen After Failure



Unconfined Stress/Strain Curves

ASTM: D2166

Project: Seneca Wind
 Client: Barr Engineering Company
 Job: 11376
 Date: 5/22/18
 Remarks:



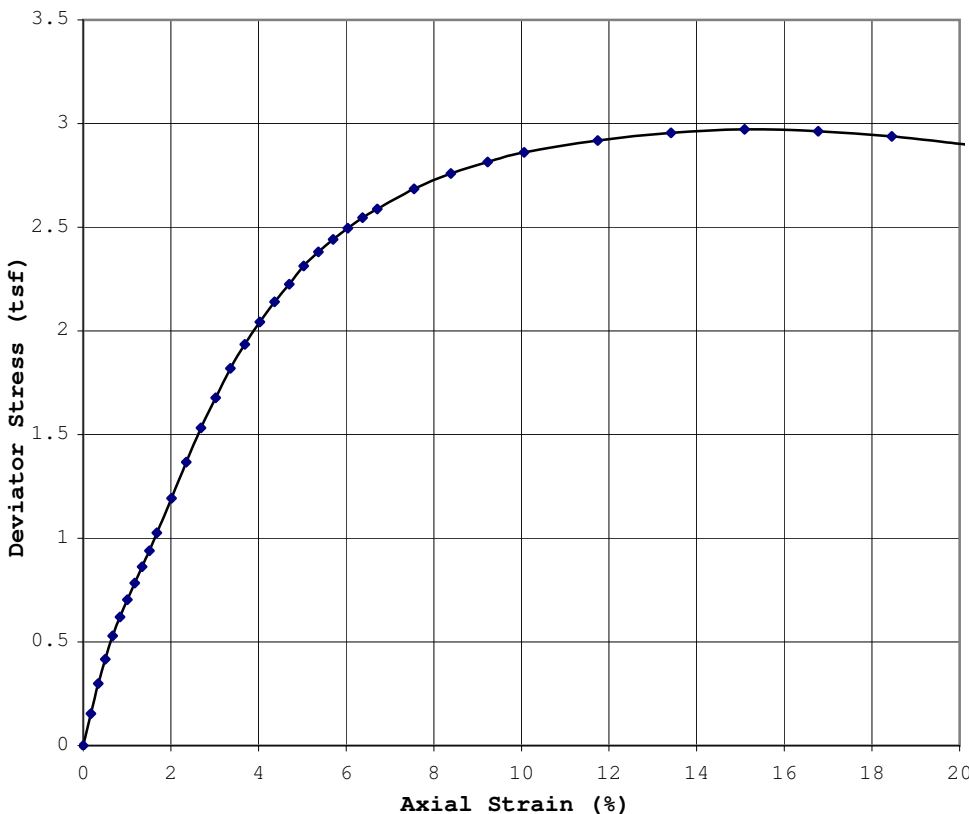
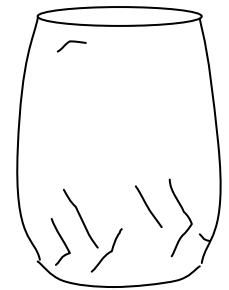
Boring: Geo-037 Depth: 17-19
 Sample #:
 Soil Type: Lean Clay w/sand and a trace of gravel (CL)

Strain Rate (in/min): 0.060
 Sample Type: 3T
 Dia. (in): 2.88 Ht. (in): 5.97
 Height to Diameter Ratio: 2.1

Unconfined Comp. Strength: 3.19 tsf
Strain at Failure (%): 16.8

W.C. (%): 17.0
 Yd (pcf): 115.0

Sketch of Specimen After Failure



Boring: Geo-037 Depth: 33-35
 Sample #:
 Soil Type: Lean Clay w/sand and a trace of gravel (CL)

Strain Rate (in/min): 0.060
 Sample Type: 3T
 Dia. (in): 2.88 Ht. (in): 5.96
 Height to Diameter Ratio: 2.1

Unconfined Comp. Strength: 2.97 tsf
Strain at Failure (%): 15.1

W.C. (%): 17.3
 Yd (pcf): 113.7

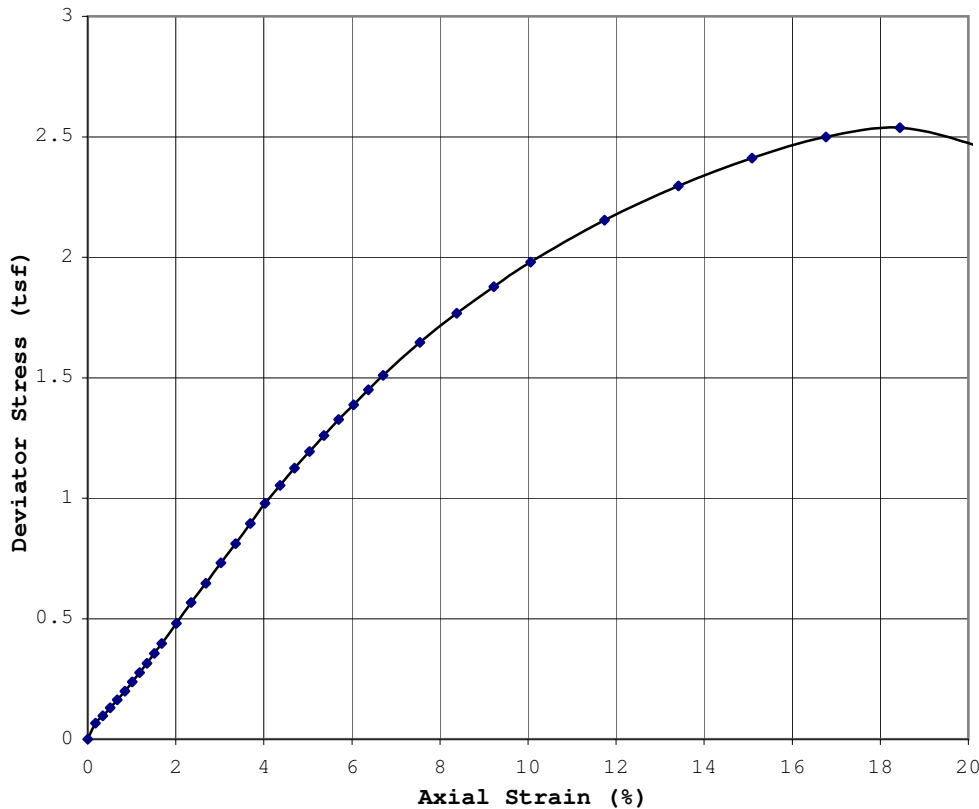
Sketch of Specimen After Failure



Unconfined Stress/Strain Curves

ASTM: D2166

Project: Seneca Wind Job: 11376
 Client: Barr Engineering Company Date: 5/22/18
 Remarks: _____



Boring: Geo-039 Depth: 28-30
 Sample #: _____

Soil Type: Sandy Lean Clay w/a little gravel (CL)

Strain Rate (in/min): 0.060

Sample Type: 3T

Dia. (in) 2.88 Ht. (in) 5.97

Height to Diameter Ratio: 2.1

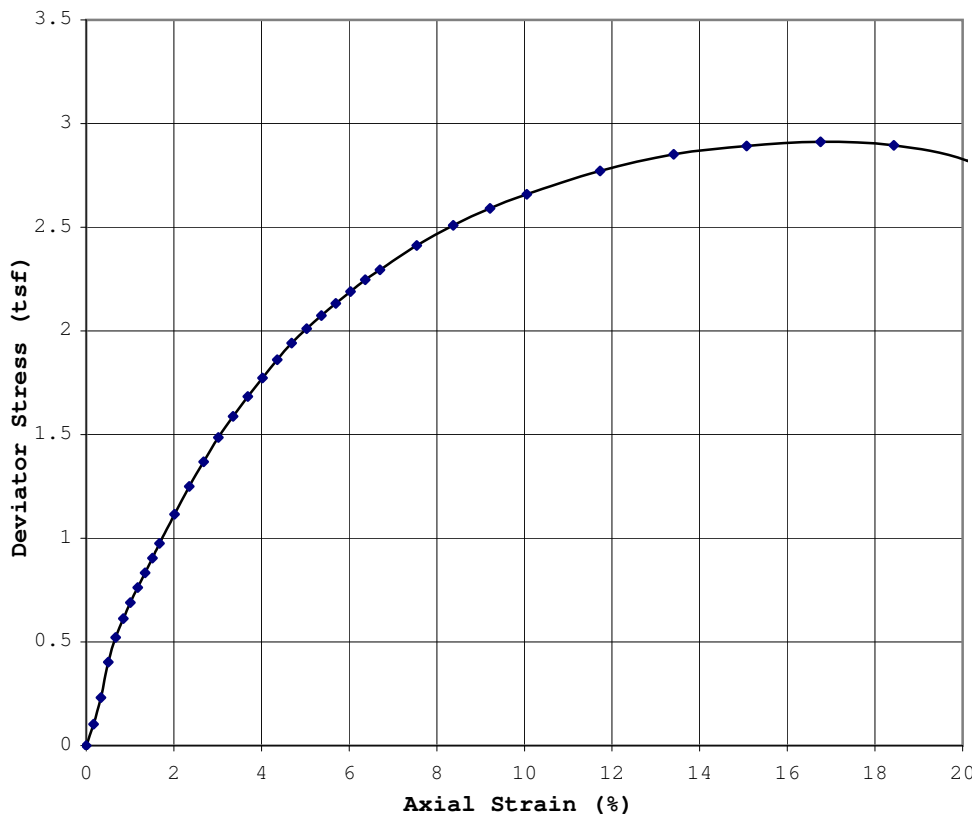
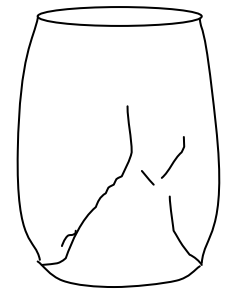
Unconfined Comp. Strength: 2.54 tsf

Strain at Failure (%): 18.4

W.C. (%): 14.0

Yd (pcf): 123.1

Sketch of Specimen After Failure



Boring: Geo-040 Depth: 23-25

Sample #: _____

Soil Type: Lean Clay w/sand and a trace of gravel (CL)

Strain Rate (in/min): 0.060

Sample Type: 3T

Dia. (in) 2.89 Ht. (in) 5.97

Height to Diameter Ratio: 2.1

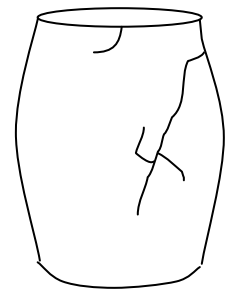
Unconfined Comp. Strength: 2.91 tsf

Strain at Failure (%): 16.8

W.C. (%): 16.6

Yd (pcf): 115.8

Sketch of Specimen After Failure

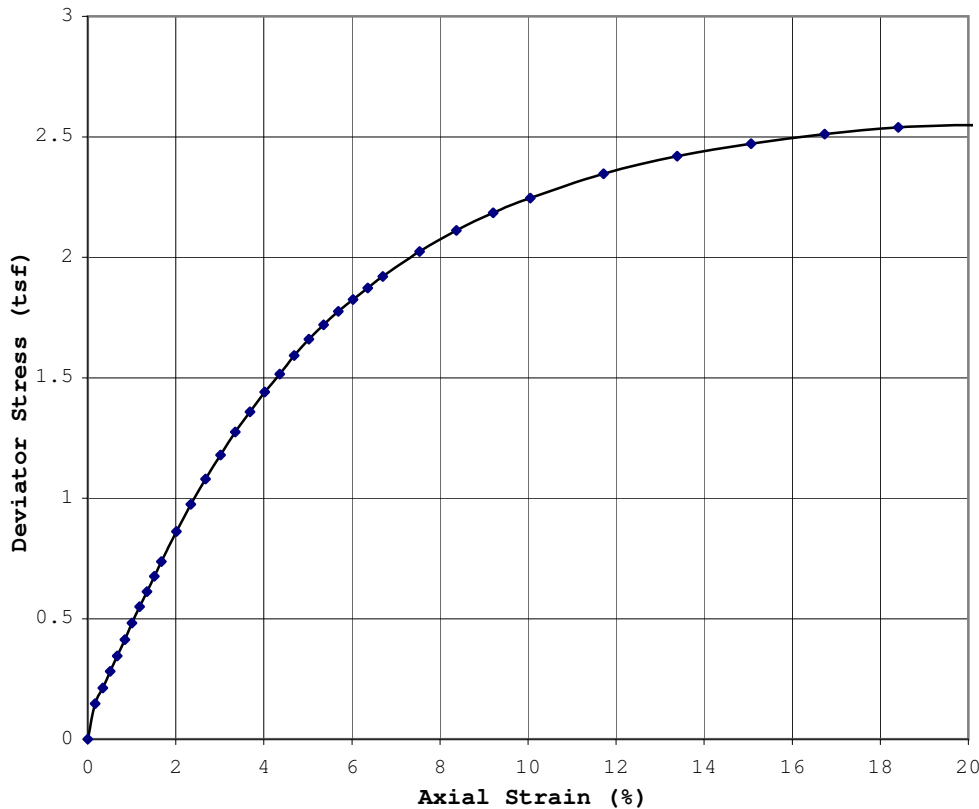


Unconfined Stress/Strain Curves

ASTM: D2166

Project: Seneca Wind
 Client: Barr Engineering Company
 Remarks:

Job: 11376
 Date: 5/22/18



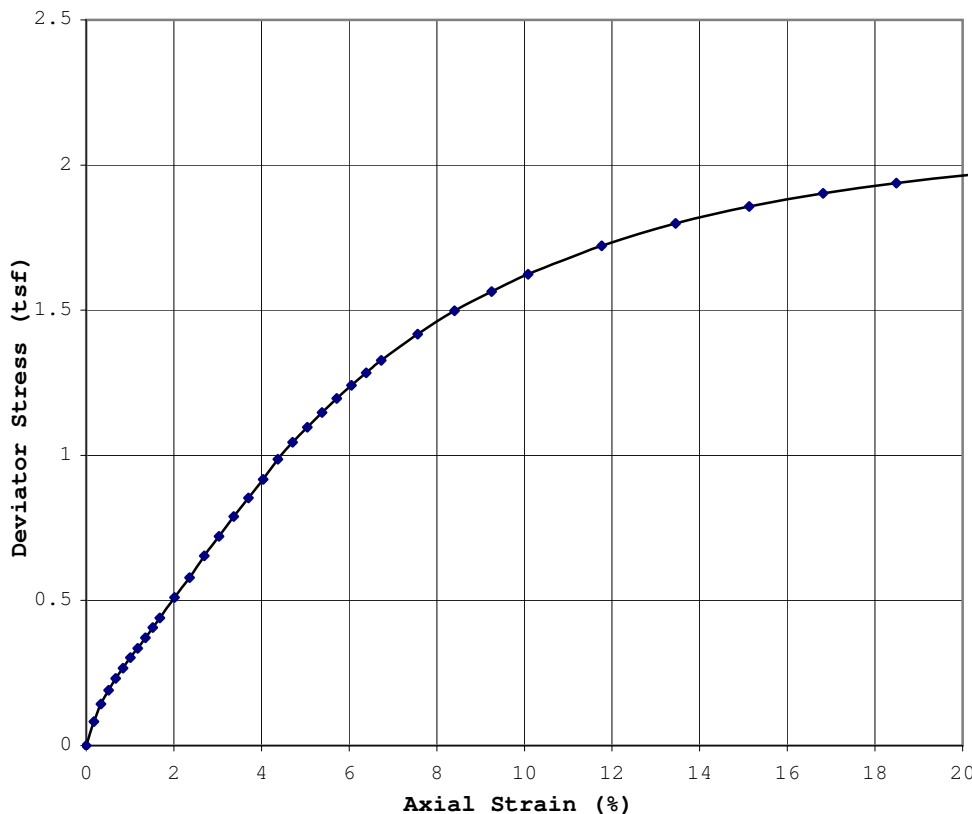
Boring: Geo-041 Depth: 15-17
 Sample #:
 Soil Type: Lean Clay w/sand and a trace of gravel (CL)

Strain Rate (in/min): 0.060
 Sample Type: 3T
 Dia. (in): 2.88 Ht. (in): 5.98
 Height to Diameter Ratio: 2.1

Unconfined Comp. Strength: 2.55 tsf
 Strain at Failure (%): 20.0

W.C. (%): 17.5
 Yd (pcf): 114.3

Sketch of Specimen After Failure



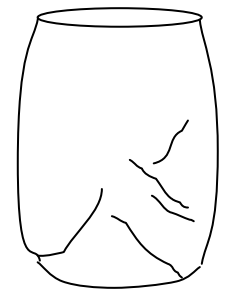
Boring: Geo-043 Depth: 30-32
 Sample #:
 Soil Type: Lean Clay w/a trace of gravel (CL)

Strain Rate (in/min): 0.060
 Sample Type: 3T
 Dia. (in): 2.89 Ht. (in): 5.95
 Height to Diameter Ratio: 2.1

Unconfined Comp. Strength: 1.97 tsf
 Strain at Failure (%): 20.0

W.C. (%): 18.6
 Yd (pcf): 112.2

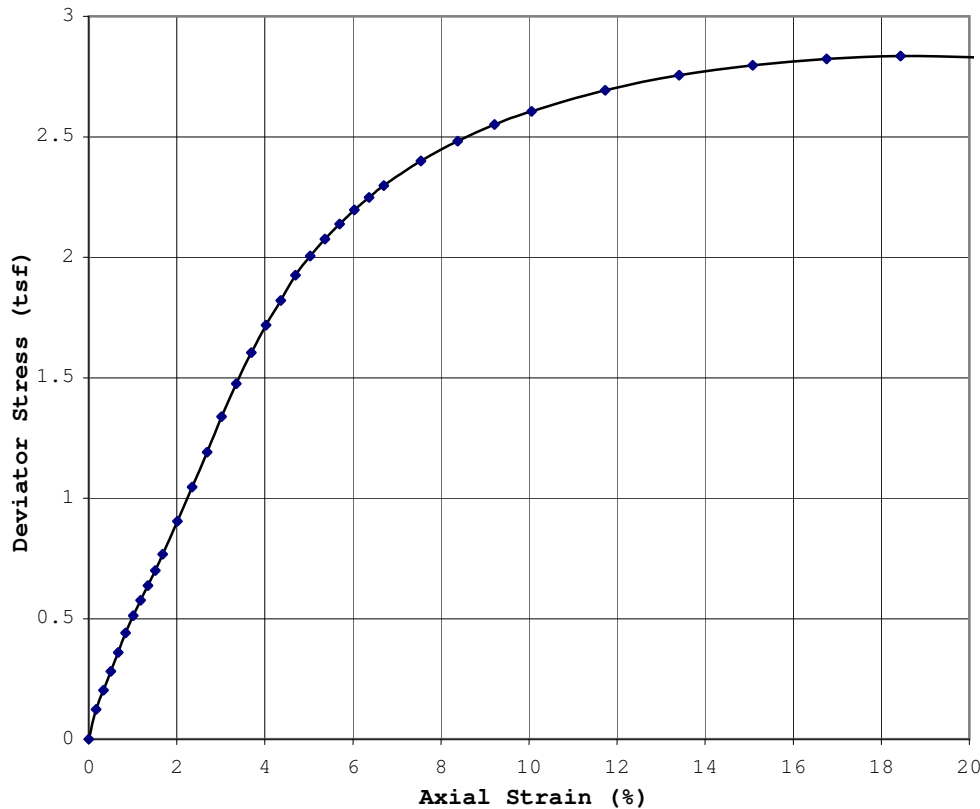
Sketch of Specimen After Failure



Unconfined Stress/Strain Curves

ASTM: D2166

Project: Seneca Wind Job: 11376
 Client: Barr Engineering Company Date: 5/22/18
 Remarks: _____



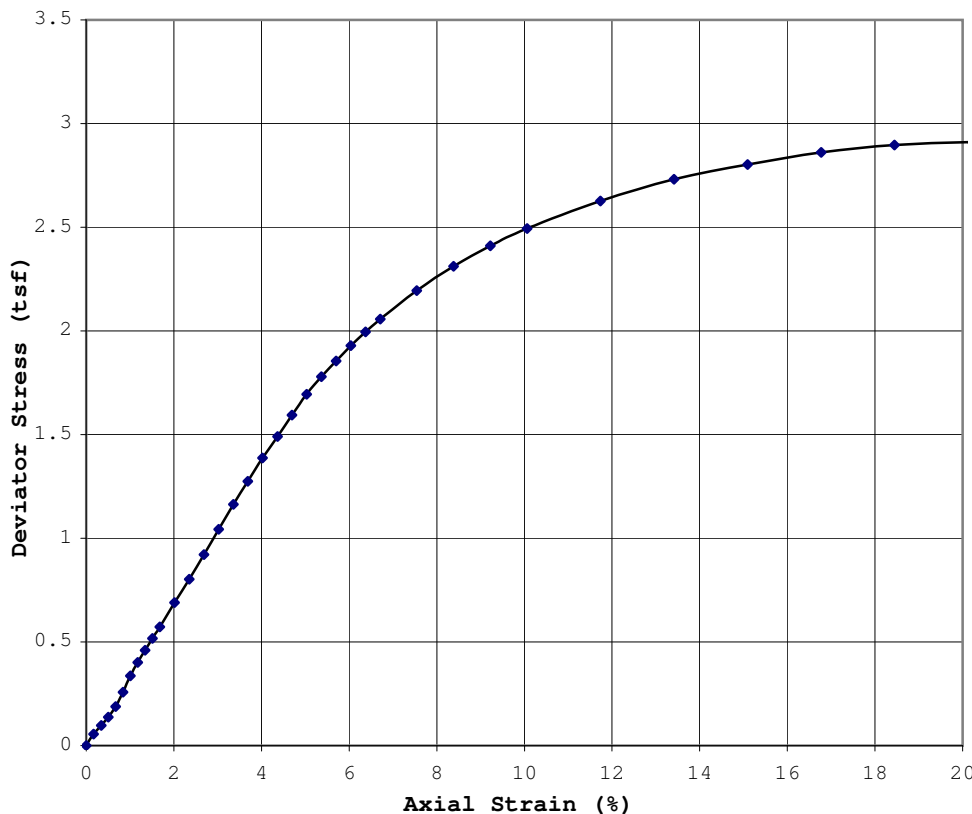
Boring: Geo-044 Depth: 25-27
 Sample #: _____
 Soil Type: Lean Clay w/sand and a trace of gravel (CL)

Strain Rate (in/min): 0.060
 Sample Type: 3T
 Dia. (in): 2.89 Ht. (in): 5.97
 Height to Diameter Ratio: 2.1

Unconfined Comp. Strength: 2.84 tsf
Strain at Failure (%): 18.4

W.C. (%): 17.7
 Yd (pcf): 113.8

Sketch of Specimen After Failure



Boring: Geo-056 Depth: 17-19
 Sample #: _____
 Soil Type: Lean Clay w/sand and a trace of gravel (CL)

Strain Rate (in/min): 0.060
 Sample Type: 3T
 Dia. (in): 2.88 Ht. (in): 5.96
 Height to Diameter Ratio: 2.1

Unconfined Comp. Strength: 2.90 tsf
Strain at Failure (%): 20.0

W.C. (%): 17.1
 Yd (pcf): 115.4

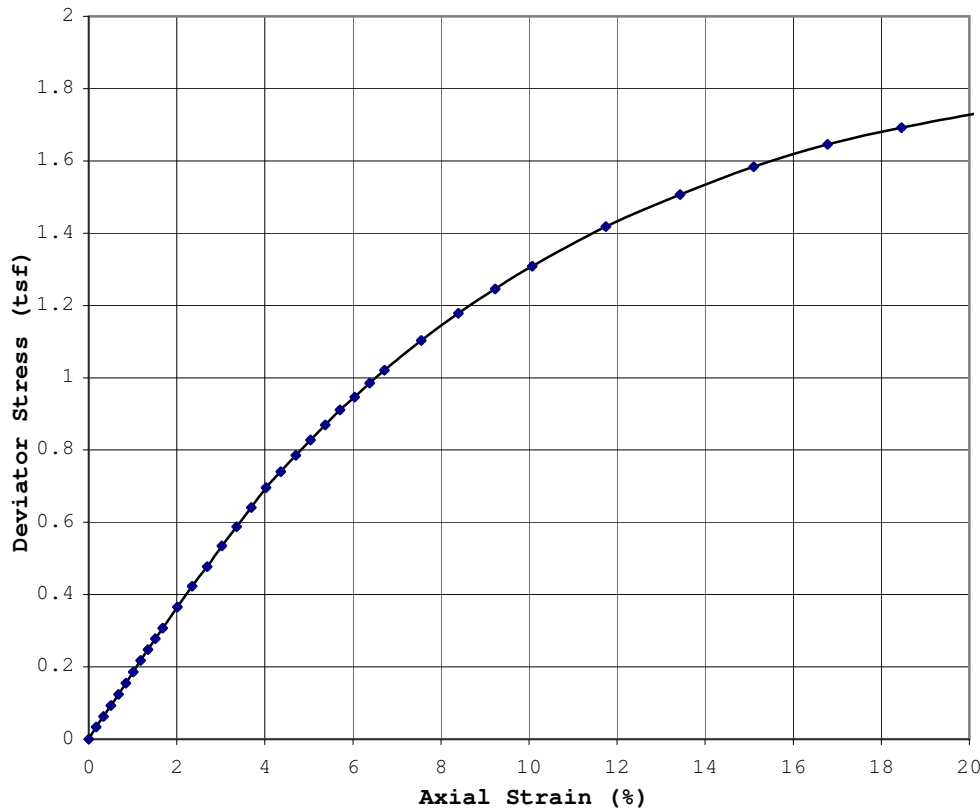
Sketch of Specimen After Failure



Unconfined Stress/Strain Curves

ASTM: D2166

Project: Seneca Wind
 Client: Barr Engineering Company
 Job: 11376
 Date: 5/22/18
 Remarks:



Boring: Geo-056 Depth: 43-45
 Sample #:

Soil Type: Sandy Lean Clay w/a little gravel (CL)

Strain Rate (in/min): 0.060

Sample Type: 3T

Dia. (in): 2.88 Ht. (in): 5.96

Height to Diameter Ratio: 2.1

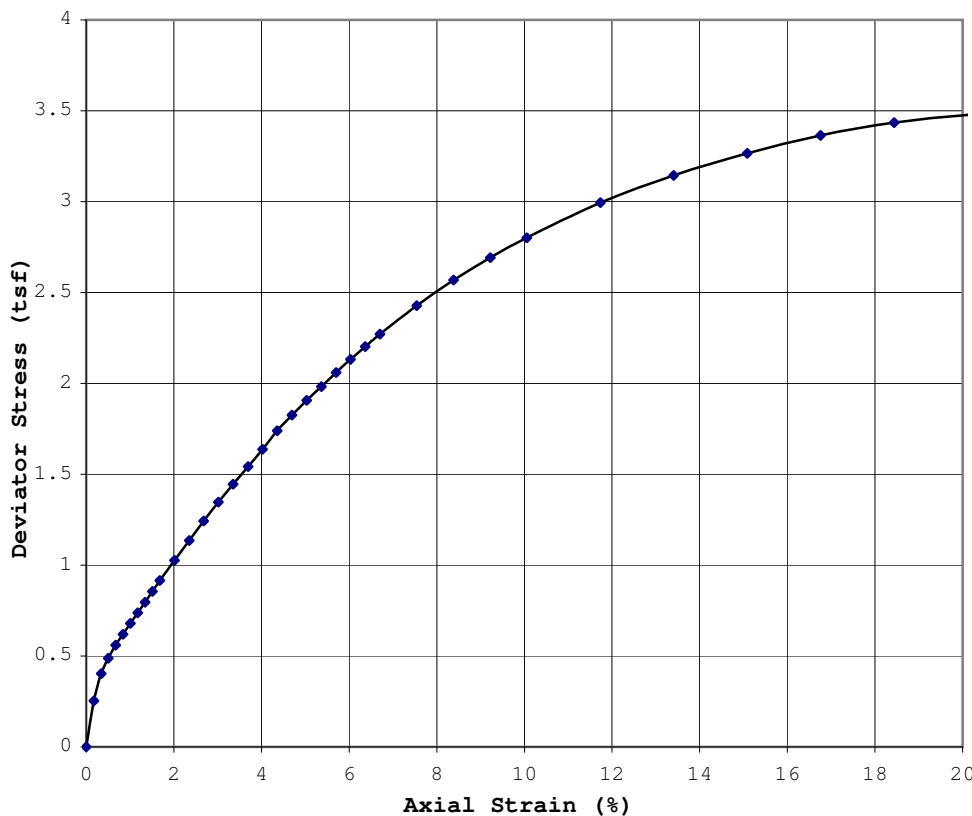
Unconfined Comp. Strength: 1.73 tsf

Strain at Failure (%): 20.0

W.C. (%): 16.7

Yd (pcf): 116.9

Sketch of Specimen After Failure



Boring: Geo-059 Depth: 18-20

Sample #:

Soil Type: Lean Clay w/sand and a trace of gravel (CL)

Strain Rate (in/min): 0.060

Sample Type: 3T

Dia. (in): 2.89 Ht. (in): 5.97

Height to Diameter Ratio: 2.1

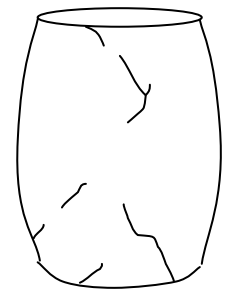
Unconfined Comp. Strength: 3.47 tsf

Strain at Failure (%): 20.0

W.C. (%): 17.0

Yd (pcf): 115.3

Sketch of Specimen After Failure

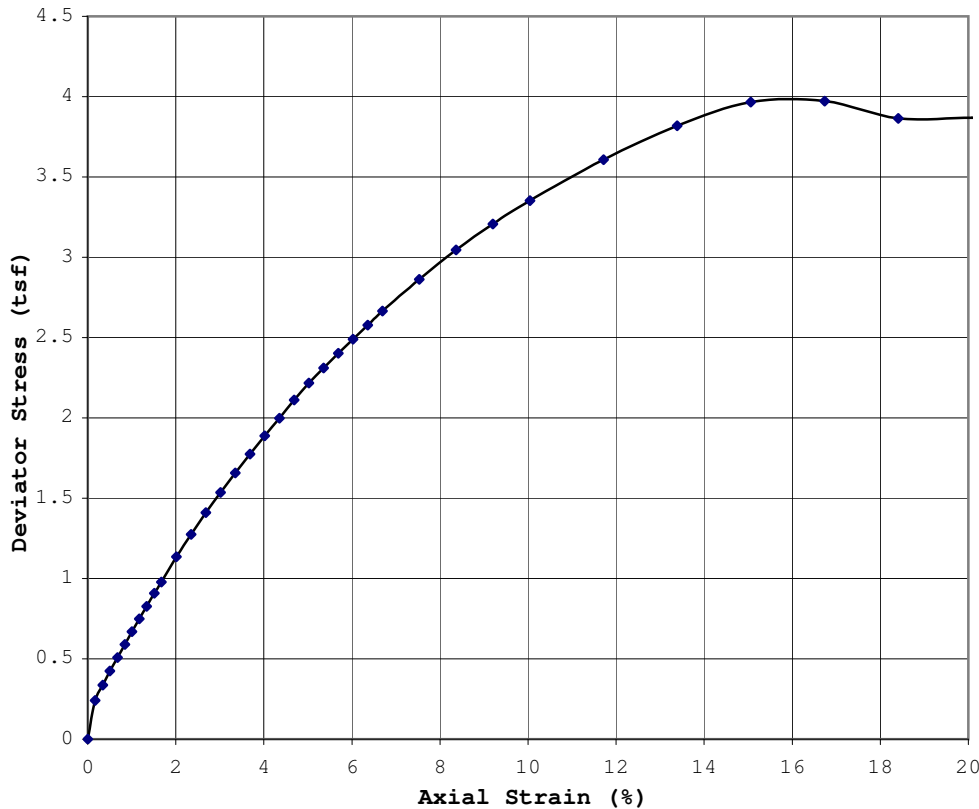


Unconfined Stress/Strain Curves

ASTM: D2166

Project: Seneca Wind
 Client: Barr Engineering Company
 Remarks:

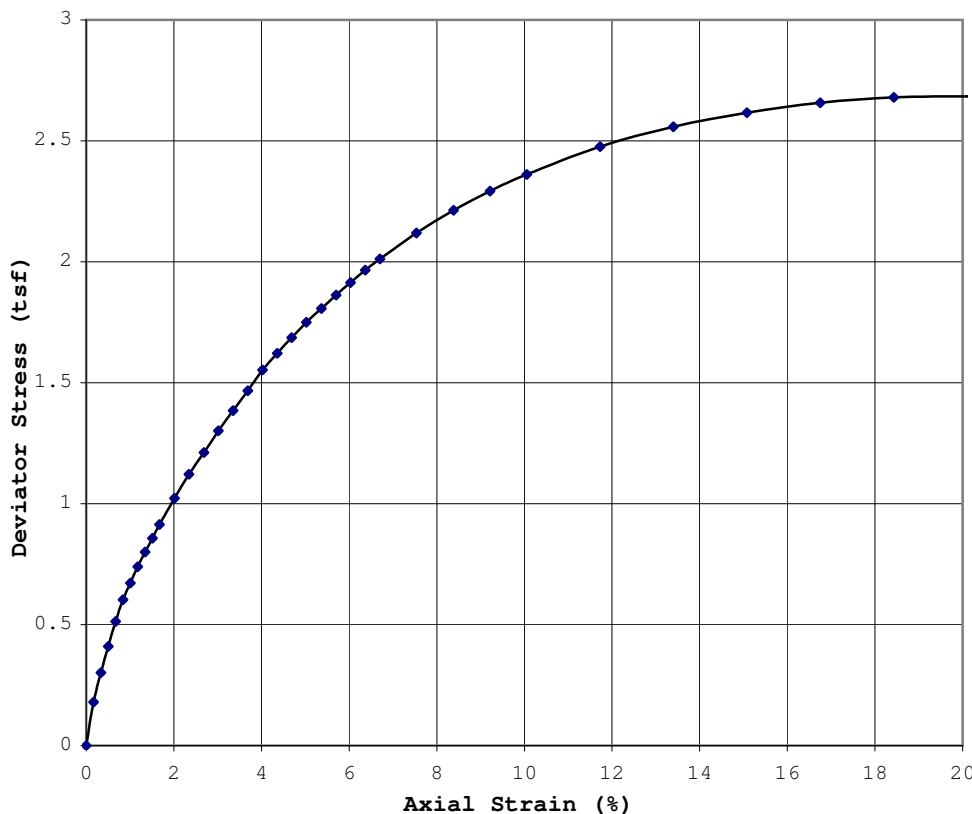
Job: 11376
 Date: 5/22/18



Boring: Geo-059 Depth: 47-49
 Sample #:
 Soil Type: Lean Clay w/sand and a little gravel (CL)
 Strain Rate (in/min): 0.060
 Sample Type: 3T
 Dia. (in): 2.89 Ht. (in): 5.98
 Height to Diameter Ratio: 2.1
 Unconfined Comp. Strength: 3.97 tsf
 Strain at Failure (%): 16.7

W.C. (%): 14.8
 Yd (pcf): 119.9

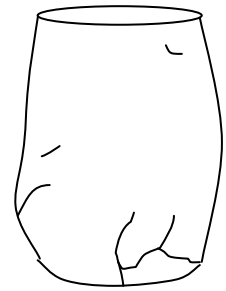
Sketch of Specimen After Failure



Boring: Geo-063 Depth: 23-25
 Sample #:
 Soil Type: Lean Clay w/sand and a trace of gravel (CL)
 Strain Rate (in/min): 0.060
 Sample Type: 3T
 Dia. (in): 2.89 Ht. (in): 5.97
 Height to Diameter Ratio: 2.1
 Unconfined Comp. Strength: 2.68 tsf
 Strain at Failure (%): 20.0

W.C. (%): 16.6
 Yd (pcf): 115.6

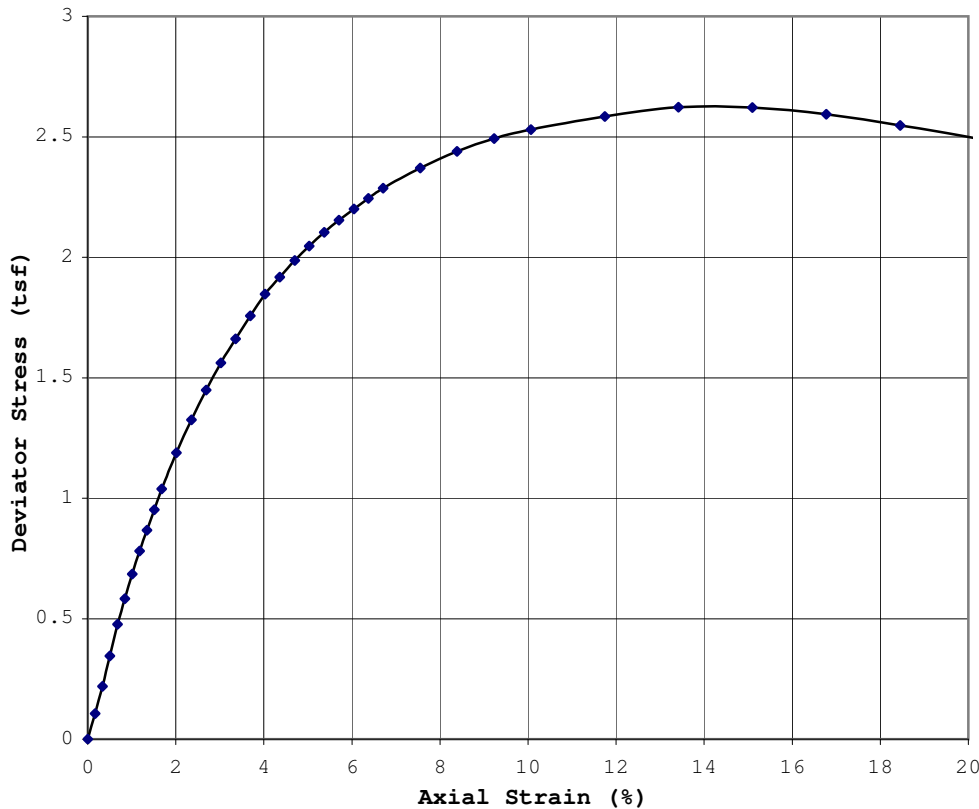
Sketch of Specimen After Failure



Unconfined Stress/Strain Curves

ASTM: D2166

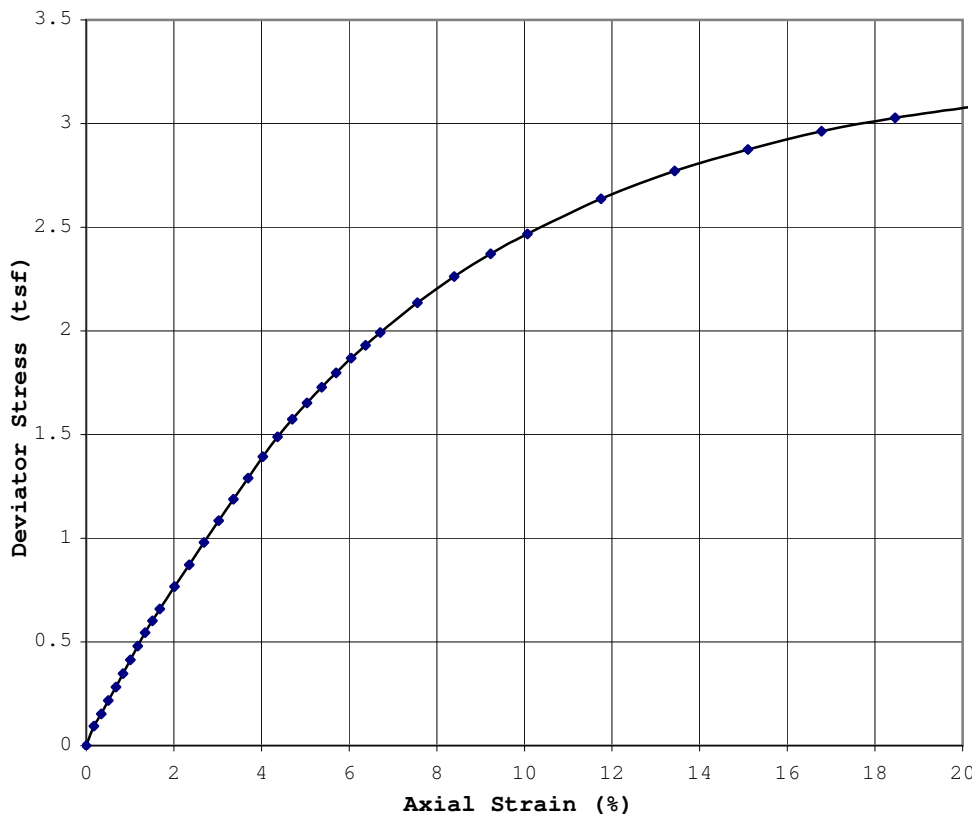
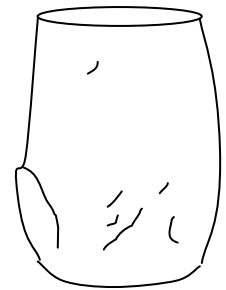
Project: Seneca Wind Job: 11376
 Client: Barr Engineering Company Date: 5/22/18
 Remarks: _____



Boring: Geo-071 Depth: 15-17
 Sample #: _____
 Soil Type: Lean Clay w/sand and a little gravel (CL)
 Strain Rate (in/min): 0.060
 Sample Type: 3T
 Dia. (in): 2.89 Ht. (in): 5.96
 Height to Diameter Ratio: 2.1
 Unconfined Comp. Strength: 2.62 tsf
 Strain at Failure (%): 13.4

W.C. (%): 16.4
 Yd (pcf): 116.3

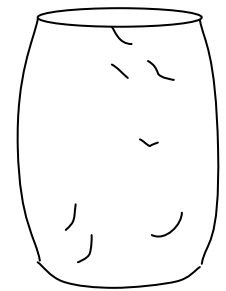
Sketch of Specimen After Failure



Boring: Geo-072 Depth: 18-20
 Sample #: _____
 Soil Type: Lean Clay w/sand and a trace of gravel (CL)
 Strain Rate (in/min): 0.060
 Sample Type: 3T
 Dia. (in): 2.89 Ht. (in): 5.96
 Height to Diameter Ratio: 2.1
 Unconfined Comp. Strength: 3.09 tsf
 Strain at Failure (%): 20.0

W.C. (%): 16.6
 Yd (pcf): 115.9

Sketch of Specimen After Failure

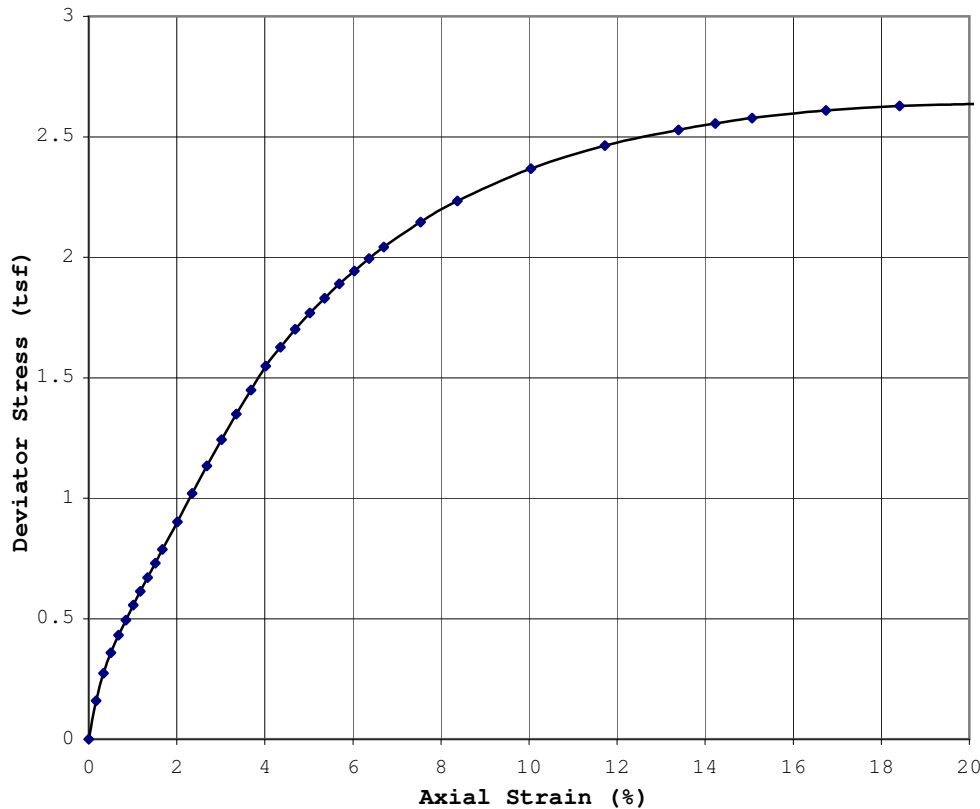


Unconfined Stress/Strain Curves

ASTM: D2166

Project: Seneca Wind
 Client: Barr Engineering Company
 Remarks:

Job: 11376
 Date: 5/22/18



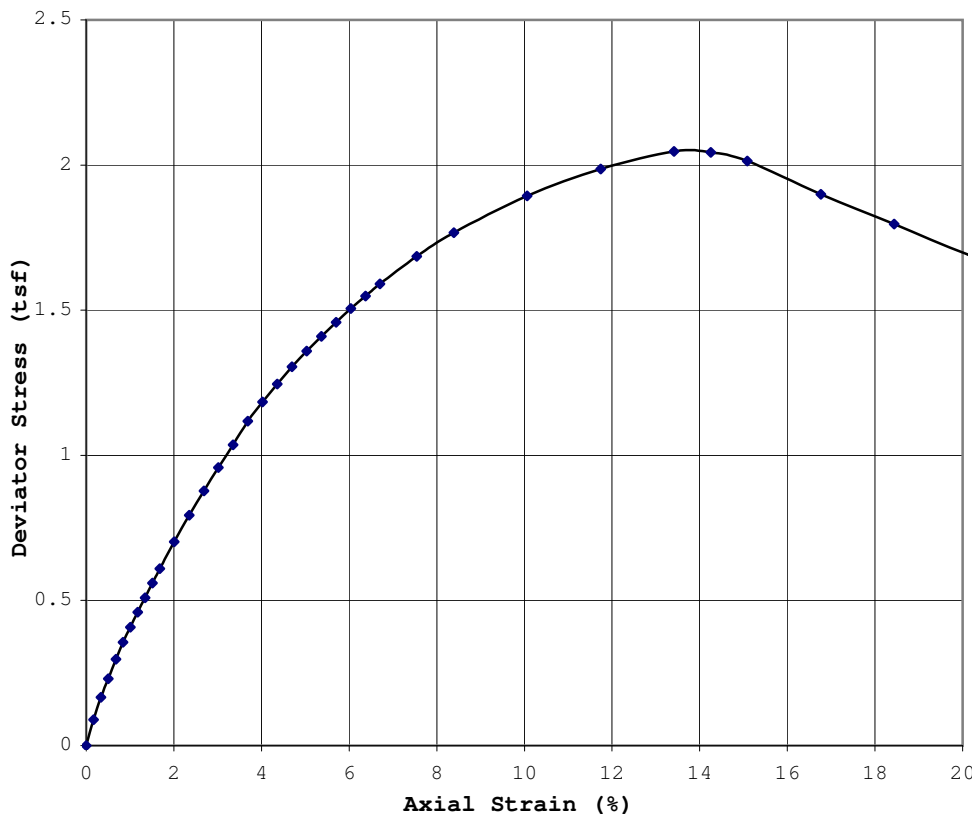
Boring: Geo-074 Depth: 23-25
 Sample #:
 Soil Type: Lean Clay w/sand and a trace of gravel (CL)

Strain Rate (in/min): 0.060
 Sample Type: 3T
 Dia. (in): 2.89 Ht. (in): 5.97
 Height to Diameter Ratio: 2.1

Unconfined Comp. Strength: 2.64 tsf
 Strain at Failure (%): 20.0

W.C. (%): 16.9
 Yd (pcf): 115.3

Sketch of Specimen After Failure



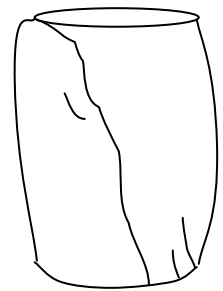
Boring: Geo-077 Depth: 22-24
 Sample #:
 Soil Type: Lean Clay w/sand and a trace of gravel (CL)

Strain Rate (in/min): 0.060
 Sample Type: 3T
 Dia. (in): 2.89 Ht. (in): 5.97
 Height to Diameter Ratio: 2.1

Unconfined Comp. Strength: 2.05 tsf
 Strain at Failure (%): 13.4

W.C. (%): 17.4
 Yd (pcf): 114.2

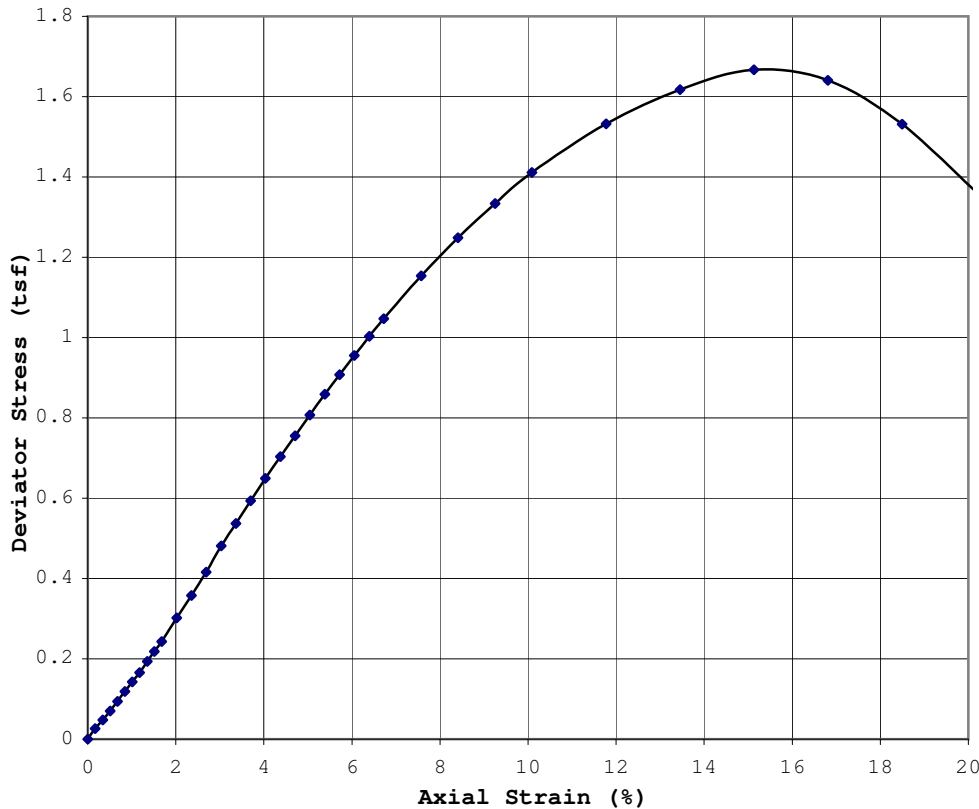
Sketch of Specimen After Failure



Unconfined Stress/Strain Curves

ASTM: D2166

Project: Seneca Wind
 Client: Barr Engineering Company
 Job: 11376
 Date: 5/22/18
 Remarks:



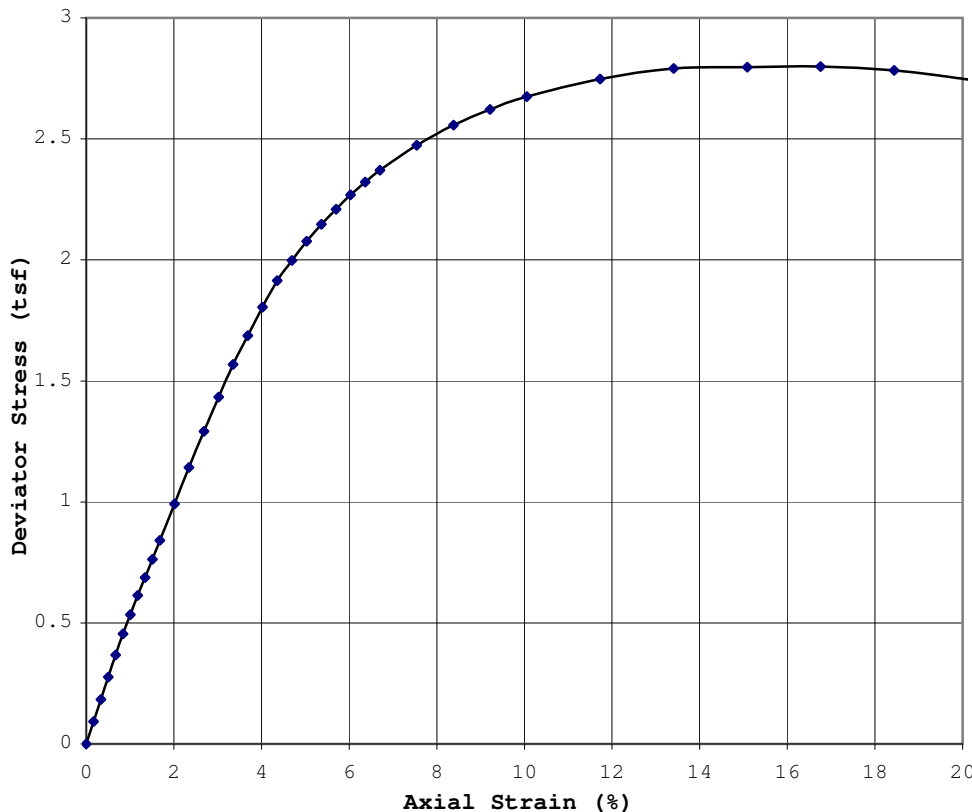
Boring: Geo-077 Depth: 45-47
 Sample #:
 Soil Type: Lean Clay w/sand and a trace of gravel (CL)

Strain Rate (in/min): 0.060
 Sample Type: 3T
 Dia. (in): 2.88 Ht. (in): 5.95
 Height to Diameter Ratio: 2.1

Unconfined Comp. Strength: 1.67 tsf
 Strain at Failure (%): 15.1

W.C. (%): 16.9
 Yd (pcf): 117.2

Sketch of Specimen After Failure



Boring: Geo-079 Depth: 20-22
 Sample #:
 Soil Type: Lean Clay w/sand and a trace of gravel (CL)

Strain Rate (in/min): 0.060
 Sample Type: 3T
 Dia. (in): 2.89 Ht. (in): 5.97
 Height to Diameter Ratio: 2.1

Unconfined Comp. Strength: 2.80 tsf
 Strain at Failure (%): 16.8

W.C. (%): 17.2
 Yd (pcf): 114.8

Sketch of Specimen After Failure

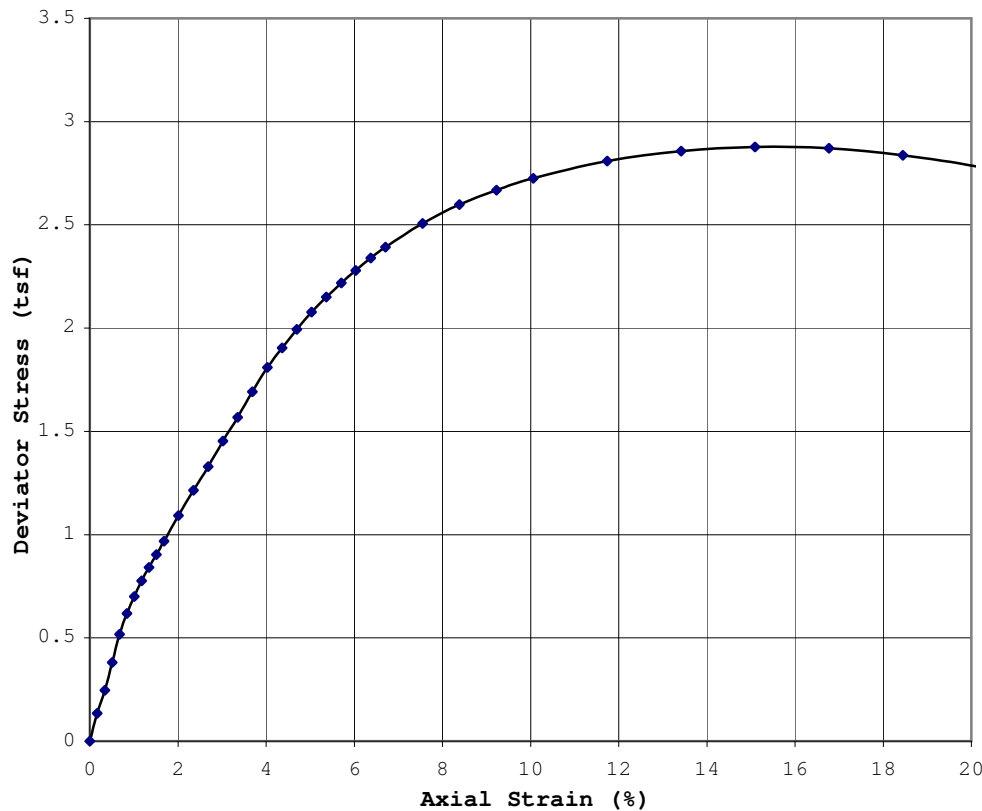


Unconfined Stress/Strain Curves

ASTM: D2166

Project: Seneca Wind
 Client: Barr Engineering Company
 Remarks:

Job: 11376
 Date: 5/22/18



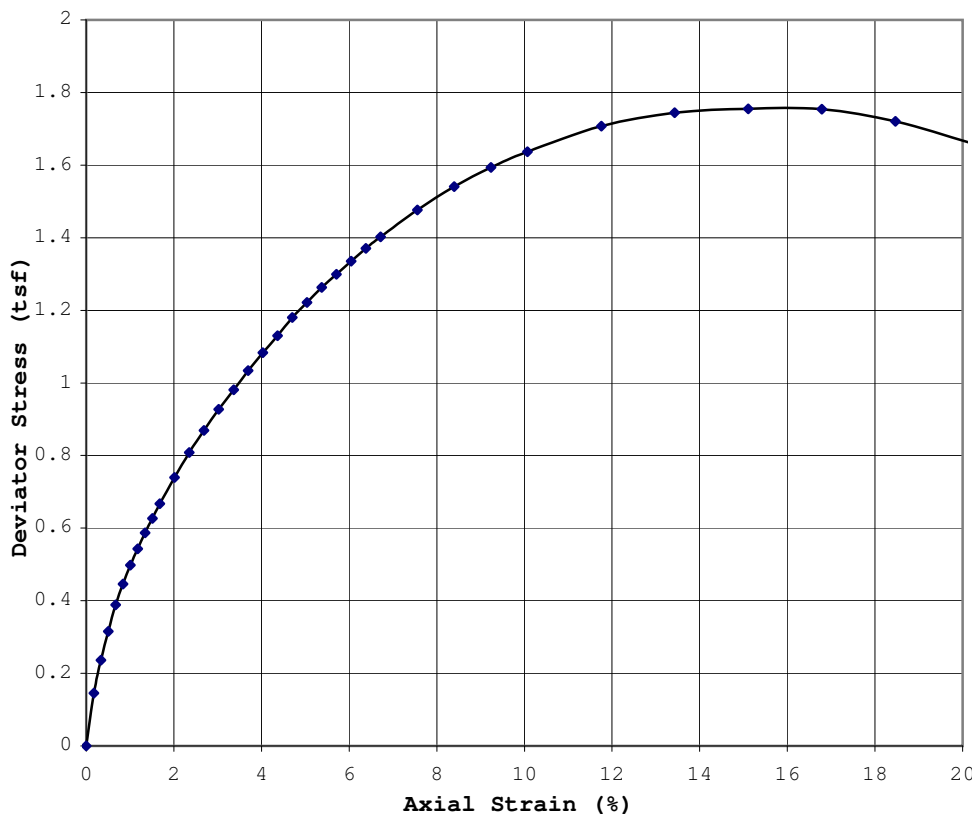
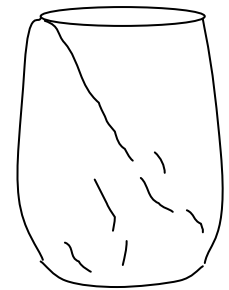
Boring: Geo-079 Depth: 35-37
 Sample #:
 Soil Type: Lean Clay w/sand and a little gravel (CL)

Strain Rate (in/min): 0.060
 Sample Type: 3T
 Dia. (in): 2.89 Ht. (in): 5.96
 Height to Diameter Ratio: 2.1

Unconfined Comp. Strength: 2.88 tsf
Strain at Failure (%): 15.1

W.C. (%): 17.6
 Yd (pcf): 113.9

Sketch of Specimen After Failure



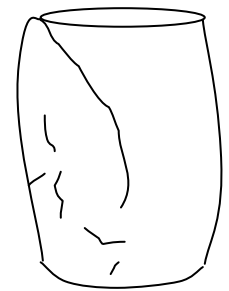
Boring: Geo-081 Depth: 22-24
 Sample #:
 Soil Type: Lean Clay w/sand and a trace of gravel (CL)

Strain Rate (in/min): 0.060
 Sample Type: 3T
 Dia. (in): 2.90 Ht. (in): 5.96
 Height to Diameter Ratio: 2.1

Unconfined Comp. Strength: 1.75 tsf
Strain at Failure (%): 15.1

W.C. (%): 16.8
 Yd (pcf): 115.9

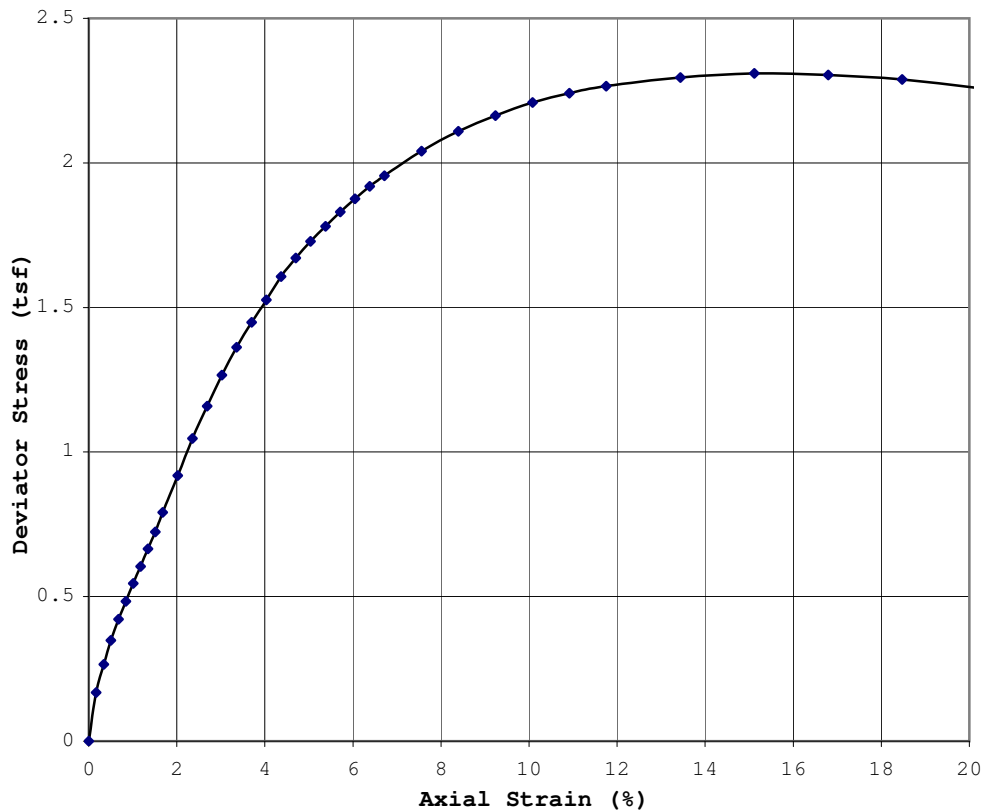
Sketch of Specimen After Failure



Unconfined Stress/Strain Curves

ASTM: D2166

Project: Seneca Wind
 Client: Barr Engineering Company
 Remarks: Job: 11376
 Date: 5/22/18



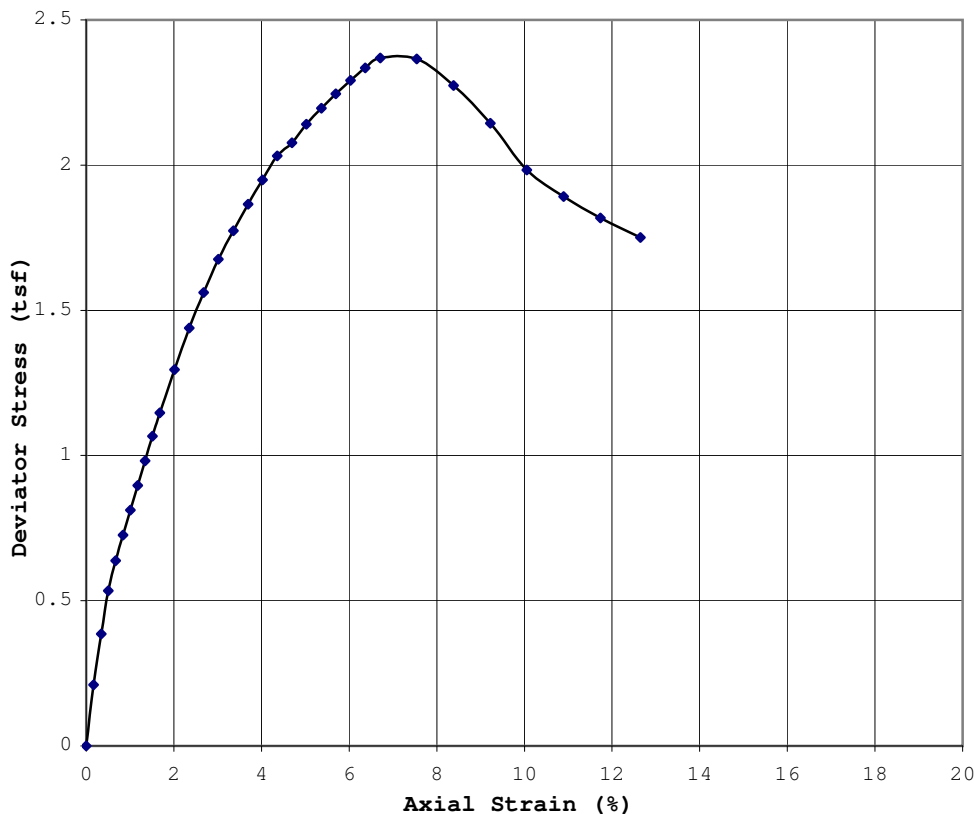
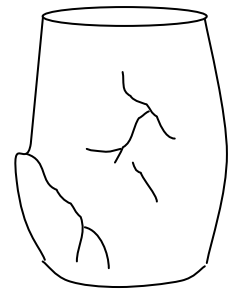
Boring: Geo-083 Depth: 18-20
 Sample #:
 Soil Type: Lean Clay w/sand and a trace of gravel (CL)

Strain Rate (in/min): 0.060
 Sample Type: 3T
 Dia. (in): 2.89 Ht. (in): 5.96
 Height to Diameter Ratio: 2.1

Unconfined Comp. Strength: 2.31 tsf
 Strain at Failure (%): 15.1

W.C. (%): 17.5
 Yd (pcf): 114.0

Sketch of Specimen After Failure



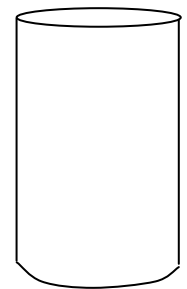
Boring: Geo-087 Depth: 18-20
 Sample #:
 Soil Type: Lean Clay w/sand and a trace of gravel (CL)

Strain Rate (in/min): 0.060
 Sample Type: 3T
 Dia. (in): 2.89 Ht. (in): 5.97
 Height to Diameter Ratio: 2.1

Unconfined Comp. Strength: 2.37 tsf
 Strain at Failure (%): 6.7

W.C. (%): 17.1
 Yd (pcf): 115.0

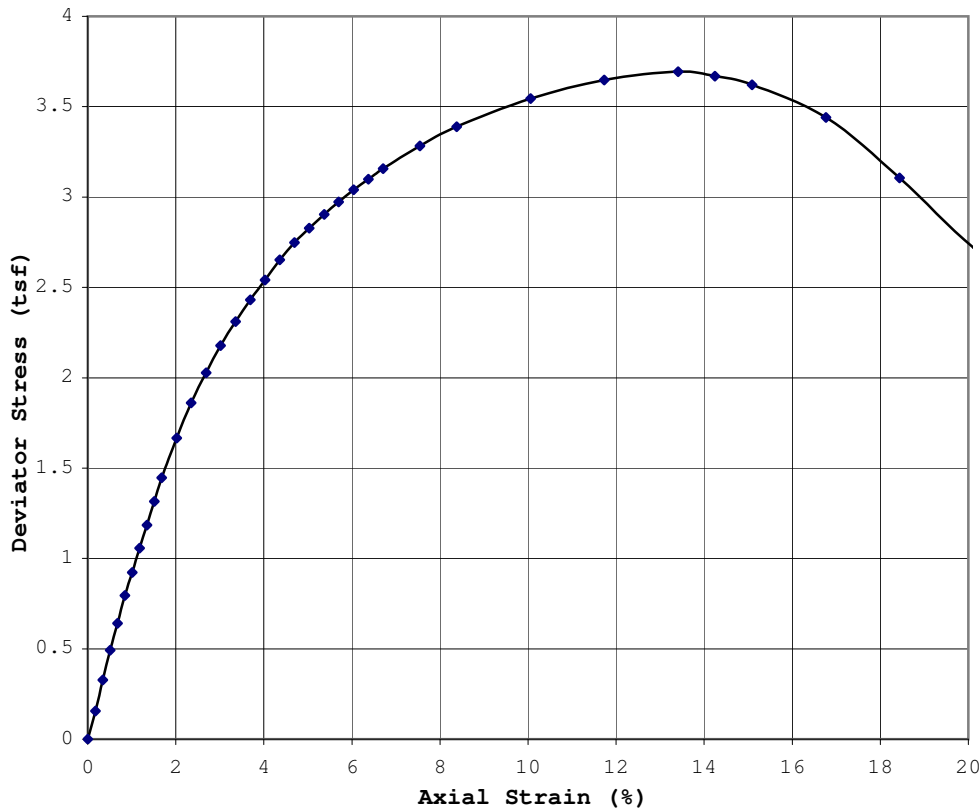
Sketch of Specimen After Failure



Unconfined Stress/Strain Curves

ASTM: D2166

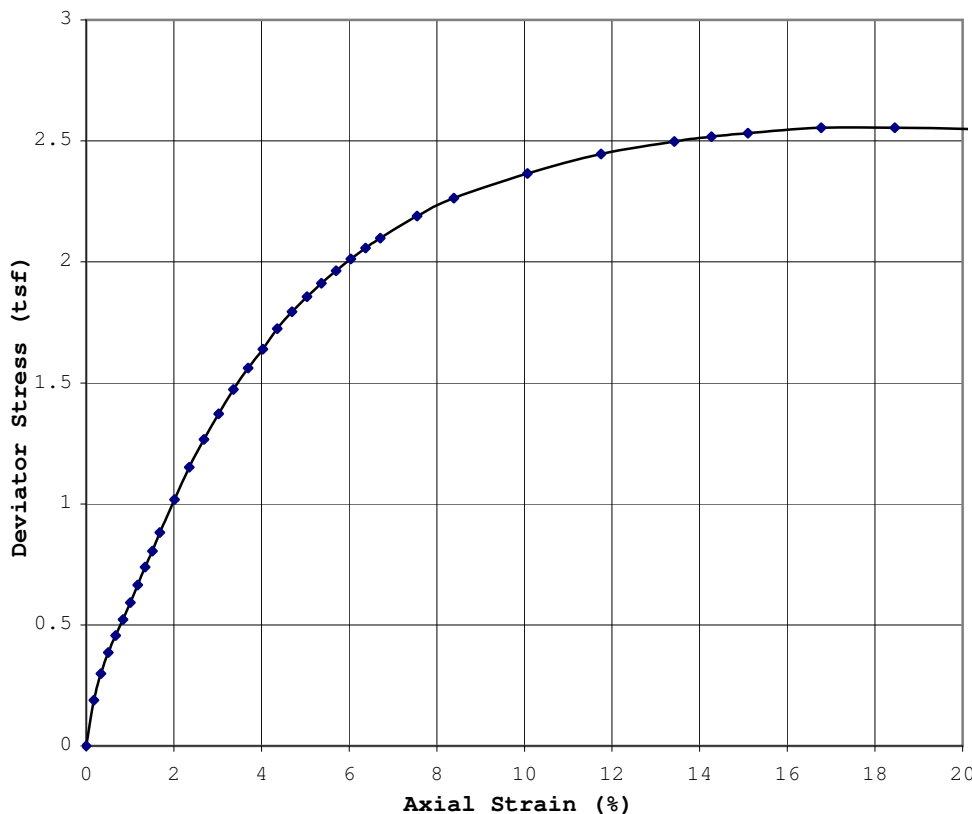
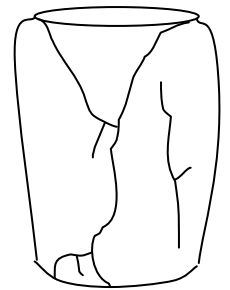
Project: Seneca Wind
 Client: Barr Engineering Company
 Remarks: Job: 11376
 Date: 5/22/18



Boring: Geo-091 Depth: 23-25
 Sample #:
 Soil Type: Lean Clay w/sand and a little gravel (CL)
 Strain Rate (in/min): 0.060
 Sample Type: 3T
 Dia. (in): 2.89 Ht. (in): 5.97
 Height to Diameter Ratio: 2.1
Unconfined Comp. Strength: 3.69 tsf
Strain at Failure (%): 13.4

W.C. (%): 15.4
 Yd (pcf): 117.4

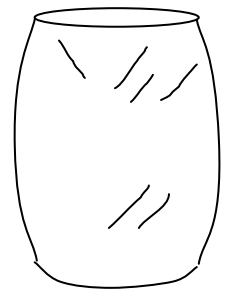
Sketch of Specimen After Failure



Boring: Geo-092 Depth: 22-24
 Sample #:
 Soil Type: Lean Clay w/sand and a little gravel (CL)
 Strain Rate (in/min): 0.060
 Sample Type: 3T
 Dia. (in): 2.89 Ht. (in): 5.96
 Height to Diameter Ratio: 2.1
Unconfined Comp. Strength: 2.55 tsf
Strain at Failure (%): 18.5

W.C. (%): 17.1
 Yd (pcf): 114.6

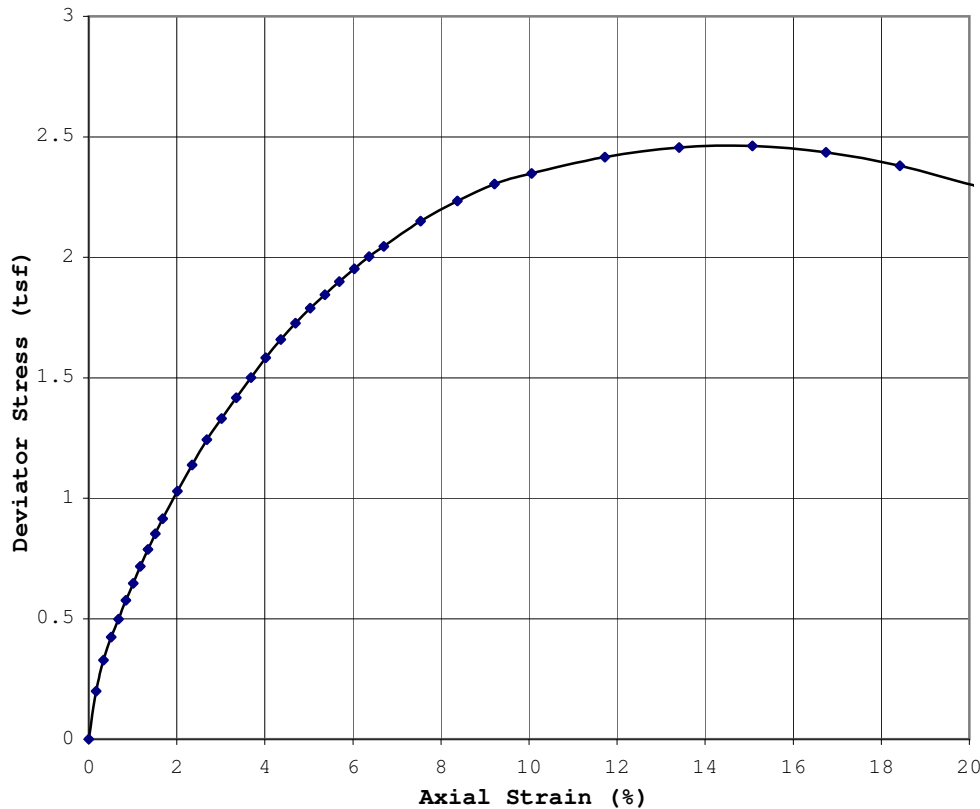
Sketch of Specimen After Failure



Unconfined Stress/Strain Curves

ASTM: D2166

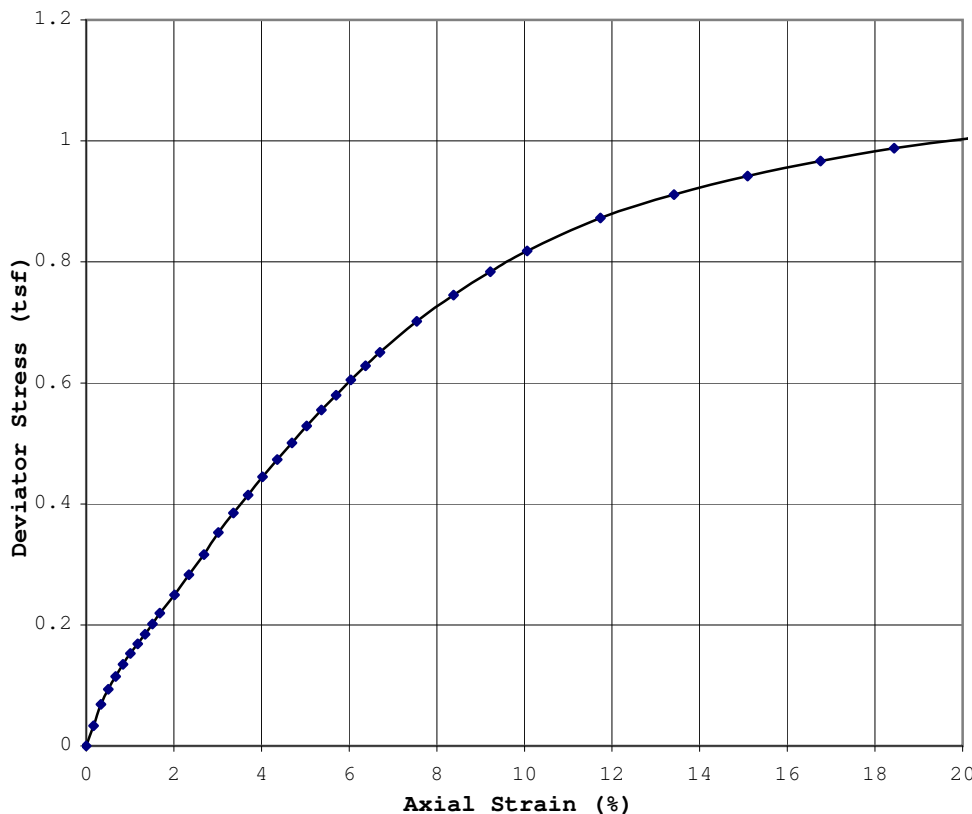
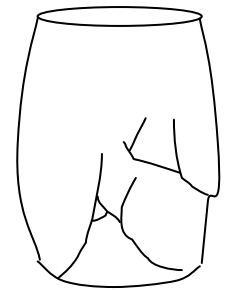
Project: Seneca Wind Job: 11376
 Client: Barr Engineering Company Date: 5/22/18
 Remarks: _____



Boring: Geo-092 Depth: 47-49
 Sample #: _____
 Soil Type: Lean Clay w/sand and a little gravel (CL)
 Strain Rate (in/min): 0.060
 Sample Type: 3T
 Dia. (in): 2.88 Ht. (in): 5.97
 Height to Diameter Ratio: 2.1
 Unconfined Comp. Strength: 2.46 tsf
 Strain at Failure (%): 15.1

W.C. (%): 17.9
 Yd (pcf): 113.1

Sketch of Specimen After Failure



Boring: Geo-093 Depth: 27-29
 Sample #: _____
 Soil Type: Lean Clay w/sand and a trace of gravel (CL)
 Strain Rate (in/min): 0.060
 Sample Type: 3T
 Dia. (in): 2.89 Ht. (in): 5.97
 Height to Diameter Ratio: 2.1
 Unconfined Comp. Strength: 1.00 tsf
 Strain at Failure (%): 20.0

W.C. (%): 18.0
 Yd (pcf): 113.2

Sketch of Specimen After Failure

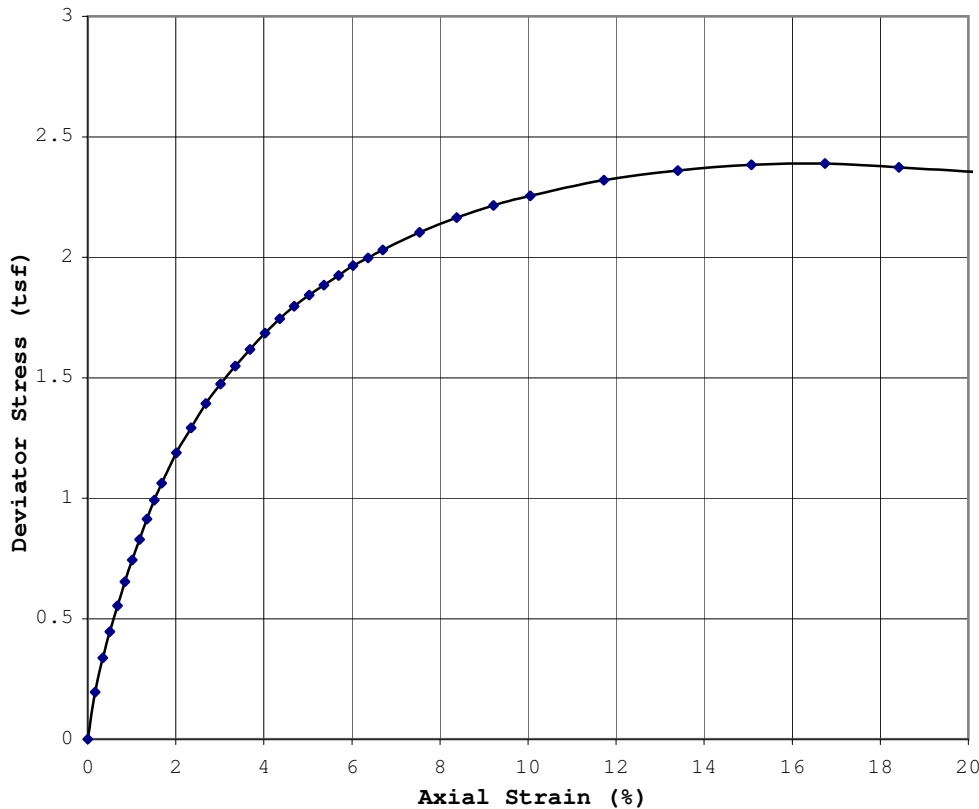


Unconfined Stress/Strain Curves

ASTM: D2166

Project: Seneca Wind
 Client: Barr Engineering Company
 Remarks:

Job: 11376
 Date: 5/22/18



Boring: Geo-009 Depth: 22-24
 Sample #:

Soil Type: Lean Clay w/sand and a trace of gravel (CL)

Strain Rate (in/min): 0.060

Sample Type: 3T

Dia. (in): 2.89 Ht. (in): 5.97

Height to Diameter Ratio: 2.1

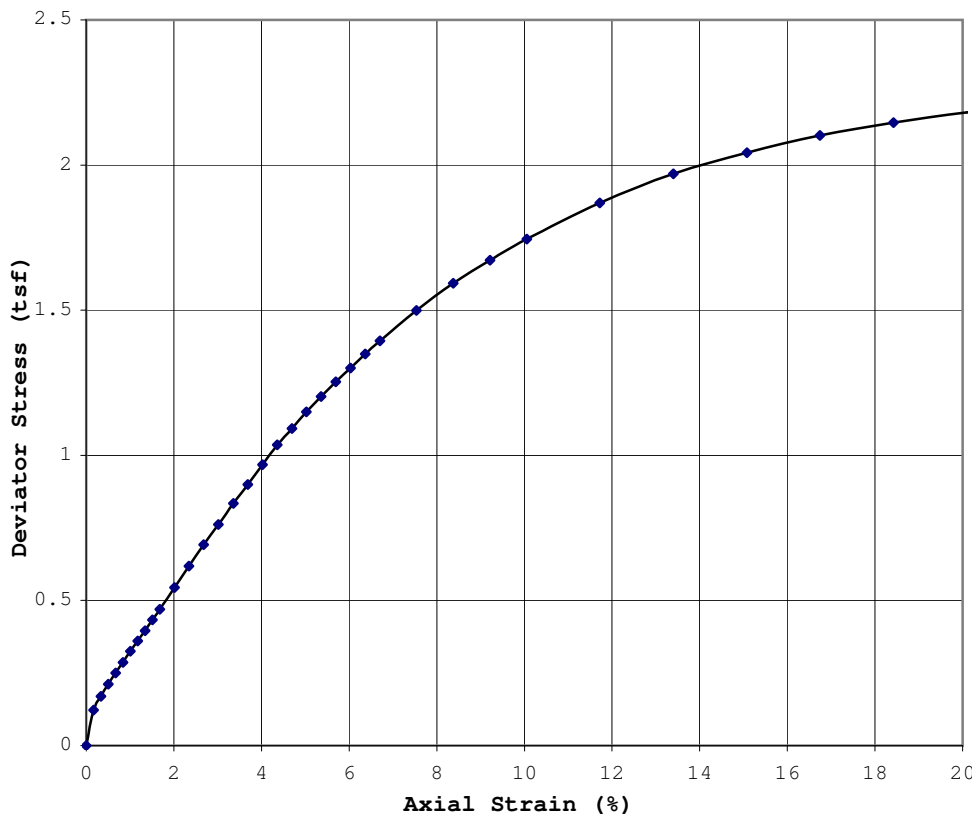
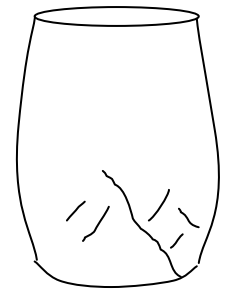
Unconfined Comp. Strength: 2.39 tsf

Strain at Failure (%): 16.7

W.C. (%): 16.8

Yd (pcf): 115.3

Sketch of Specimen After Failure



Boring: Geo-042 Depth: 17-19

Sample #:

Soil Type: Lean Clay w/sand and a trace of gravel (CL)

Strain Rate (in/min): 0.060

Sample Type: 3T

Dia. (in): 2.88 Ht. (in): 5.97

Height to Diameter Ratio: 2.1

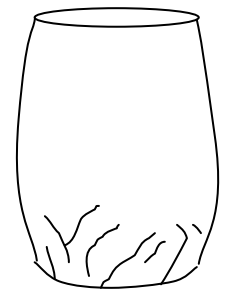
Unconfined Comp. Strength: 2.18 tsf

Strain at Failure (%): 20.0

W.C. (%): 17.1

Yd (pcf): 114.5

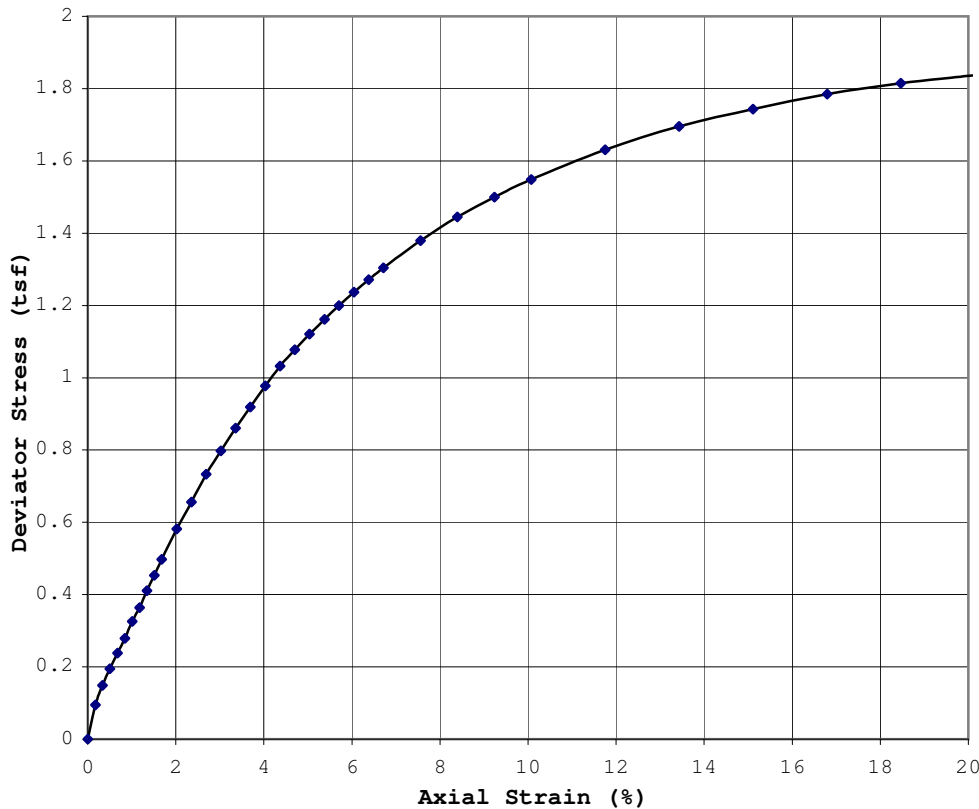
Sketch of Specimen After Failure



Unconfined Stress/Strain Curves

ASTM: D2166

Project: Seneca Wind
 Client: Barr Engineering Company
 Job: 11376
 Date: 5/22/18
 Remarks:



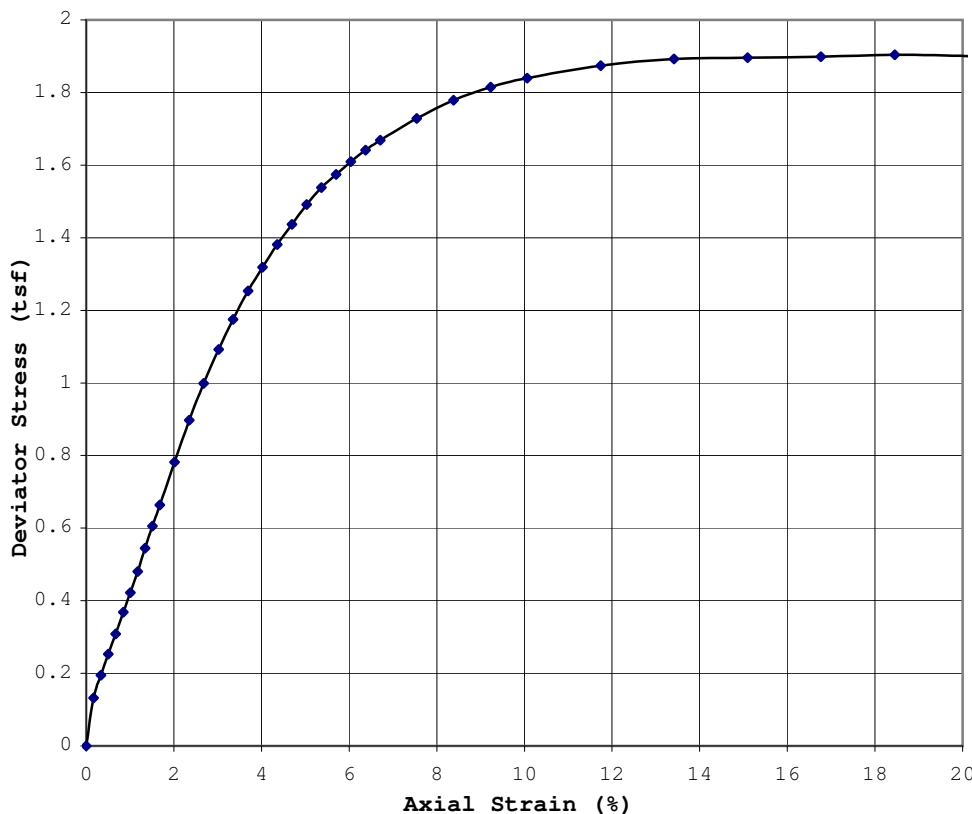
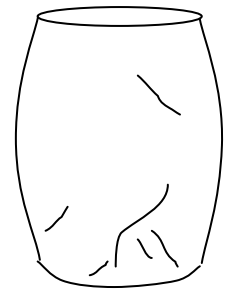
Boring: Geo-042 Depth: 37-39
 Sample #:
 Soil Type: Lean Clay w/sand and a trace of gravel (CL)

Strain Rate (in/min): 0.060
 Sample Type: 3T
 Dia. (in): 2.88 Ht. (in): 5.96
 Height to Diameter Ratio: 2.1

Unconfined Comp. Strength: 1.83 tsf
 Strain at Failure (%): 20.0

W.C. (%): 18.3
 Yd (pcf): 112.3

Sketch of Specimen After Failure



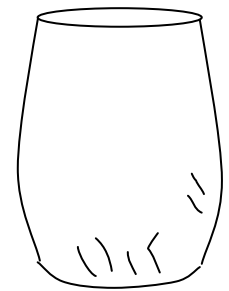
Boring: Geo-076 Depth: 15-17
 Sample #:
 Soil Type: Lean Clay w/sand and a trace of gravel (CL)

Strain Rate (in/min): 0.060
 Sample Type: 3T
 Dia. (in): 2.88 Ht. (in): 5.96
 Height to Diameter Ratio: 2.1

Unconfined Comp. Strength: 1.90 tsf
 Strain at Failure (%): 18.5

W.C. (%): 17.7
 Yd (pcf): 113.7

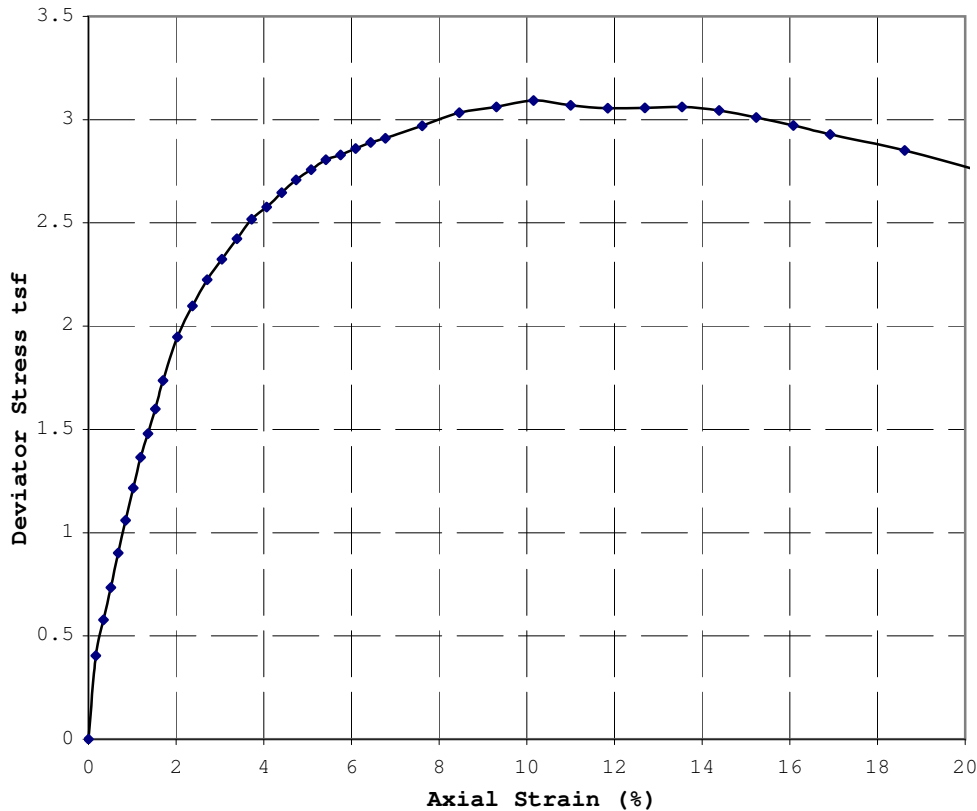
Sketch of Specimen After Failure



Unconsolidated-Undrained Triaxial Compressive Strength

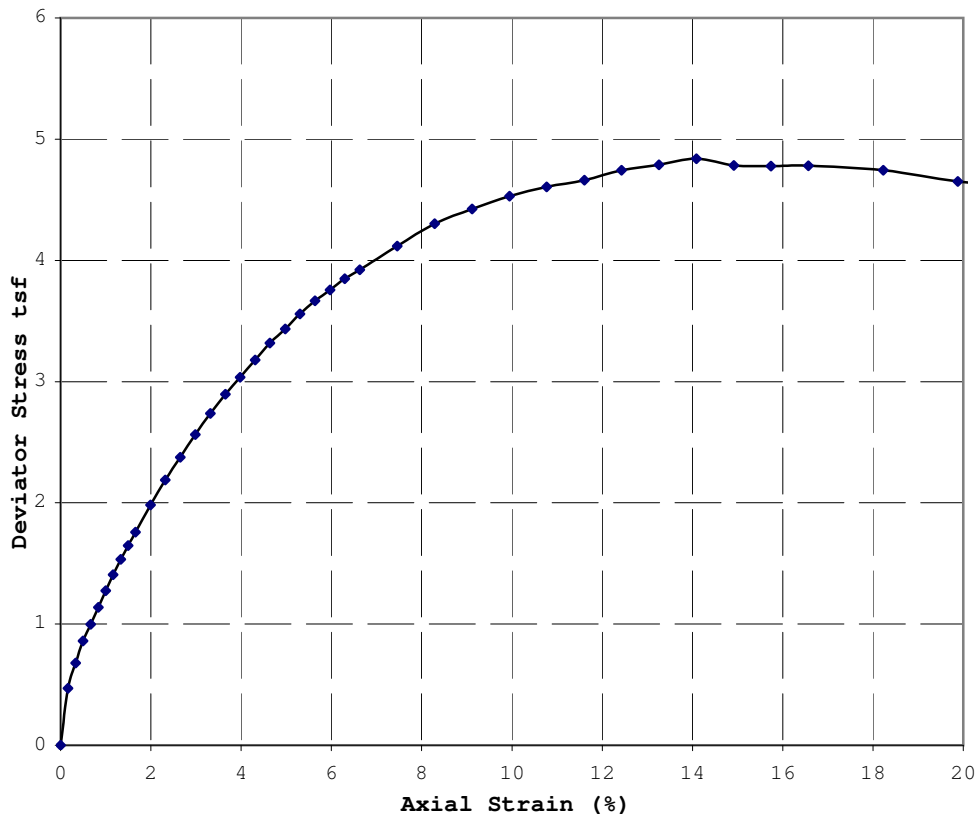
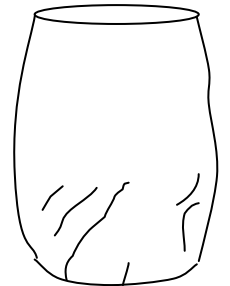
Triaxial U-U Stress/Strain Curves (ASTM:D2850)

Project: Seneca Wind Job: 11376
 Client: Barr Engineering Company Date: 4/23/18
 Remarks: Specimens trimmed to given sizes; Allowed to adjust under applied confining pressures for about 10 minutes.



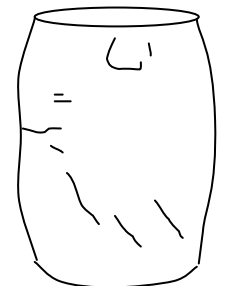
Boring: Geo-001 Depth: 17-19
 Sample #: _____
 Soil Type: Lean Clay w/sand and a little gravel (CL)
 Strain Rate (in/min): 0.060
 Sample Type: 3T
 Dia. (in): 2.89 Ht. (in): 5.91
 Height to Diameter Ratio: 2.04
Max Deviator Stress: 3.09 tsf
Strain at Failure (%): 10.2
 Confining Pressure: 0.95 tsf
 W.C. (%): 17.1
 Yd (pcf): 114.6

Sketch of Specimen After Failure



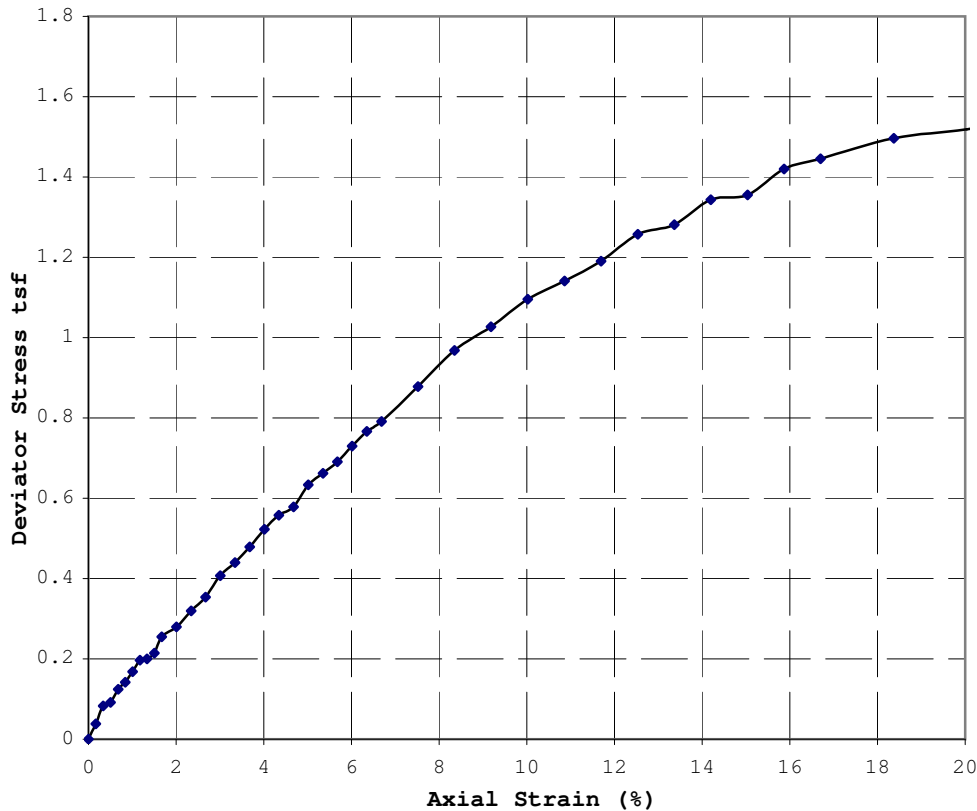
Boring: Geo-001 Depth: 43-44.5
 Sample #: _____
 Soil Type: Lean Clay w/sand and a little gravel (CL)
 Strain Rate (in/min): 0.060
 Sample Type: 3T
 Dia. (in): 2.88 Ht. (in): 6.04
 Height to Diameter Ratio: 2.09
Max Deviator Stress: 4.84 tsf
Strain at Failure (%): 14.1
 Confining Pressure: 2.23 tsf
 W.C. (%): 16.1
 Yd (pcf): 117.5

Sketch of Specimen After Failure



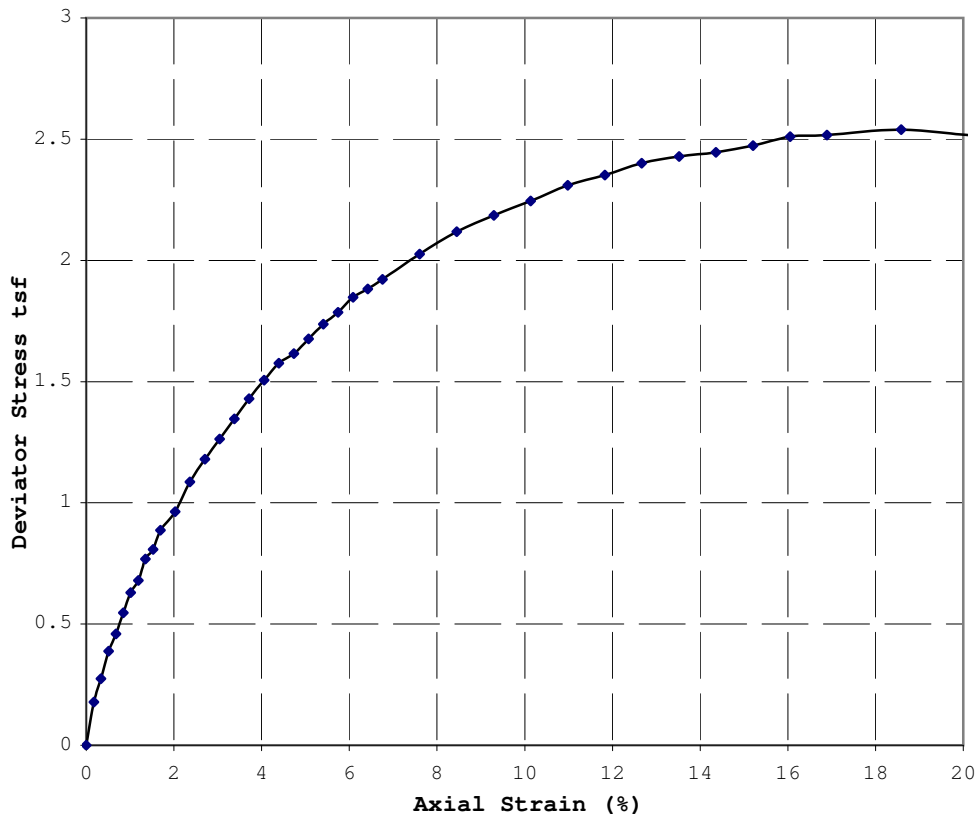
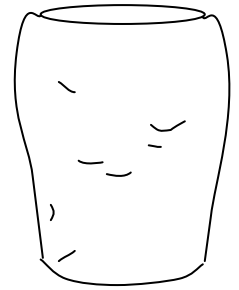
Triaxial U-U Stress/Strain Curves (ASTM:D2850)

Project: Seneca Wind Job: 11376
 Client: Barr Engineering Company Date: 4/24/18
 Remarks: Specimens trimmed to given sizes; Allowed to adjust under applied confining pressures for about 10 minutes.



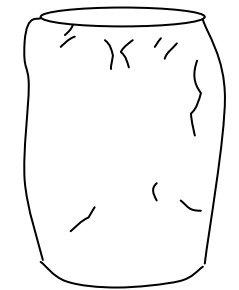
Boring: Geo-003 Depth: 16.5-17.5
 Sample #: _____
 Soil Type: Sandy Lean Clay w/a little gravel (CL)
 Strain Rate (in/min): 0.060
 Sample Type: 3T
 Dia. (in): 2.86 Ht. (in): 5.99
 Height to Diameter Ratio: 2.09
 Max Deviator Stress: 1.52 tsf
 Strain at Failure (%): 20.0
 Confining Pressure: 0.88 tsf
 W.C. (%): 14.7
 Yd (pcf): 120.1

Sketch of Specimen After Failure



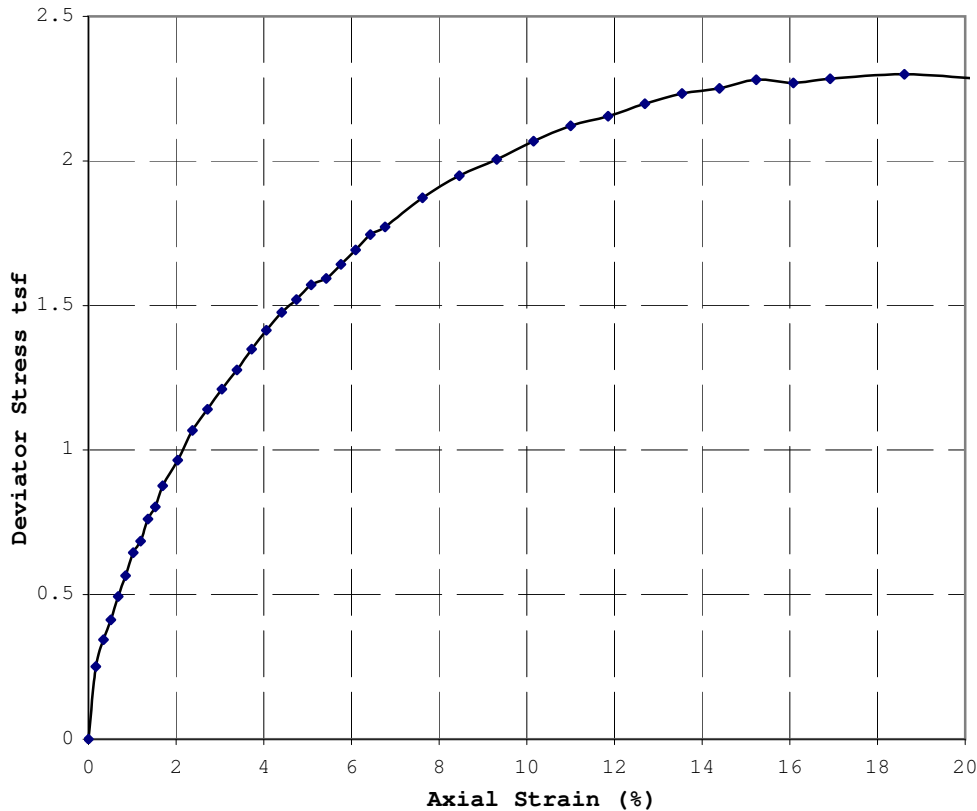
Boring: Geo-019 Depth: 12-14
 Sample #: _____
 Soil Type: Lean Clay w/sand and a little gravel (CL)
 Strain Rate (in/min): 0.060
 Sample Type: 3T
 Dia. (in): 2.89 Ht. (in): 5.92
 Height to Diameter Ratio: 2.05
 Max Deviator Stress: 2.54 tsf
 Strain at Failure (%): 20.0
 Confining Pressure: 0.7 tsf
 W.C. (%): 14.5
 Yd (pcf): 119.8

Sketch of Specimen After Failure



Triaxial U-U Stress/Strain Curves (ASTM:D2850)

Project: Seneca Wind Job: 11376
 Client: Barr Engineering Company Date: 4/24/18
 Remarks: Specimens trimmed to given sizes; Allowed to adjust under applied confining pressures for about 10 minutes.



Boring: Geo-020 Depth: 16.5-18.5
 Sample #:

Soil Type: Sandy Lean Clay w/a little gravel (CL)

Strain Rate (in/min): 0.060

Sample Type: 3T

Dia. (in): 2.89 Ht. (in): 5.91

Height to Diameter Ratio: 2.05

Max Deviator Stress: 2.30 tsf

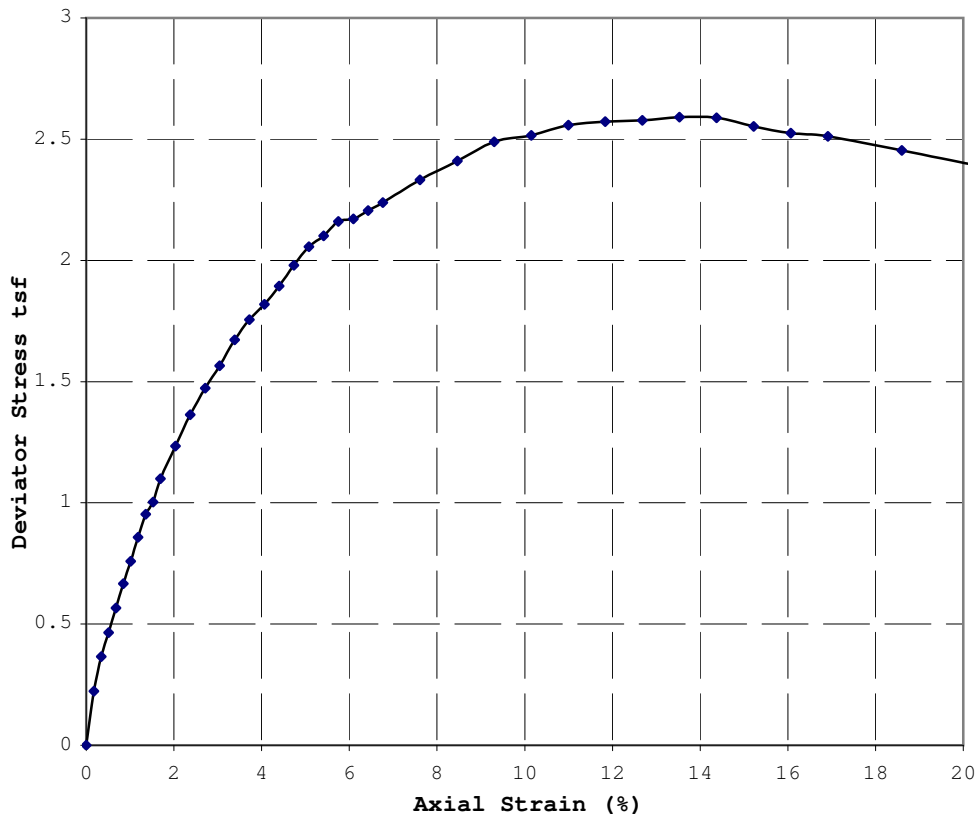
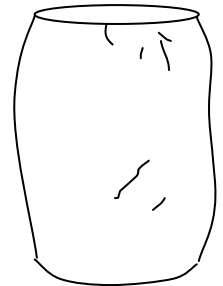
Strain at Failure (%): 18.6

Confining Pressure: 0.93 tsf

W.C. (%): 14.6

Yd (pcf): 120.5

Sketch of Specimen After Failure



Boring: Geo-022 Depth: 7.5-9.5

Sample #:

Soil Type: Sandy Lean Clay w/a little gravel (CL)

Strain Rate (in/min): 0.060

Sample Type: 3T

Dia. (in): 2.88 Ht. (in): 5.91

Height to Diameter Ratio: 2.05

Max Deviator Stress: 2.59 tsf

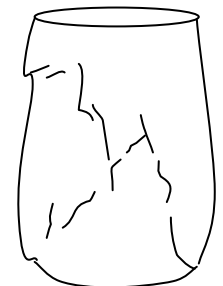
Strain at Failure (%): 13.5

Confining Pressure: 0.48 tsf

W.C. (%): 18.5

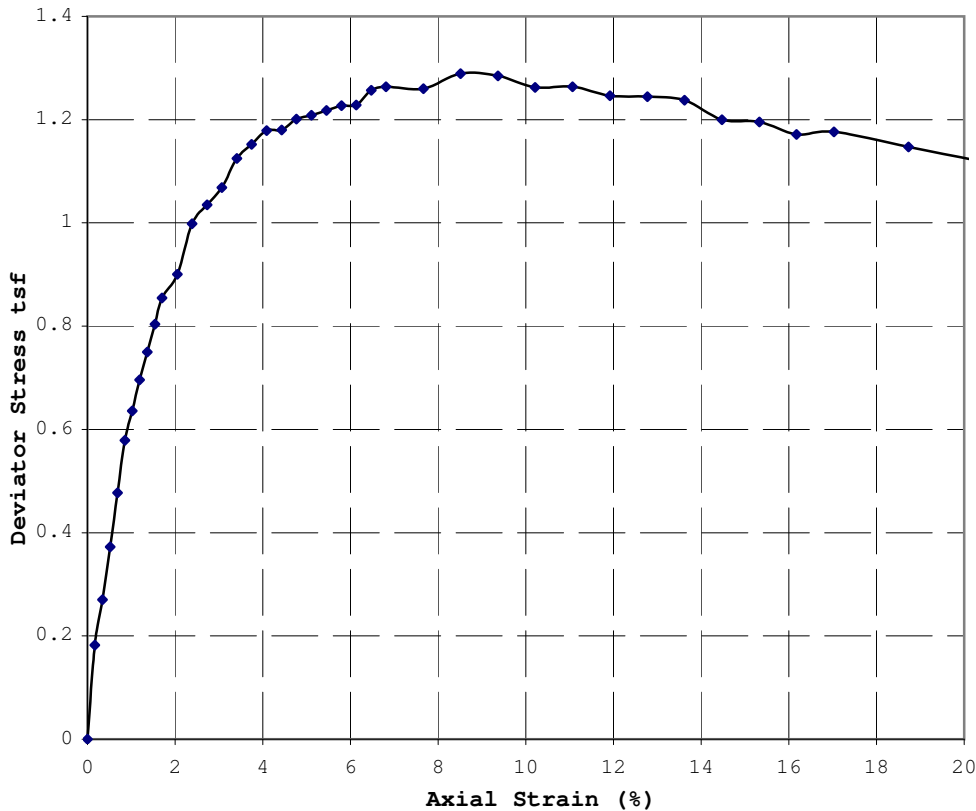
Yd (pcf): 111.0

Sketch of Specimen After Failure



Triaxial U-U Stress/Strain Curves (ASTM:D2850)

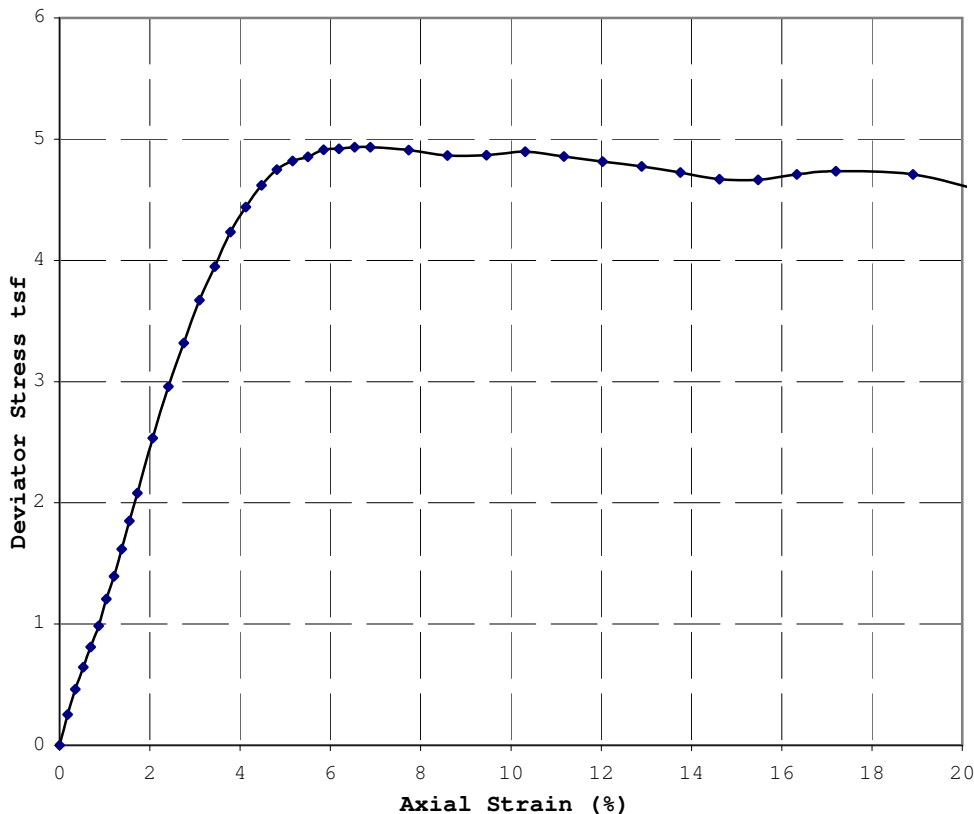
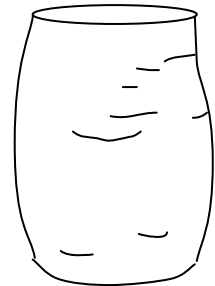
Project: Seneca Wind Job: 11376
 Client: Barr Engineering Company Date: 4/24/18
 Remarks: Specimens trimmed to given sizes; Allowed to adjust under applied confining pressures for about 10 minutes.



Boring: Geo-047 Depth: 10-12
 Sample #:
 Soil Type: Silty Sand w/layers of Lean Clay (SM)
 Strain Rate (in/min): 0.060
 Sample Type: 3T
 Dia. (in): 2.88 Ht. (in): 5.87
 Height to Diameter Ratio: 2.04
Max Deviator Stress: 1.29 tsf
Strain at Failure (%): 8.5
 Confining Pressure: 0.6 tsf

W.C. (%) 22.3
 Yd (pcf) 93.3

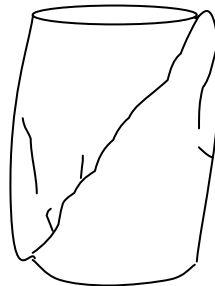
Sketch of Specimen After Failure



Boring: Geo-053 Depth: 7.5-8.5
 Sample #:
 Soil Type: Lean Clay w/sand and a little gravel (CL)
 Strain Rate (in/min): 0.060
 Sample Type: 3T
 Dia. (in): 2.89 Ht. (in): 5.82
 Height to Diameter Ratio: 2.02
Max Deviator Stress: 4.93 tsf
Strain at Failure (%): 6.9
 Confining Pressure: 0.43 tsf

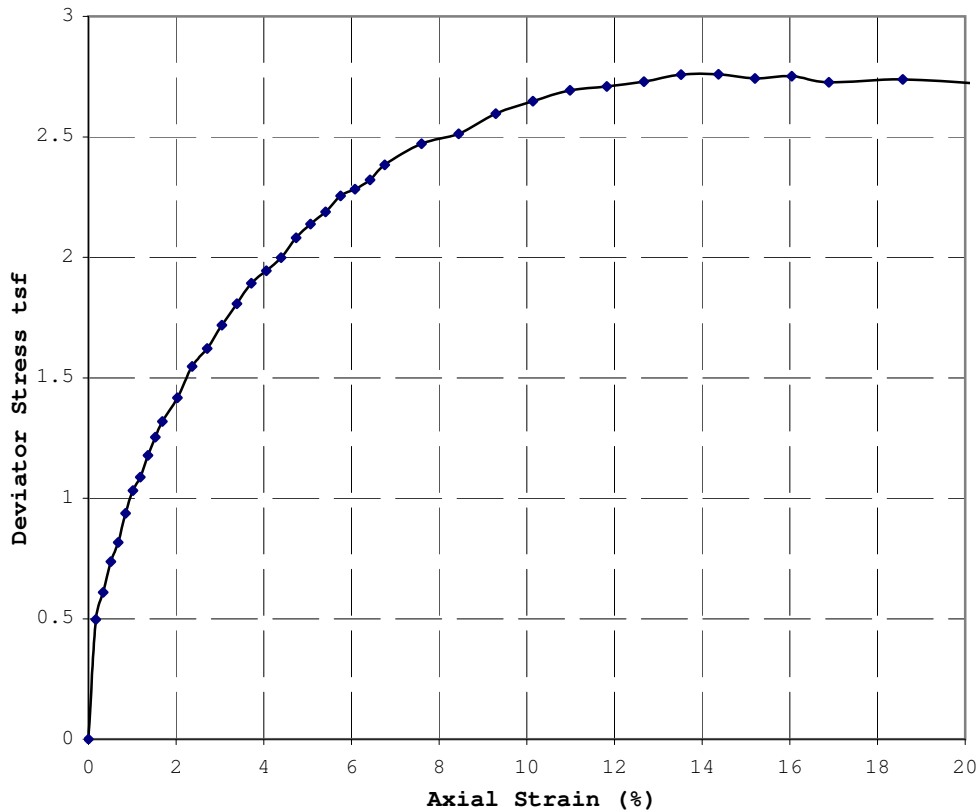
W.C. (%) 17.0
 Yd (pcf) 114.6

Sketch of Specimen After Failure



Triaxial U-U Stress/Strain Curves (ASTM:D2850)

Project: Seneca Wind Job: 11376
 Client: Barr Engineering Company Date: 5/22/18
 Remarks: Specimens trimmed to given sizes; Allowed to adjust under applied confining pressures for about 10 minutes.

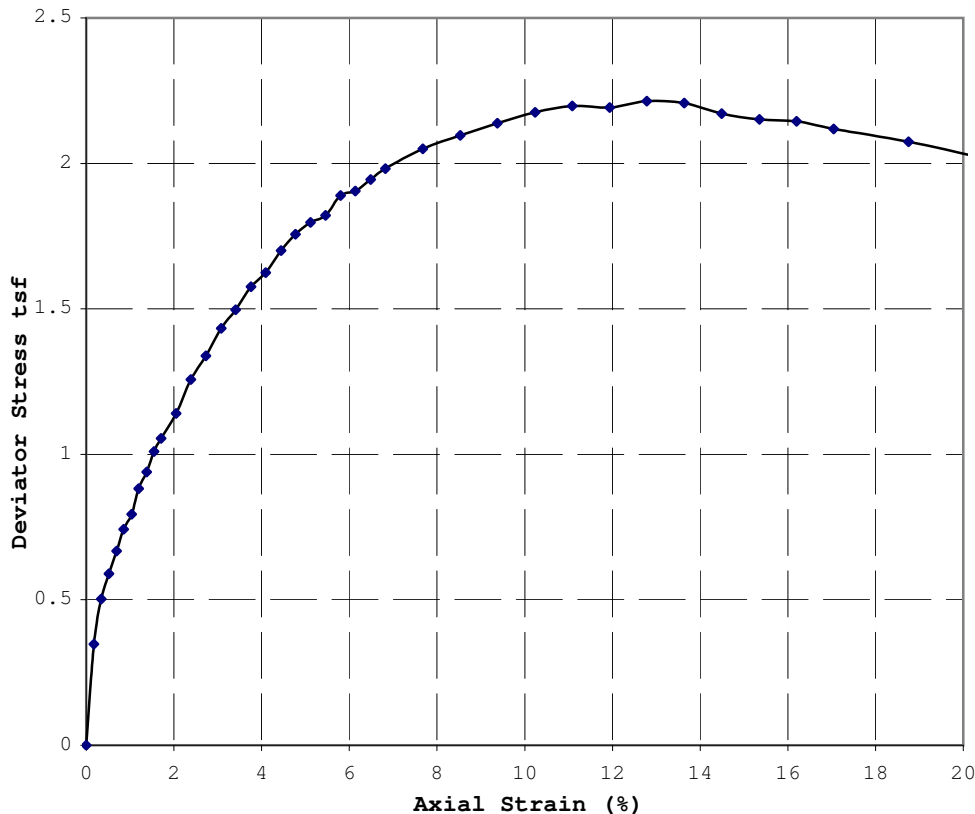


Boring: Geo-007 Depth: 27-29
 Sample #:
 Soil Type: Lean Clay w/sand and a little gravel (CL)

Strain Rate (in/min): 0.060
 Sample Type: 3T
 Dia. (in): 2.89 Ht. (in): 5.92
 Height to Diameter Ratio: 2.05

Max Deviator Stress: 2.76 tsf
 Strain at Failure (%): 14.4
 Confining Pressure: 1.5 tsf
 W.C. (%): 12.8
 Yd (pcf): 124.0

Sketch of Specimen After Failure



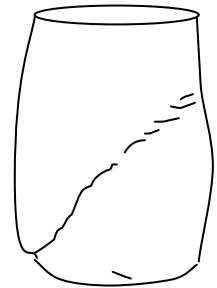
Boring: Geo-038 Depth: 14.5-16.5
 Sample #:
 Soil Type: Lean Clay w/sand and a trace of gravel (CL)

Strain Rate (in/min): 0.060
 Sample Type: 3T

Dia. (in): 2.88 Ht. (in): 5.87
 Height to Diameter Ratio: 2.04

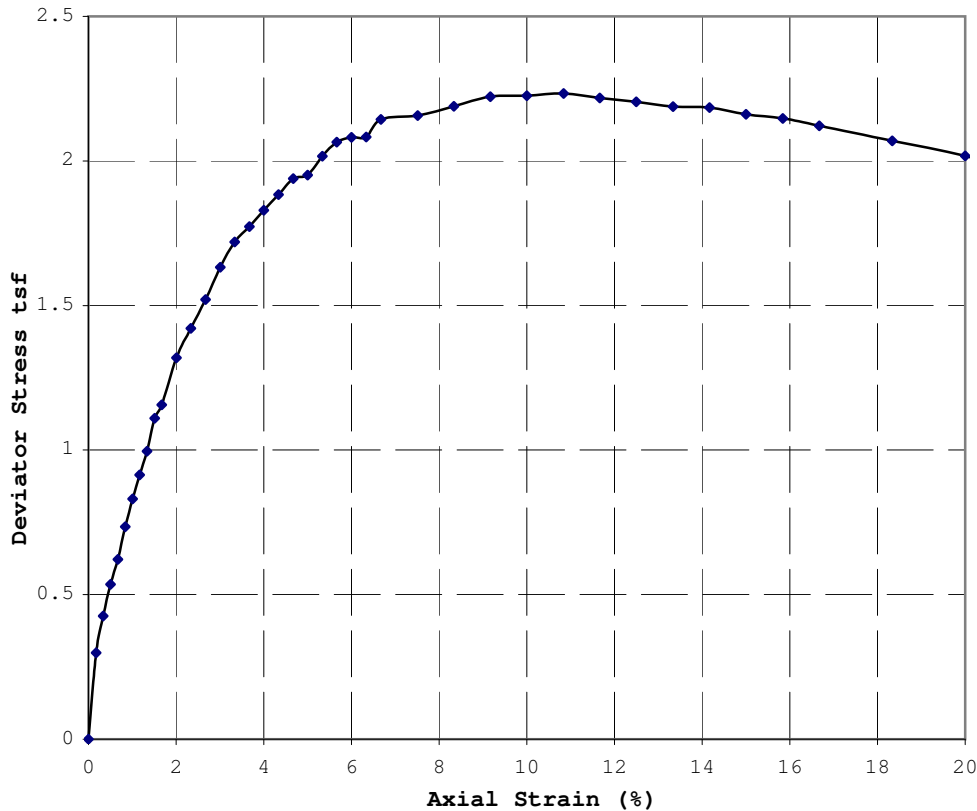
Max Deviator Stress: 2.21 tsf
 Strain at Failure (%): 12.8
 Confining Pressure: 0.75 tsf
 W.C. (%): 17.0
 Yd (pcf): 115.5

Sketch of Specimen After Failure



Triaxial U-U Stress/Strain Curves (ASTM:D2850)

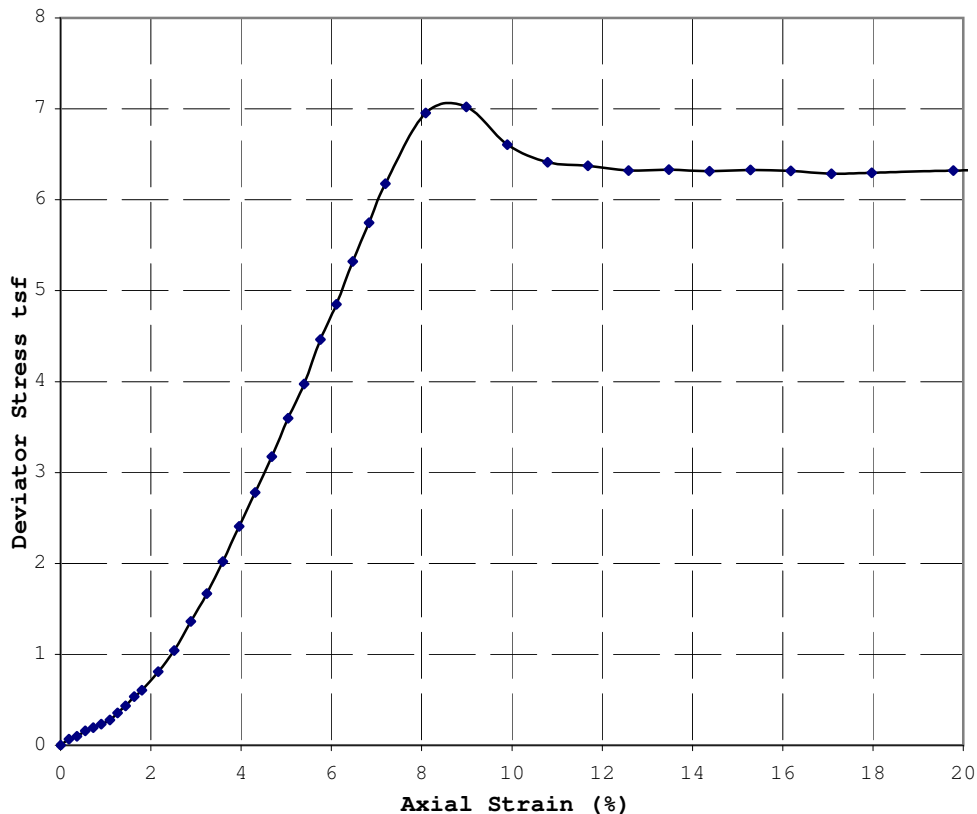
Project: Seneca Wind Job: 11376
 Client: Barr Engineering Company Date: 5/22/18
 Remarks: Specimens trimmed to given sizes; Allowed to adjust under applied confining pressures for about 10 minutes.



Boring: Geo-070 Depth: 25-27
 Sample #:
 Soil Type: Lean Clay w/sand and a little gravel (CL)
 Strain Rate (in/min): 0.060
 Sample Type: 3T
 Dia. (in): 2.89 Ht. (in): 6.00
 Height to Diameter Ratio: 2.08
Max Deviator Stress: 2.23 tsf
Strain at Failure (%): 10.8
 Confining Pressure: 1.25 tsf

W.C. (%) 17.0
 Yd (pcf): 115.2

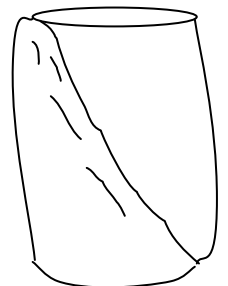
Sketch of Specimen After Failure



Boring: Geo-072 Depth: 28-30
 Sample #:
 Soil Type: Silty Clay (CL-ML/ML)
 Strain Rate (in/min): 0.060
 Sample Type: 3T
 Dia. (in): 2.91 Ht. (in): 5.56
 Height to Diameter Ratio: 1.91
Max Deviator Stress: 7.02 tsf
Strain at Failure (%): 9.0
 Confining Pressure: 1.5 tsf

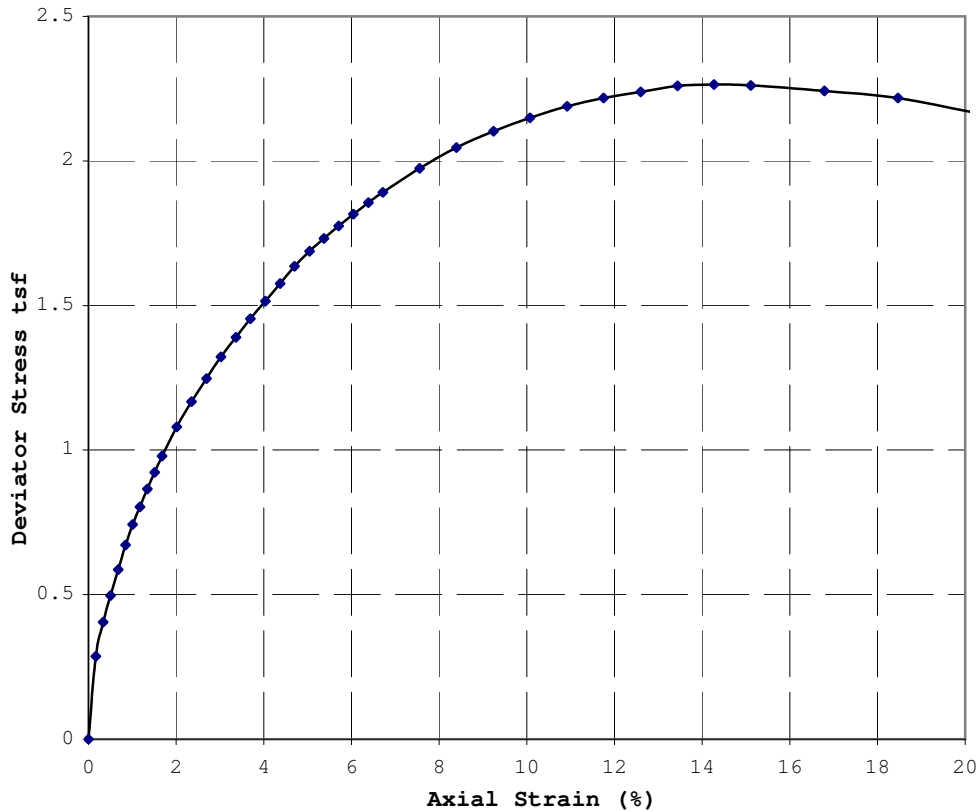
W.C. (%) 22.9
 Yd (pcf): 103.4

Sketch of Specimen After Failure



Triaxial U-U Stress/Strain Curves (ASTM:D2850)

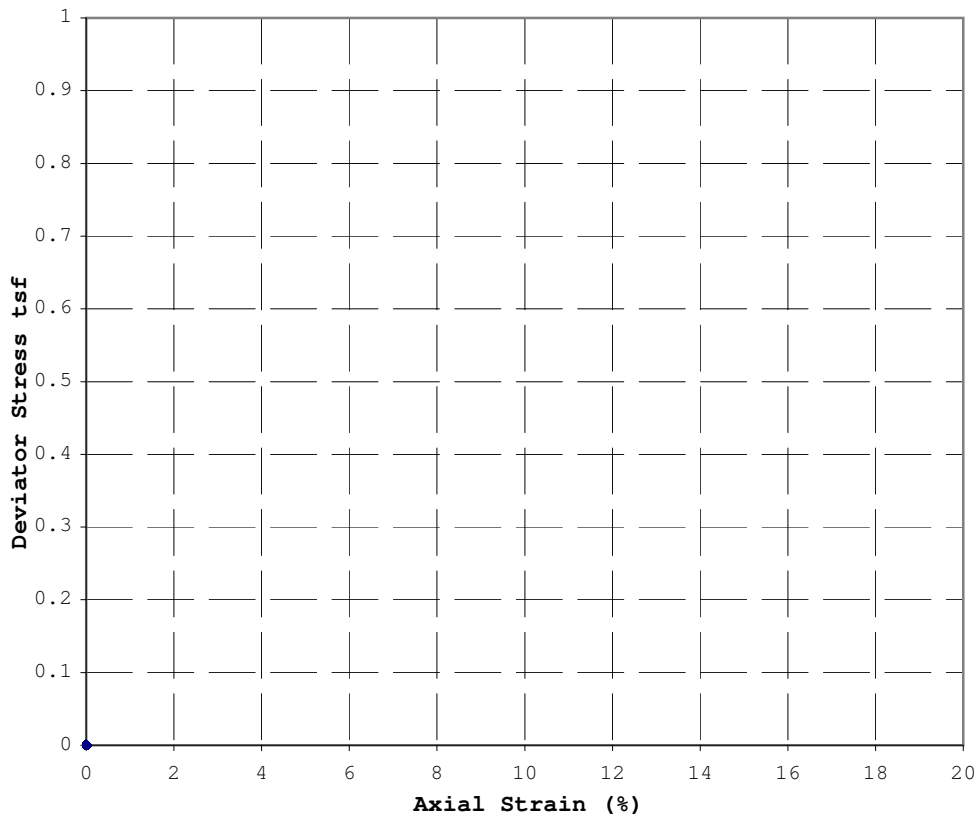
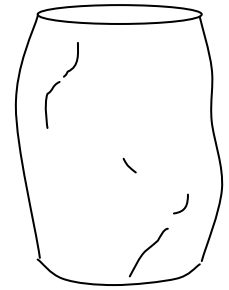
Project: Seneca Wind Job: 11376
 Client: Barr Engineering Company Date: 5/22/18
 Remarks: Specimens trimmed to given sizes; Allowed to adjust under applied confining pressures for about 10 minutes.



Boring: Geo-052 Depth: 22-24
 Sample #: _____
 Soil Type: Lean Clay w/sand and a little gravel (CL)
 Strain Rate (in/min): 0.060
 Sample Type: 3T
 Dia. (in) 2.90 Ht. (in) 5.96
 Height to Diameter Ratio: 2.06
 Max Deviator Stress: 2.26 tsf
 Strain at Failure (%): 14.3
 Confining Pressure: 1.25 tsf

W.C. (%) 17.3
 Yd (pcf) 114.0

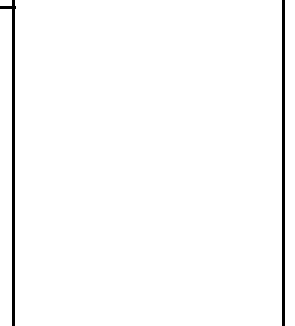
Sketch of Specimen After Failure



Boring: _____ Depth: _____
 Sample #: _____
 Soil Type: _____
 Strain Rate (in/min): _____
 Sample Type: _____
 Dia. (in): _____ Ht. (in): _____
 Height to Diameter Ratio: _____
 Max Deviator Stress: _____ tsf
 Strain at Failure (%): _____
 Confining Pressure: _____ tsf

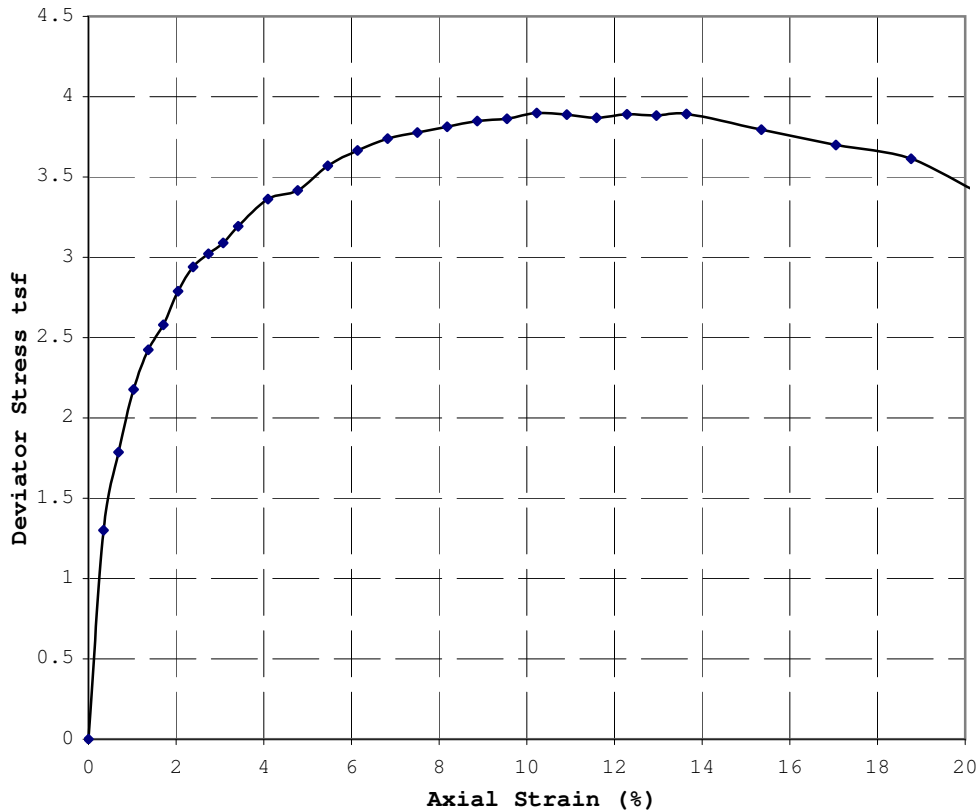
W.C. (%): _____
 Yd (pcf): _____

Sketch of Specimen After Failure



Triaxial U-U Stress/Strain Curves (ASTM:D2850)

Project: Seneca Wind Job: 11376
 Client: Barr Engineering Company Date: 4/30/18
 Remarks: Specimens trimmed to given sizes; Allowed to adjust under applied confining pressures for about 10 minutes.

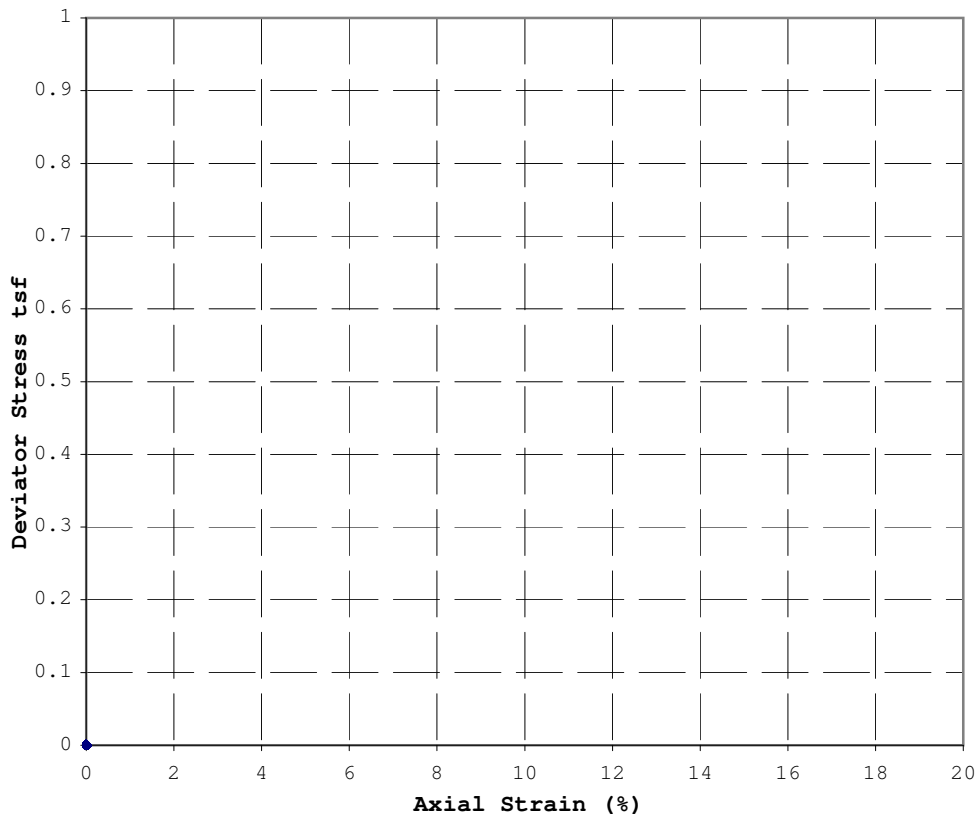
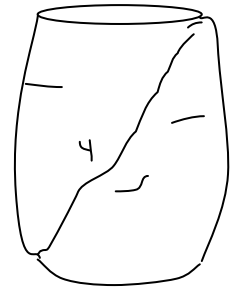


Boring: Geo-084 Depth: 15-15.5
 Sample #:
 Soil Type: Lean Clay w/sand and a trace of gravel (CL)

Strain Rate (in/min): 0.030
 Sample Type: 3T
 Dia. (in): 1.44 Ht. (in): 2.93
 Height to Diameter Ratio: 2.04

Max Deviator Stress: 3.90 tsf
 Strain at Failure (%): 10.2
 Confining Pressure: 0.78 tsf
 W.C. (%): 18.1
 Yd (pcf): 111.3

Sketch of Specimen After Failure



Boring: Depth:
 Sample #:
 Soil Type:

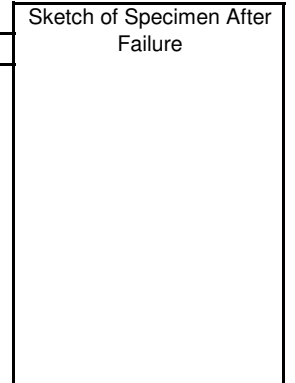
Strain Rate (in/min):
 Sample Type:

Dia. (in): Ht. (in):
 Height to Diameter Ratio:

Max Deviator Stress: tsf
 Strain at Failure (%):
 Confining Pressure: tsf

W.C. (%):
 Yd (pcf):

Sketch of Specimen After Failure



Direct Shear

Direct Shear Test

ASTM: D3080

Job No.: 11376

Project/Client: Seneca Wind / Barr Engineering Company

Boring No.: Geo-030

Sample: Composite

Soil Type: Silty Sand with Gravel (SW-SM)

Depth (ft): 40-41.5,45-46.5

Sample Type: Bags

Test Date: 5/8/2018

Date Reported: 5/18/2018

Shear Rate

0.003 (in/min)

Remarks: Specimens reconstituted given density using #4 material; Inundated after applying normal load. Consolidated and sheared to given displacements at constant rate of 0.003inches/minute.

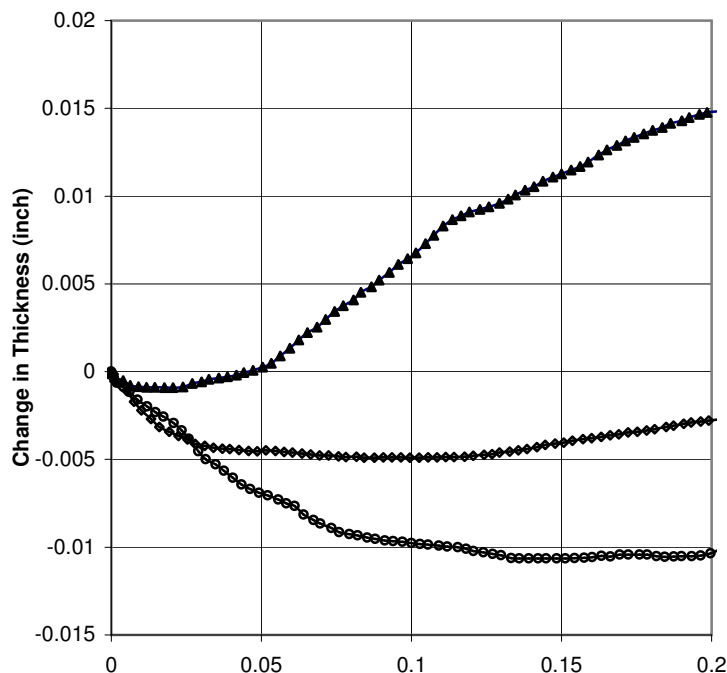
Liquid Limit:

Plastic Limit:

Plasticity Index:

(*) = Assumed Specific Gravity

Specific Gravity (*): 2.67



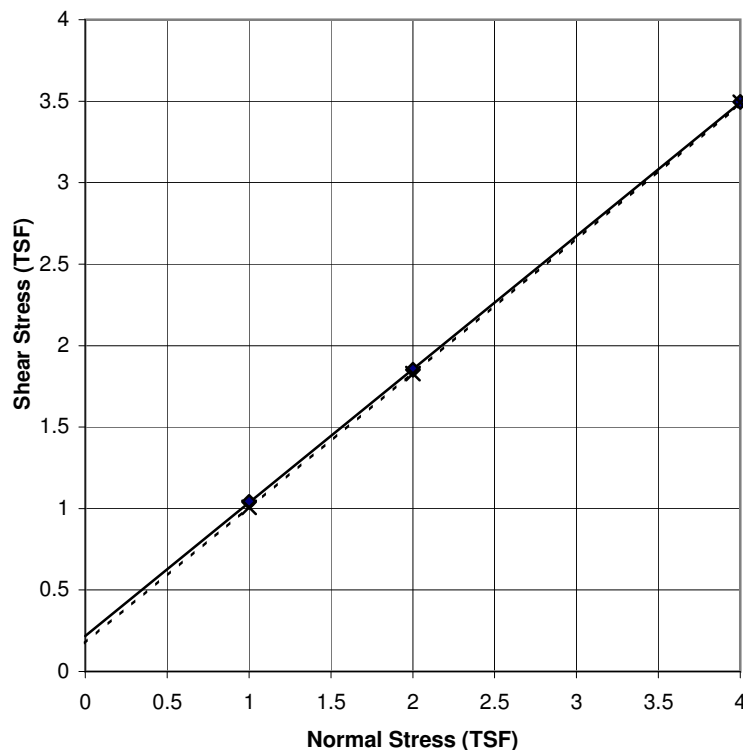
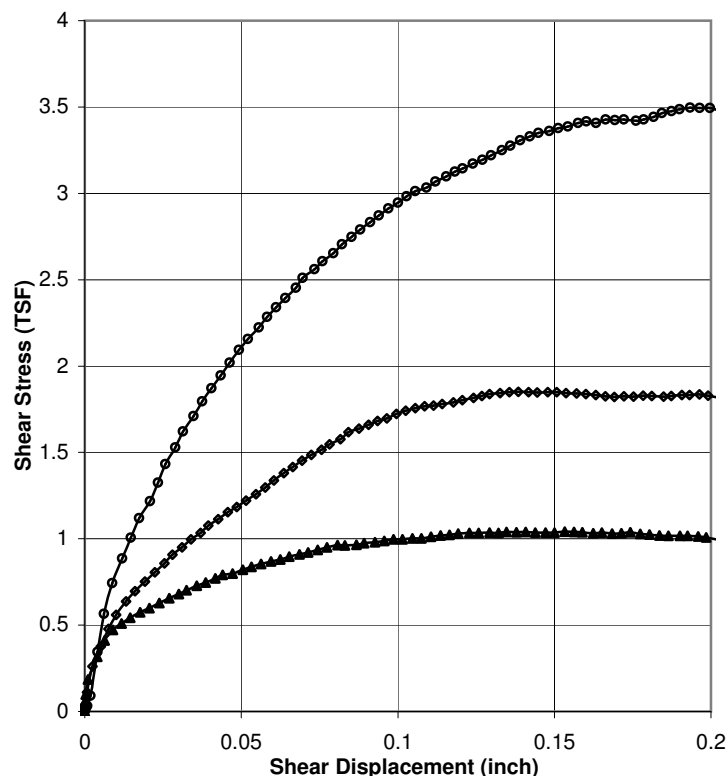
Failure Criterion:

Max Stress	A	B	C	D
Initial	▲	◇	○	X
Diameter (In.)	2.50	2.50	2.50	
Thickness (In.)	0.97	0.97	0.97	
Water Content (%)	12.0	12.0	12.0	
Dry Density (pcf)	114.9	114.9	114.9	
Before Shear				
Thickness (In.)	0.95	0.93	0.92	
Water Content (%)	15.9	15.2	14.6	
Dry Density (pcf)	116.9	118.6	119.9	

Normal Stress	1.00	2.00	4.00	
Shear Stress (Peak)	1.04	1.85	3.50	

"These tests are for informational purposes only and must be reviewed by a qualified professional engineer to verify that the test parameters shown are appropriate for any particular design."

◆ Peak Conditions	x At Given Shear Disp. Of: 0.2
Friction Angle: $\phi = 39.3 \text{ deg.}$	Friction Angle: $\phi = 39.6 \text{ deg.}$
Apparent Cohesion: 0.218 TSF	Apparent Cohesion: 0.174 TSF



Uniaxial Compressive Shear Strength of Rock

Unconfined Compressive Strength of Intact Rock (ASTM:D7012) Method C

Job: 11376

Date: 05/18/18

Project: Seneca Wind
Client: Barr Engineering Company

Sample Identification

Boring: Geo-025 Location:
Sample: Depth: 47.5-48

Laboratory Analysis

Visual Classification: Core

Specimen Dimensions

Ht (in): 5.01

Dia (in): 1.99

Area (in2): 3.11

Moisture Content % 3.2%

Wet Density (PCF) 159.3

Dry Density (PCF) 154.3

Ht to Dia. Ratio: 2.5 : 1

Peak Strength

867.5

TSF

12048

PSI

Remarks:: Specimen cut to given dimension without the use of water.

Before Test



After Test



Unconfined Compressive Strength of Intact Rock (ASTM:D7012) Method C

Job: 11376

Date: 05/18/18

Project: Seneca Wind
Client: Barr Engineering Company

Sample Identification

Boring: Geo-025 Location:
Sample: Depth: 58-58.5

Laboratory Analysis

Visual Classification: Core

Specimen Dimensions

Ht (in): 4.54

Dia (in): 1.99

Area (in2): 3.12

Moisture Content % 1.9%

Wet Density (PCF) 165.3

Dry Density (PCF) 162.3

Ht to Dia. Ratio: 2.3 : 1

Peak Strength

874.1

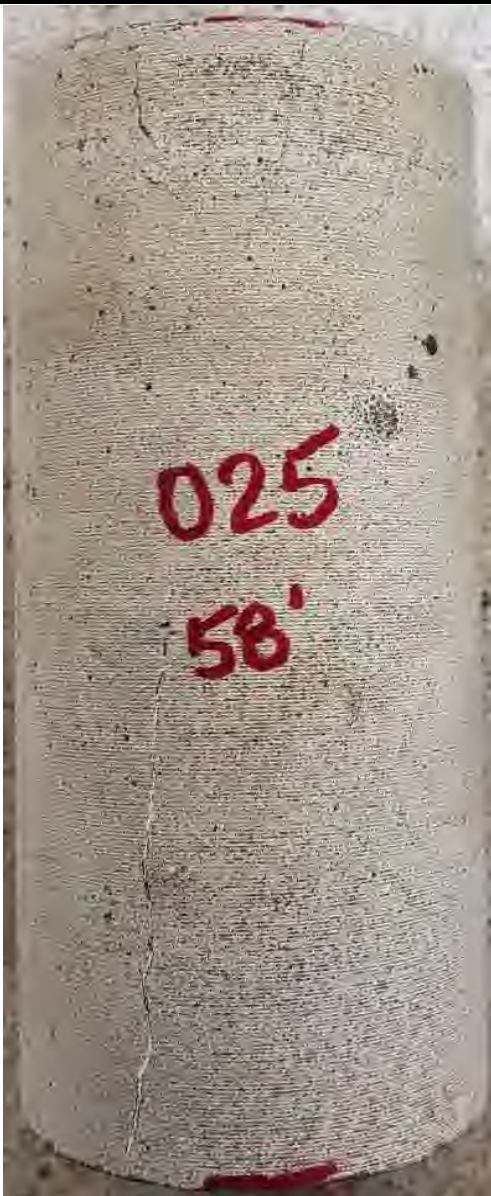
TSF

12140

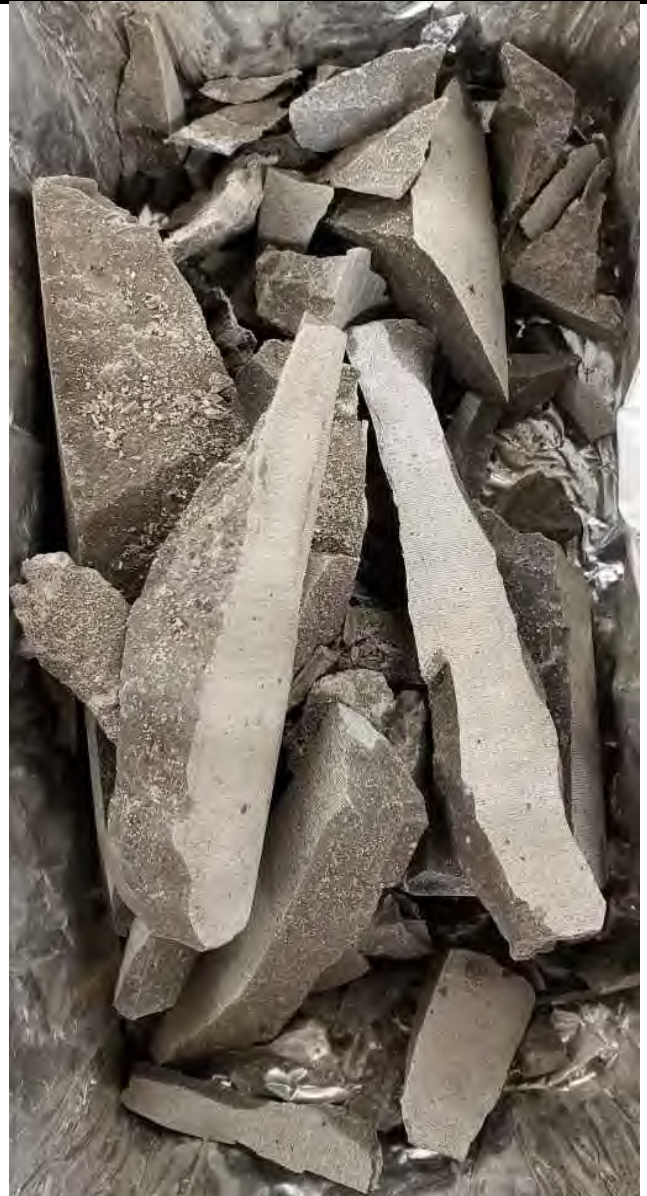
PSI

Remarks:: Specimen cut to given dimension without the use of water.

Before Test



After Test



Unconfined Compressive Strength of Intact Rock (ASTM:D7012) Method C

Job: 11376

Date: 05/18/18

Project: Seneca Wind

Client: Barr Engineering Company

Sample Identification

Boring: Geo-031

Location:

Sample:

Depth:

28

Laboratory Analysis

Visual Classification:

Core

Specimen Dimensions

Ht (in): 4.61

Dia (in): 2.00

Area (in²): 3.13

Peak Strength

1280.9

TSF

17790

PSI

Moisture Content % 2.5%

Wet Density (PCF) 164.6

Dry Density (PCF) 160.6

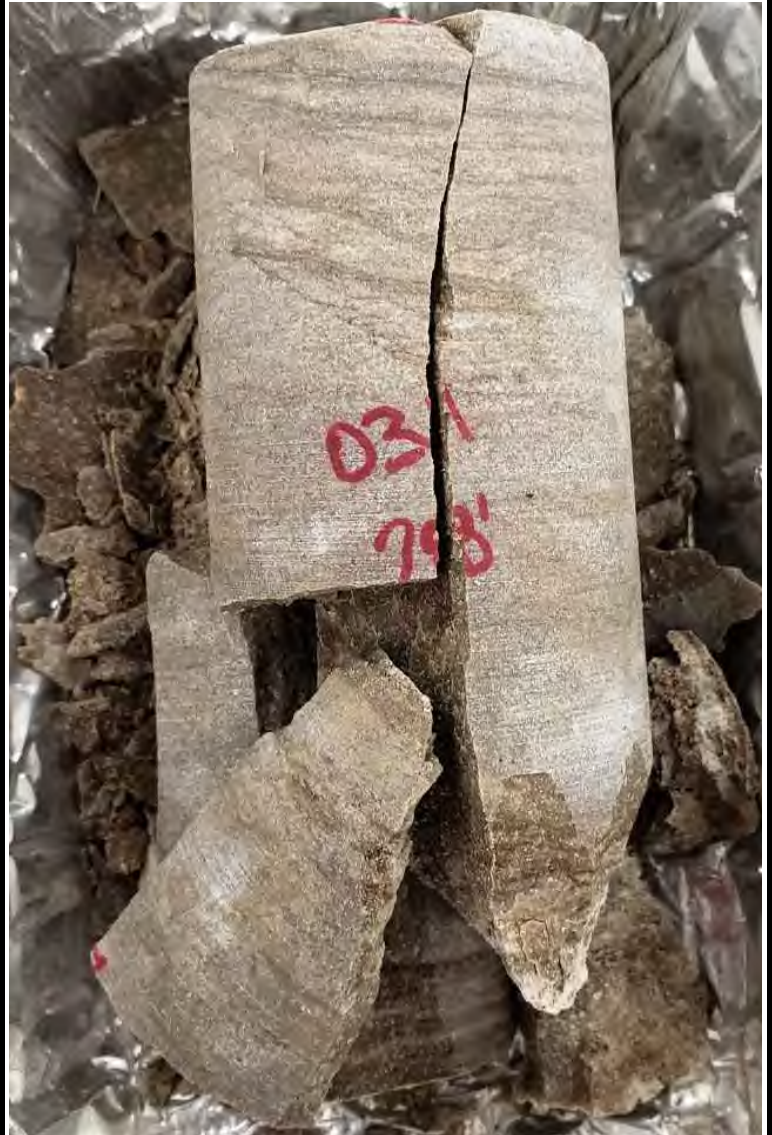
Ht to Dia. Ratio: 2.3 : 1

Remarks:: Specimen cut to given dimension without the use of water.

Before Test

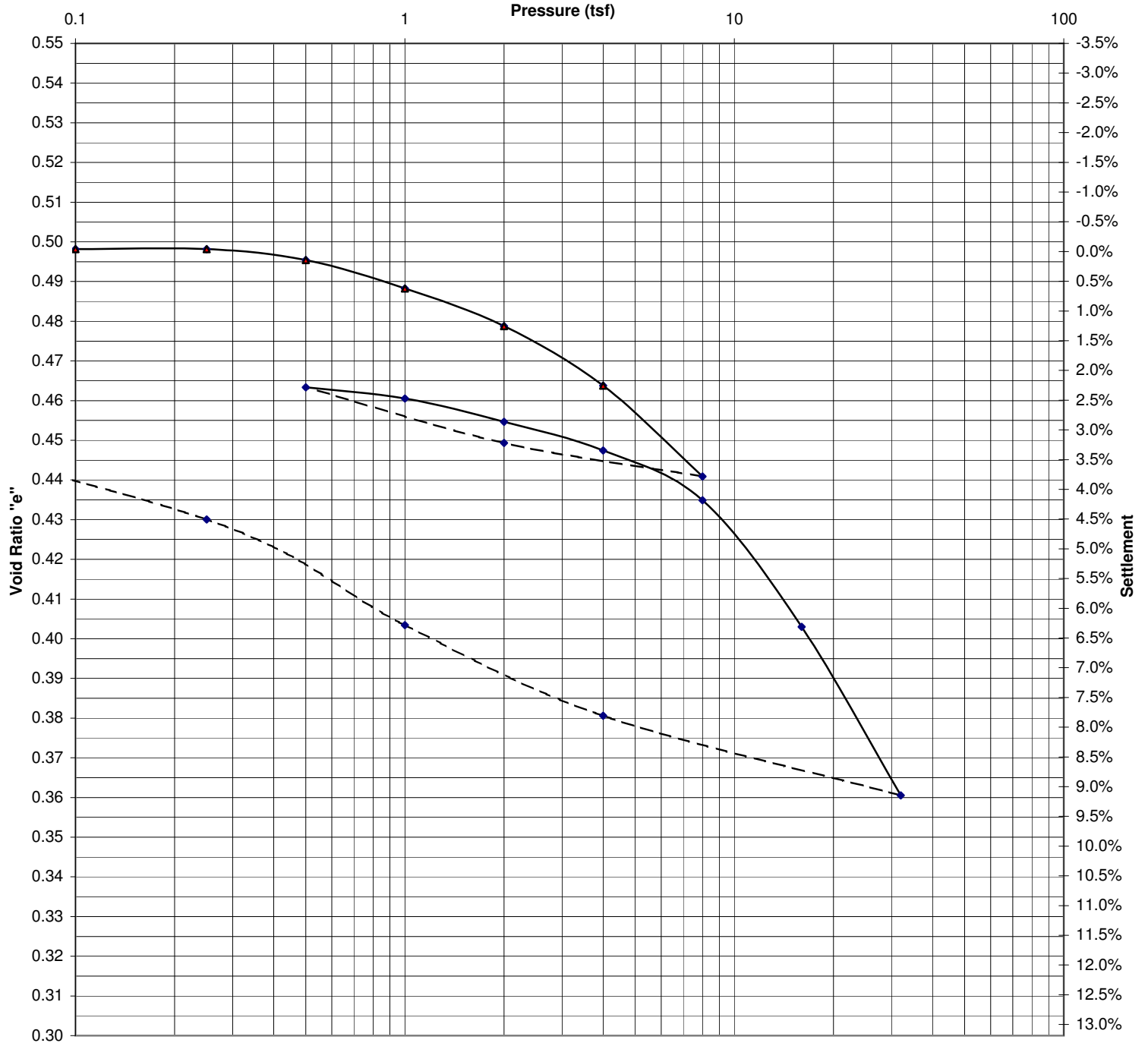


After Test



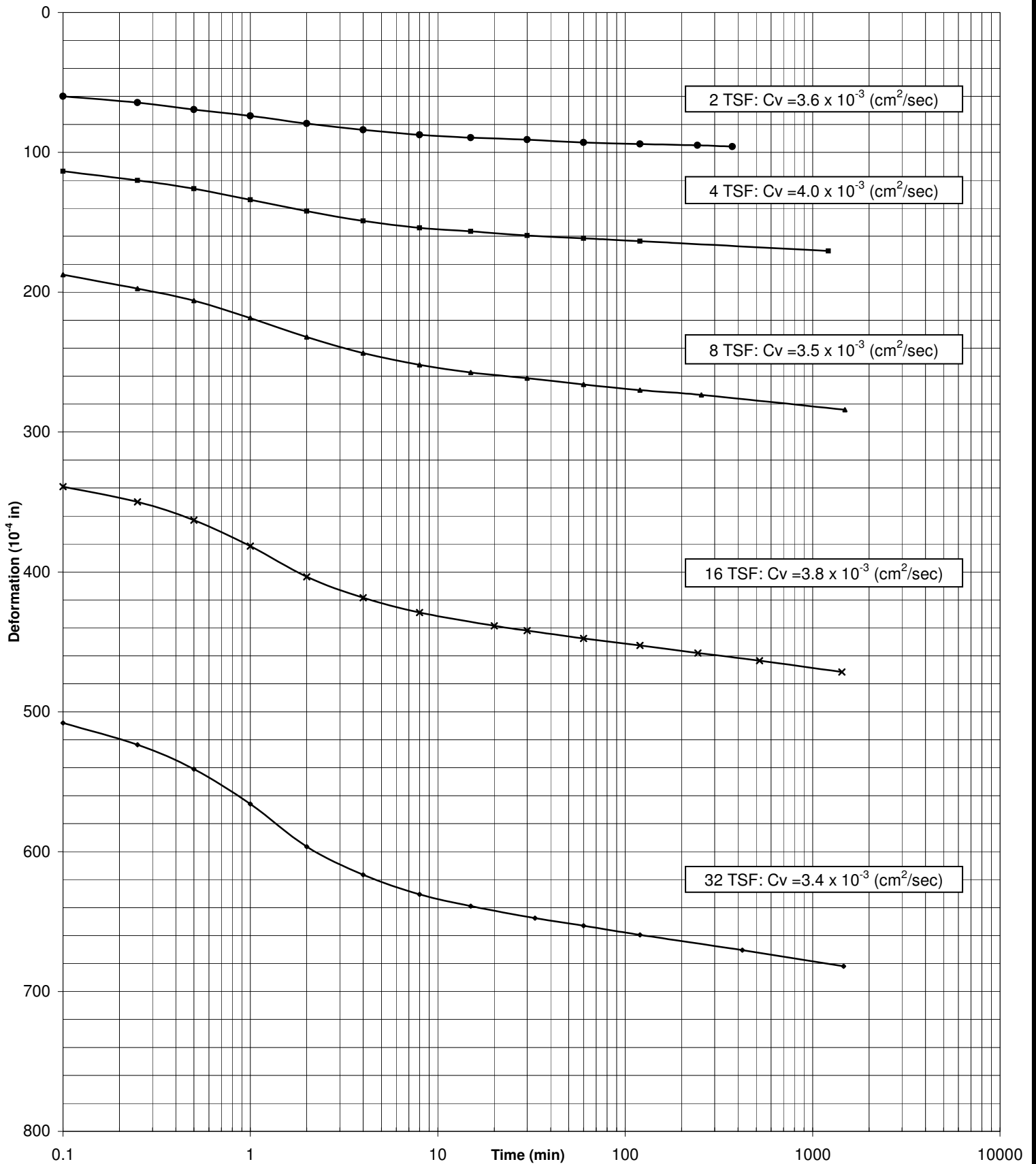
Consolidation Testing

Void Ratio and % Settlement vs. Log of Pressure



Project: Seneca Wind / Barr Engineering Company					Date: 5/24/18		
Sample #:		Boring #: Geo-037		Depth ft: 17-19		Job #: 11376	
Soil Type: Lean Clay w/a trace of gravel (CL)							
Initial W/C (%): 18.0		Dry Density (pcf): 113.4		LL: PL: PI:		Gs: 2.72 (Assumed)	
Organic Content (%):		Initial Height (in.): 0.743		Diameter (in.): 2.503		e _o = 0.498	
Preconsolidation Pressure (Pc): 4.0 tsf		Compression Index (Cc): 0.13		Recompression Index (Cr):			0.02
Remarks: Testing performed in general accordance with ASTM:D2435							

Consolidation Log of Time Curves



Project: Seneca Wind / Barr Engineering Company

Date: 5/24/18

Sample #:

Boring #: Geo-037

Depth ft: 17-19

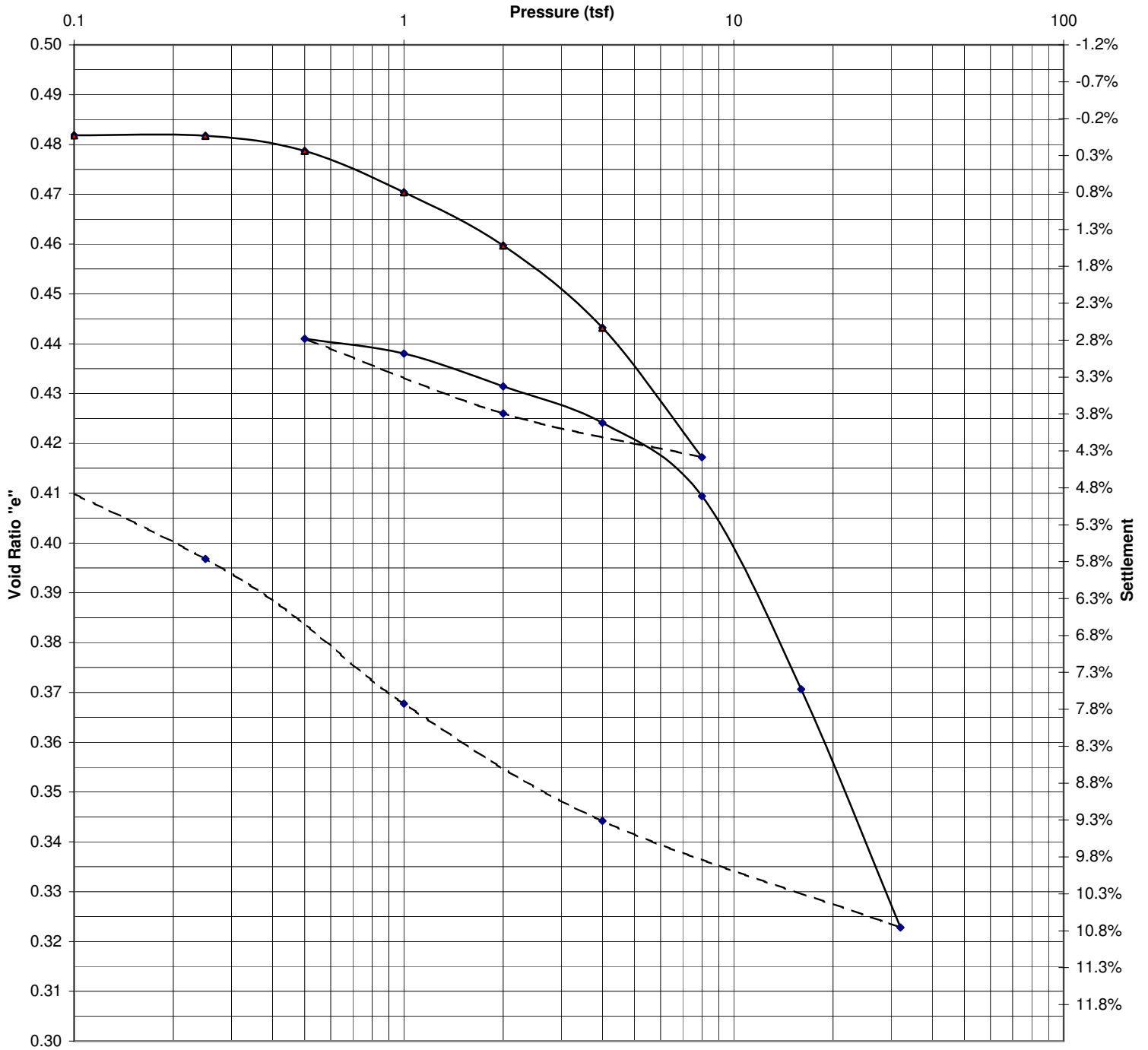
Job #: 11376

9530 James Avenue South



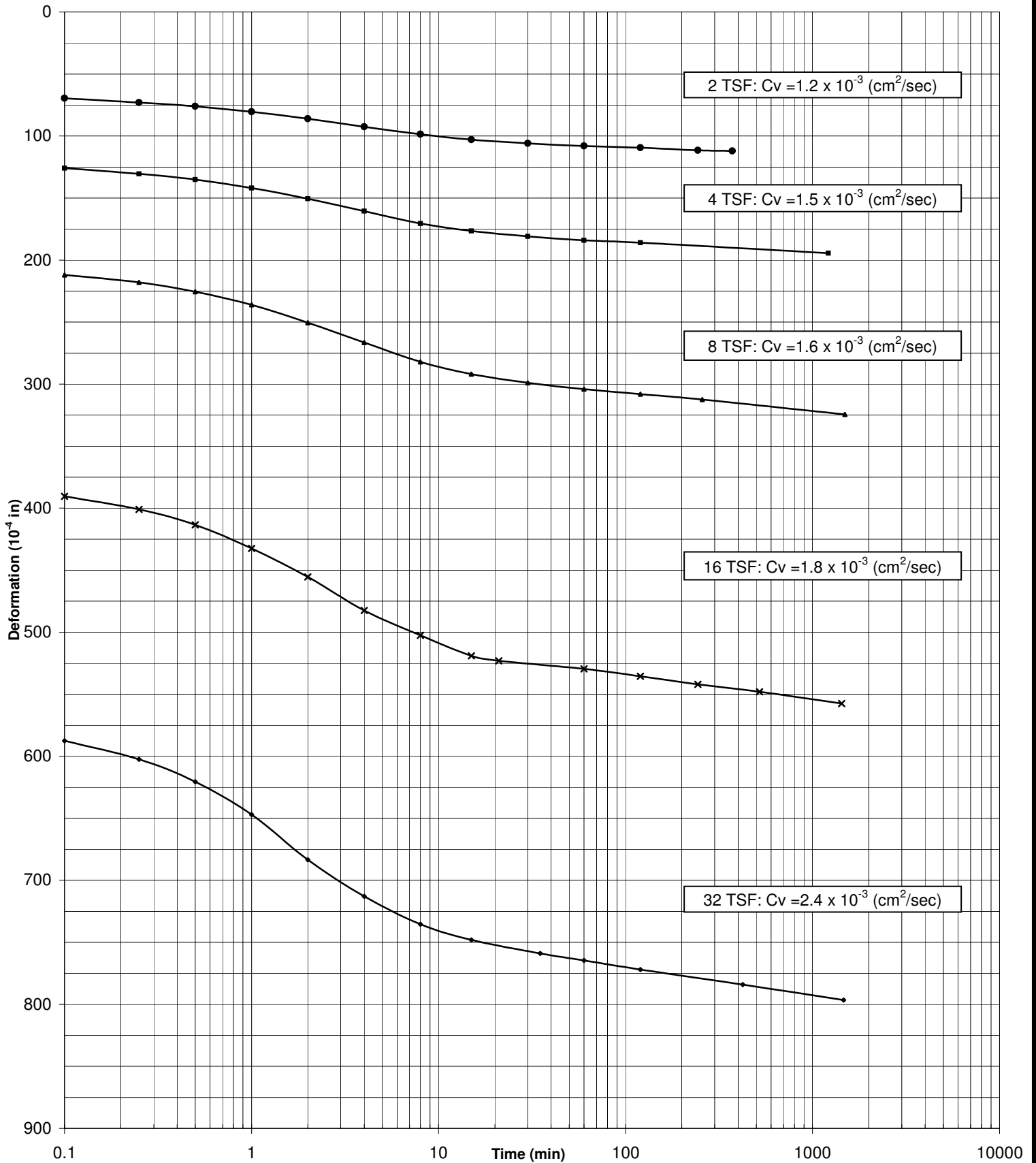
Bloomington, Minnesota 55431

Void Ratio and % Settlement vs. Log of Pressure



Project: Seneca Wind / Barr Engineering Company							Date:	5/22/18
Sample #:		Boring #: Geo-079			Depth ft: 20-22		Job #:	11376
Soil Type: Lean Clay w/a trace of gravel (CL)								
Initial W/C (%): 18.0		Dry Density (pcf): 113.7		LL:	PL:	PI:	Gs:	2.70 (Assumed)
Organic Content (%):		Initial Height (in.): 0.741		Diameter (in.): 2.506		e _o = 0.482		
Preconsolidation Pressure (Pc): 4.2 tsf			Compression Index (Cc): 0.16		Recompression Index (Cr): 0.03			
Remarks: Testing performed in general accordance with ASTM:D2435								

Consolidation Log of Time Curves



Project: Seneca Wind / Barr Engineering Company

Date: 5/22/18

Sample #:

Boring #: Geo-079

Depth ft: 20-22

Job #: 11376

9530 James Avenue South



Bloomington, Minnesota 55431

Chemical Testing

pH Testing Summary Sheet (ASTM:D4972)

Project: Seneca Wind Job: 11376
 Client: Barr Engineering Company Date: 5/18/2018

Boring / Location	Sample	Sample Type	Depth (ft)	pH	Visual Classification
Geo-005		Bulk	1-3	5.2	Lean Clay (CL)
Geo-006		Bulk	1-3	5.5	Lean Clay (CL)
Geo-015		Bulk	1-3	5.0	Lean Clay (CL/CH)
Geo-038		Bulk	1-3	4.6	Lean Clay w/sand and a trace of gravel (CL)
Geo-087		Bulk	1-3	6.4	Lean Clay (CL)
Geo-089		Bulk	1-3	5.1	Lean Clay (CL/CH)
Sub-1		Bulk	1-3	7.1	Lean Clay (CL/CH)

May 22, 2018

Soil Engineering Testing, Inc.

Sample Delivery Group: L993601
Samples Received: 05/12/2018
Project Number: 11376
Description: Seneca Wind

Report To: John Whelan
9530 James Ave. South
Bloomington, MN 55431

Entire Report Reviewed By:



John Hawkins
Technical Service Representative

Results relate only to the items tested or calibrated and are reported as rounded values. This test report shall not be reproduced, except in full, without written approval of the laboratory. Where applicable, sampling conducted by ESC is performed per guidance provided in laboratory standard operating procedures: 060302, 060303, and 060304.



Cp: Cover Page	1	¹ Cp
Tc: Table of Contents	2	
Ss: Sample Summary	3	² Tc
Cn: Case Narrative	4	
Sr: Sample Results	5	³ Ss
GEO-005 L993601-01	5	
GEO-006 L993601-02	6	⁴ Cn
GEO-015 L993601-03	7	⁵ Sr
GEO-038 L993601-04	8	
GEO-087 L993601-05	9	⁶ Qc
GEO-089 L993601-06	10	
SUB-01 L993601-07	11	⁷ Gl
Qc: Quality Control Summary	12	⁸ Al
Wet Chemistry by Method 9056A	12	
Gl: Glossary of Terms	14	⁹ Sc
Al: Accreditations & Locations	15	
Sc: Sample Chain of Custody	16	

SAMPLE SUMMARY

ONE LAB. NATIONWIDE.



GEO-005 L993601-01 Solid

			Collected by	Collected date/time	Received date/time
				05/11/18 10:00	05/12/18 08:45
Method	Batch	Dilution	Preparation date/time	Analysis date/time	Analyst
Wet Chemistry by Method 9056A	WG1110773	1	05/14/18 16:02	05/15/18 06:50	MAJ

¹ Cp

² Tc

³ Ss

GEO-006 L993601-02 Solid

			Collected by	Collected date/time	Received date/time
				05/11/18 10:00	05/12/18 08:45
Method	Batch	Dilution	Preparation date/time	Analysis date/time	Analyst
Wet Chemistry by Method 9056A	WG1114584	1	05/21/18 14:15	05/21/18 20:13	MAJ

⁴ Cn

⁵ Sr

GEO-015 L993601-03 Solid

			Collected by	Collected date/time	Received date/time
				05/11/18 10:00	05/12/18 08:45
Method	Batch	Dilution	Preparation date/time	Analysis date/time	Analyst
Wet Chemistry by Method 9056A	WG1114584	1	05/21/18 14:15	05/21/18 20:30	MAJ

⁶ Qc

⁷ Gl

GEO-038 L993601-04 Solid

			Collected by	Collected date/time	Received date/time
				05/11/18 10:00	05/12/18 08:45
Method	Batch	Dilution	Preparation date/time	Analysis date/time	Analyst
Wet Chemistry by Method 9056A	WG1114584	1	05/21/18 14:15	05/21/18 21:35	MAJ

⁸ Al

⁹ Sc

GEO-087 L993601-05 Solid

			Collected by	Collected date/time	Received date/time
				05/11/18 10:00	05/12/18 08:45
Method	Batch	Dilution	Preparation date/time	Analysis date/time	Analyst
Wet Chemistry by Method 9056A	WG1114584	1	05/21/18 14:15	05/21/18 21:52	MAJ

GEO-089 L993601-06 Solid

			Collected by	Collected date/time	Received date/time
				05/11/18 10:00	05/12/18 08:45
Method	Batch	Dilution	Preparation date/time	Analysis date/time	Analyst
Wet Chemistry by Method 9056A	WG1114584	1	05/21/18 14:15	05/21/18 22:08	MAJ

SUB-01 L993601-07 Solid

			Collected by	Collected date/time	Received date/time
				05/11/18 10:00	05/12/18 08:45
Method	Batch	Dilution	Preparation date/time	Analysis date/time	Analyst
Wet Chemistry by Method 9056A	WG1114584	1	05/21/18 14:15	05/21/18 22:25	MAJ



All sample aliquots were received at the correct temperature, in the proper containers, with the appropriate preservatives, and within method specified holding times, unless qualified or notated within the report. Where applicable, all MDL (LOD) and RDL (LOQ) values reported for environmental samples have been corrected for the dilution factor used in the analysis. All radiochemical sample results for solids are reported on a dry weight basis with the exception of tritium, carbon-14 and radon, unless wet weight was requested by the client. All Method and Batch Quality Control are within established criteria except where addressed in this case narrative, a non-conformance form or properly qualified within the sample results. By my digital signature below, I affirm to the best of my knowledge, all problems/anomalies observed by the laboratory as having the potential to affect the quality of the data have been identified by the laboratory, and no information or data have been knowingly withheld that would affect the quality of the data.

John Hawkins
Technical Service Representative

¹ Cp

² Tc

³ Ss

⁴ Cn

⁵ Sr

⁶ Qc

⁷ Gl

⁸ Al

⁹ Sc



Collected date/time: 05/11/18 10:00

L993601

Wet Chemistry by Method 9056A

Analyte	Result	Qualifier	RDL	Dilution	Analysis	Batch
	mg/kg		mg/kg		date / time	
Chloride	249		10.0	1	05/15/2018 06:50	WG1110773
Sulfate	ND		50.0	1	05/15/2018 06:50	WG1110773

¹ Cp

² Tc

³ Ss

⁴ Cn

⁵ Sr

⁶ Qc

⁷ Gl

⁸ Al

⁹ Sc



Wet Chemistry by Method 9056A

Analyte	Result mg/kg	Qualifier	RDL mg/kg	Dilution	Analysis date / time	Batch
Chloride	148		10.0	1	05/21/2018 20:13	WG1114584
Sulfate	69.4		50.0	1	05/21/2018 20:13	WG1114584

¹ Cp

² Tc

³ Ss

⁴ Cn

⁵ Sr

⁶ Qc

⁷ Gl

⁸ Al

⁹ Sc



Wet Chemistry by Method 9056A

Analyte	Result mg/kg	Qualifier	RDL mg/kg	Dilution	Analysis date / time	Batch
Chloride	85.2	J3	10.0	1	05/21/2018 20:30	WG1114584
Sulfate	73.0		50.0	1	05/21/2018 20:30	WG1114584

¹ Cp

² Tc

³ Ss

⁴ Cn

⁵ Sr

⁶ Qc

⁷ Gl

⁸ Al

⁹ Sc



Wet Chemistry by Method 9056A

Analyte	Result mg/kg	Qualifier	RDL mg/kg	Dilution	Analysis date / time	Batch
Chloride	110		10.0	1	05/21/2018 21:35	WG1114584
Sulfate	ND		50.0	1	05/21/2018 21:35	WG1114584

¹ Cp

² Tc

³ Ss

⁴ Cn

⁵ Sr

⁶ Qc

⁷ Gl

⁸ Al

⁹ Sc



Wet Chemistry by Method 9056A

Analyte	Result mg/kg	Qualifier	RDL mg/kg	Dilution	Analysis date / time	Batch
Chloride	169		10.0	1	05/21/2018 21:52	WG1114584
Sulfate	ND		50.0	1	05/21/2018 21:52	WG1114584

¹ Cp

² Tc

³ Ss

⁴ Cn

⁵ Sr

⁶ Qc

⁷ Gl

⁸ Al

⁹ Sc



Wet Chemistry by Method 9056A

Analyte	Result mg/kg	Qualifier	RDL mg/kg	Dilution	Analysis date / time	Batch
Chloride	106		10.0	1	05/21/2018 22:08	WG1114584
Sulfate	77.7		50.0	1	05/21/2018 22:08	WG1114584

¹ Cp

² Tc

³ Ss

⁴ Cn

⁵ Sr

⁶ Qc

⁷ Gl

⁸ Al

⁹ Sc



Wet Chemistry by Method 9056A

Analyte	Result mg/kg	Qualifier	RDL mg/kg	Dilution	Analysis date / time	Batch
Chloride	104		10.0	1	05/21/2018 22:25	WG1114584
Sulfate	ND		50.0	1	05/21/2018 22:25	WG1114584

¹ Cp

² Tc

³ Ss

⁴ Cn

⁵ Sr

⁶ Qc

⁷ Gl

⁸ Al

⁹ Sc



Method Blank (MB)

(MB) R3309824-1 05/14/18 22:53

	MB Result	MB Qualifier	MB MDL	MB RDL
Analyte	mg/kg		mg/kg	mg/kg
Chloride	U		0.795	10.0
Sulfate	U		0.570	50.0



L993598-02 Original Sample (OS) • Duplicate (DUP)

(OS) L993598-02 05/15/18 02:13 • (DUP) R3309824-6 05/15/18 02:28

	Original Result	DUP Result	Dilution	DUP RPD	DUP Qualifier	DUP RPD Limits
Analyte	mg/kg	mg/kg		%		%
Chloride	154	208	1	29.8	J3	15
Sulfate	ND	18.3	1	0.000		15

L993600-08 Original Sample (OS) • Duplicate (DUP)

(OS) L993600-08 05/15/18 06:20 • (DUP) R3309824-7 05/15/18 06:35

	Original Result	DUP Result	Dilution	DUP RPD	DUP Qualifier	DUP RPD Limits
Analyte	mg/kg	mg/kg		%		%
Chloride	68.9	67.2	1	2.56		15
Sulfate	ND	33.9	1	0.000		15

Laboratory Control Sample (LCS) • Laboratory Control Sample Duplicate (LCSD)

(LCS) R3309824-2 05/14/18 23:08 • (LCSD) R3309824-3 05/14/18 23:23

	Spike Amount	LCS Result	LCSD Result	LCS Rec.	LCSD Rec.	Rec. Limits	LCS Qualifier	LCSD Qualifier	RPD	RPD Limits
Analyte	mg/kg	mg/kg	mg/kg	%	%	%			%	%
Chloride	200	208	191	104	95.5	80.0-120			8.49	15
Sulfate	200	194	191	97.0	95.3	80.0-120			1.78	15

Method Blank (MB)

(MB) R3312026-1 05/21/18 18:02

	MB Result	MB Qualifier	MB MDL	MB RDL
Analyte	mg/kg		mg/kg	mg/kg
Chloride	U		0.795	10.0
Sulfate	U		0.570	50.0

L993601-03 Original Sample (OS) • Duplicate (DUP)

(OS) L993601-03 05/21/18 20:30 • (DUP) R3312026-4 05/21/18 20:46

	Original Result	DUP Result	Dilution	DUP RPD	DUP Qualifier	DUP RPD Limits
Analyte	mg/kg	mg/kg		%		%
Chloride	85.2	111	1	26.2	J3	15
Sulfate	73.0	81.2	1	10.6		15

L993605-09 Original Sample (OS) • Duplicate (DUP)

(OS) L993605-09 05/22/18 01:42 • (DUP) R3312026-5 05/22/18 01:58

	Original Result	DUP Result	Dilution	DUP RPD	DUP Qualifier	DUP RPD Limits
Analyte	mg/kg	mg/kg		%		%
Chloride	45.1	47.9	1	6.06		15

Laboratory Control Sample (LCS) • Laboratory Control Sample Duplicate (LCSD)

(LCS) R3312026-2 05/21/18 18:18 • (LCSD) R3312026-3 05/21/18 18:35

	Spike Amount	LCS Result	LCSD Result	LCS Rec.	LCSD Rec.	Rec. Limits	LCS Qualifier	LCSD Qualifier	RPD	RPD Limits
Analyte	mg/kg	mg/kg	mg/kg	%	%	%			%	%
Chloride	200	211	211	105	105	80.0-120			0.0584	15
Sulfate	200	215	214	107	107	80.0-120			0.119	15

1Cp

2Tc

3Ss

4Cn

5Sr

6Qc

7Gl

8Al

9Sc



Guide to Reading and Understanding Your Laboratory Report

The information below is designed to better explain the various terms used in your report of analytical results from the Laboratory. This is not intended as a comprehensive explanation, and if you have additional questions please contact your project representative.

Abbreviations and Definitions

MDL	Method Detection Limit.
ND	Not detected at the Reporting Limit (or MDL where applicable).
RDL	Reported Detection Limit.
Rec.	Recovery.
RPD	Relative Percent Difference.
SDG	Sample Delivery Group.
U	Not detected at the Reporting Limit (or MDL where applicable).
Analyte	The name of the particular compound or analysis performed. Some Analyses and Methods will have multiple analytes reported.
Dilution	If the sample matrix contains an interfering material, the sample preparation volume or weight values differ from the standard, or if concentrations of analytes in the sample are higher than the highest limit of concentration that the laboratory can accurately report, the sample may be diluted for analysis. If a value different than 1 is used in this field, the result reported has already been corrected for this factor.
Limits	These are the target % recovery ranges or % difference value that the laboratory has historically determined as normal for the method and analyte being reported. Successful QC Sample analysis will target all analytes recovered or duplicated within these ranges.
Original Sample	The non-spiked sample in the prep batch used to determine the Relative Percent Difference (RPD) from a quality control sample. The Original Sample may not be included within the reported SDG.
Qualifier	This column provides a letter and/or number designation that corresponds to additional information concerning the result reported. If a Qualifier is present, a definition per Qualifier is provided within the Glossary and Definitions page and potentially a discussion of possible implications of the Qualifier in the Case Narrative if applicable.
Result	The actual analytical final result (corrected for any sample specific characteristics) reported for your sample. If there was no measurable result returned for a specific analyte, the result in this column may state "ND" (Not Detected) or "BDL" (Below Detectable Levels). The information in the results column should always be accompanied by either an MDL (Method Detection Limit) or RDL (Reporting Detection Limit) that defines the lowest value that the laboratory could detect or report for this analyte.
Case Narrative (Cn)	A brief discussion about the included sample results, including a discussion of any non-conformances to protocol observed either at sample receipt by the laboratory from the field or during the analytical process. If present, there will be a section in the Case Narrative to discuss the meaning of any data qualifiers used in the report.
Quality Control Summary (Qc)	This section of the report includes the results of the laboratory quality control analyses required by procedure or analytical methods to assist in evaluating the validity of the results reported for your samples. These analyses are not being performed on your samples typically, but on laboratory generated material.
Sample Chain of Custody (Sc)	This is the document created in the field when your samples were initially collected. This is used to verify the time and date of collection, the person collecting the samples, and the analyses that the laboratory is requested to perform. This chain of custody also documents all persons (excluding commercial shippers) that have had control or possession of the samples from the time of collection until delivery to the laboratory for analysis.
Sample Results (Sr)	This section of your report will provide the results of all testing performed on your samples. These results are provided by sample ID and are separated by the analyses performed on each sample. The header line of each analysis section for each sample will provide the name and method number for the analysis reported.
Sample Summary (Ss)	This section of the Analytical Report defines the specific analyses performed for each sample ID, including the dates and times of preparation and/or analysis.

Qualifier Description

J3	The associated batch QC was outside the established quality control range for precision.
----	--

1 Cp

2 Tc

3 Ss

4 Cn

5 Sr

6 Qc

7 Gl

8 Al

9 Sc



ESC Lab Sciences is the only environmental laboratory accredited/certified to support your work nationwide from one location. One phone call, one point of contact, one laboratory. No other lab is as accessible or prepared to handle your needs throughout the country. Our capacity and capability from our single location laboratory is comparable to the collective totals of the network laboratories in our industry. The most significant benefit to our one location design is the design of our laboratory campus. The model is conducive to accelerated productivity, decreasing turn-around time, and preventing cross contamination, thus protecting sample integrity. Our focus on premium quality and prompt service allows us to be YOUR LAB OF CHOICE.

* Not all certifications held by the laboratory are applicable to the results reported in the attached report.

* Accreditation is only applicable to the test methods specified on each scope of accreditation held by ESC Lab Sciences.

State Accreditations

Alabama	40660	Nebraska	NE-OS-15-05
Alaska	17-026	Nevada	TN-03-2002-34
Arizona	AZ0612	New Hampshire	2975
Arkansas	88-0469	New Jersey–NELAP	TN002
California	2932	New Mexico ¹	n/a
Colorado	TN00003	New York	11742
Connecticut	PH-0197	North Carolina	Env375
Florida	E87487	North Carolina ¹	DW21704
Georgia	NELAP	North Carolina ³	41
Georgia ¹	923	North Dakota	R-140
Idaho	TN00003	Ohio–VAP	CL0069
Illinois	200008	Oklahoma	9915
Indiana	C-TN-01	Oregon	TN200002
Iowa	364	Pennsylvania	68-02979
Kansas	E-10277	Rhode Island	LA000356
Kentucky ^{1 6}	90010	South Carolina	84004
Kentucky ²	16	South Dakota	n/a
Louisiana	AI30792	Tennessee ^{1 4}	2006
Louisiana ¹	LA180010	Texas	T 104704245-17-14
Maine	TN0002	Texas ⁵	LAB0152
Maryland	324	Utah	TN00003
Massachusetts	M-TN003	Vermont	VT2006
Michigan	9958	Virginia	460132
Minnesota	047-999-395	Washington	C847
Mississippi	TN00003	West Virginia	233
Missouri	340	Wisconsin	9980939910
Montana	CERT0086	Wyoming	A2LA

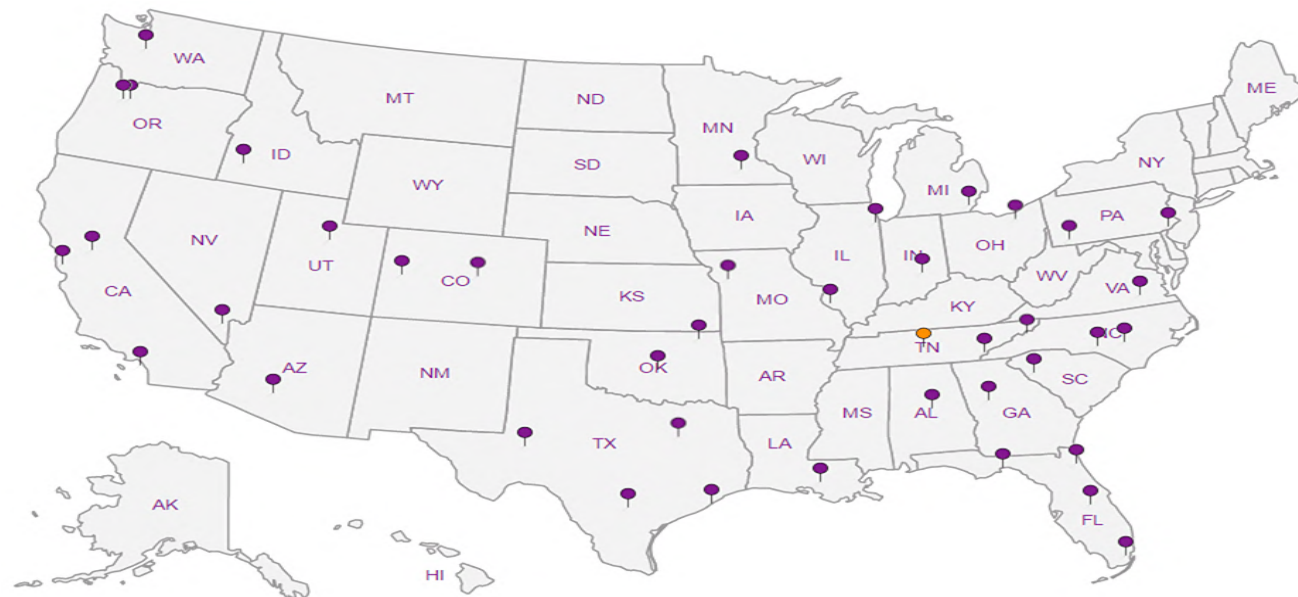
Third Party Federal Accreditations

A2LA – ISO 17025	1461.01	AIHA-LAP, LLC EMLAP	100789
A2LA – ISO 17025 ⁵	1461.02	DOD	1461.01
Canada	1461.01	USDA	P330-15-00234
EPA–Crypto	TN00003		

¹ Drinking Water ² Underground Storage Tanks ³ Aquatic Toxicity ⁴ Chemical/Microbiological ⁵ Mold ⁶ Wastewater n/a Accreditation not applicable

Our Locations

ESC Lab Sciences has sixty-four client support centers that provide sample pickup and/or the delivery of sampling supplies. If you would like assistance from one of our support offices, please contact our main office. ESC Lab Sciences performs all testing at our central laboratory.



Soil Engineering Testing, Inc.

9530 James Ave. South
Bloomington, MN 55431

Billing Information

John Whelan
9530 James Ave. South
Bloomington, MN 55431

Report to:

JOHN WHELAN

Email To:

whelan@soilengineeringtesting.com

Project

Description: SENECA WIND

City/State

Collected:

Phone: 952-884-6833

Fax: 952-884-6923

Client Project

11376

Lab Project

Collected by (print):

Site/Facility ID #

P.O. #

Collected by (signature):

Rush? (Lab MUST Be Notified)

Same Day Five Day
Next Day 5 Day (Rad Only)
Two Day 10 Day (Rad Only)
Three Day

Quote #

Date Results Needed

Immediately
Packed on Ice N Y

No.

Sample ID	Comp/Grab	Matrix *	Depth	Date	Time	Enters
-----------	-----------	----------	-------	------	------	--------

SULFATE ION

CHLORIDE ION

GEO-005		SS	1-3'	5/11/18	10:00	1	X	X
GEO-006						1	X	X
GEO-015						1	X	X
GEO-038						1	X	X
GEO-087						1	X	X
GEO-089						1	X	X
SUB-01						1	X	X

* Matrix
SS - Soil AMR - Air F - Filter
GW - Groundwater B - Bioassay
WW - Wastewater
DW - Drinking Water
OT - Other

Remarks:

Samples returned via:

UPS X FedEx Courier

Tracking # 4276 0145 0A29

pH Temp

Flow Other

Sample Receipt Checklist

COC Seal Present/Intact: ☒ Y ☐ N
COC Signed/Accurate: ☒ Y ☐ N
Bottles Arrive Intact: ☒ Y ☐ N
Correct bottles used: ☒ Y ☐ N
Sufficient volume sent: ☒ Y ☐ N
If Applicable
VOA Zero Headpace: ☐ Y ☒ N
Preservation Correct/Checked: ☐ Y ☒ N

Relinquished by (Signature)

Date

5/11/18

Time

15:00

Received by (Signature)

Trip Blank Received: Yes ☒ No

HCL/MeOH
TBR

Relinquished by (Signature)

Date

Time

Received by (Signature)

Temp: °C Bottles Received

7

If preservation required by Login: Date/Time

Relinquished by (Signature)

Date

Time

Received for lab by (Signature)

Date: 5/12/18

0845

Condition

NCF 10



12088 Lebanon Rd
Mount Juliet, TN 37122
Phone: 615-758-5858
Phone: 800-787-5859
Fax: 615-758-5859



L# 493601

H198

Account: SOIENG8MN

Template:

Prologis:

TSR: 341 - John Hawkins

PB:

Shipped Via:

Remarks Samples

Appendix E

Electrical Resistivity Report

May 24, 2018

Mr. Mike Goodwin
Construction Manager
sPower
2180 South 1300 East, Suite 600
Salt Lake City, Utah 84106

**Re: Soil Electrical Resistivity Testing
Seneca Wind Project
Seneca County, Ohio**

Dear Mr. Goodwin:

Barr Engineering Co. (Barr), under contract and authorization from sPower, performed soil electrical resistivity measurements at the Seneca Wind Project site in April of 2018 as part of the design phase geotechnical investigation. This letter presents the methods and results of the requested testing.

Methods

A total of seven sets of electrical resistivity tests were conducted at the site by Barr personnel. Six tests were performed at proposed turbine locations, and one test was performed at the proposed substation location. Test locations were provided by s-Power prior to beginning the field investigation.

Barr personnel verified the test locations with a handheld GPS unit. Barr personnel located the electrical resistivity sample locations based on the provided coordinates for the corresponding boring locations.

The test locations are shown on Figure 1. Coordinates of each test location and other pertinent information can be found in Table 1.

Table 1 Testing Conditions and Coordinates

Resistivity Testing Conditions		UTM Coordinates, Zone 17 [m] [NAD83]	
Geotech ID (Turbine ID-4/4/18 Array)	Ambient Air Temperature [°F]	Easting	Northing
GEO-005 (2.3-5)	50	330232.5	4550876.8
GEO-006 (2.3-6)	50	334368.0	4551577.3
GEO-015 (3.8-3)	55	323462.6	4546020.6
GEO-038 (3.8-24)	50	339979.5	4549899.6
GEO-087 (3.8-16)	55	327652.3	4547804.6
GEO-089 (3.8-37)	50	328728.7	4545293.9
SUB-Res	55	327329.0	4547035.0

Barr conducted the work in accordance with ASTM method G57 "Standard Test Method for Field Measurement of Soil Resistivity Using the Wenner Four-Electrode Method" (equivalent to IEEE Std. 81). A

single resistivity array orientation was utilized at each test location and resistivity measurements in the array orientation were obtained. Each measurement at the tested locations corresponded to one of five electrode ("a") spacings: 2.5, 5, 10, 20, and 40 feet.

The equipment used to collect the data consisted of a resistivity meter, four metal electrodes and connecting wire. A Mini-Res Ultra resistivity meter manufactured by L & R Instruments, Inc. was used to collect the data. The resistivity meter read in resistance (Ω) directly, and did not require the conversion of electrical potential (V) and inductance (I) to calculate resistance (V/I in Ω). Before and after each array was completed, the resistivity meter was connected to a resistor of known resistance, and the resulting values were compared to the known resistance value for quality assurance and quality control purposes. The meter was properly calibrated for all test locations and no instrument adjustments had to be made.

Co-linear arrays of four electrodes were placed in the ground for each measurement. Electrical current was input to the ground through the two outer electrodes of the array. The voltage drop produced by the resulting electrical field was measured across the two inner electrodes. The "a" spacing was increased with each measurement, expanding the array about a common center for the tested locations. Increasing the electrode separation increases the depth of investigation and indicates vertical variation in resistivity.

Apparent resistivity (ρ_a) was calculated for each measurement and corresponding electrode spacing (a) using the resistance measurement (Ω) and the geometric factor (K) as follows:

$$\rho_a = K(V/I) \text{ where } K = 2\pi a$$

All field measurements and calculated values of apparent resistivity are presented in the attachments.

Results and Discussion

Soil resistivity variations are likely associated with differences in soil type, layer thicknesses, and degree of water saturation or frost in the near surface soils. Higher moisture contents generally reduce the electrical resistivity of a soil. Sandy soils generally have a higher resistivity than clayey and silty soils. Clay soils with moderate moisture contents tend to exhibit lower resistivity on the order of 1,000 to 5,000 ohm-centimeters (Ω -cm). Bedrock tends to exhibit relatively high resistivity values on the order of 10,000 to over 100,000 Ω -cm.

Apparent resistivity measurements at the turbine locations ranged from 2,246 to 16,183 (Ω -cm), with an average of 4,317 Ω -cm. Apparent resistivity measurements at the substation location ranged from 2,662 to 3,827 Ω -cm, with an average of 3,379 Ω -cm.

The test results indicate some variability with the smaller "a" spacings, but in general, the apparent resistivity was relatively consistent across the site. Most of the readings fell within a relatively tight range of about 3,000 to 5,000 Ω -cm, with the exception of measurements at "a" spacings of 20 and 40 feet at GEO-089, which indicated much higher values, likely indicative of relatively shallow bedrock, which was confirmed during the soil borings.

In general, apparent resistivity measurements in soil that are less than 2,000 Ω -cm indicate that the soil could be considered corrosive when in direct contact with steel foundations. The apparent resistivity measurements at the tested locations indicate that the soil is not likely to be corrosive for steel in direct contact with native soils at all tested locations.

Table 2 provides a summary of the range found in apparent resistivity with electrode “a” spacing at the turbine locations.

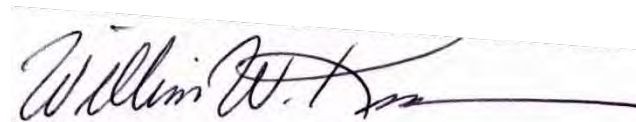
Table 2 Summary of Apparent Resistivity versus Electrode Spacing at Turbine Locations

Turbine Locations Electrode Spacing [Feet]	Apparent Resistivity [Ω -cm]		
	Range	Mean	Standard Deviation
2.5	2,921 – 8,015	4,268	1,469
5	2,969 – 5,693	3,815	863
10	2,856 – 5,875	3,810	1,036
20	2,388 – 8,750	4,109	2,188
40	2,246 – 16,183	5,585	4,884

Closing

Thank you for the opportunity to provide this service. Please call me at 952-832-2797 with questions or requests for additional information.

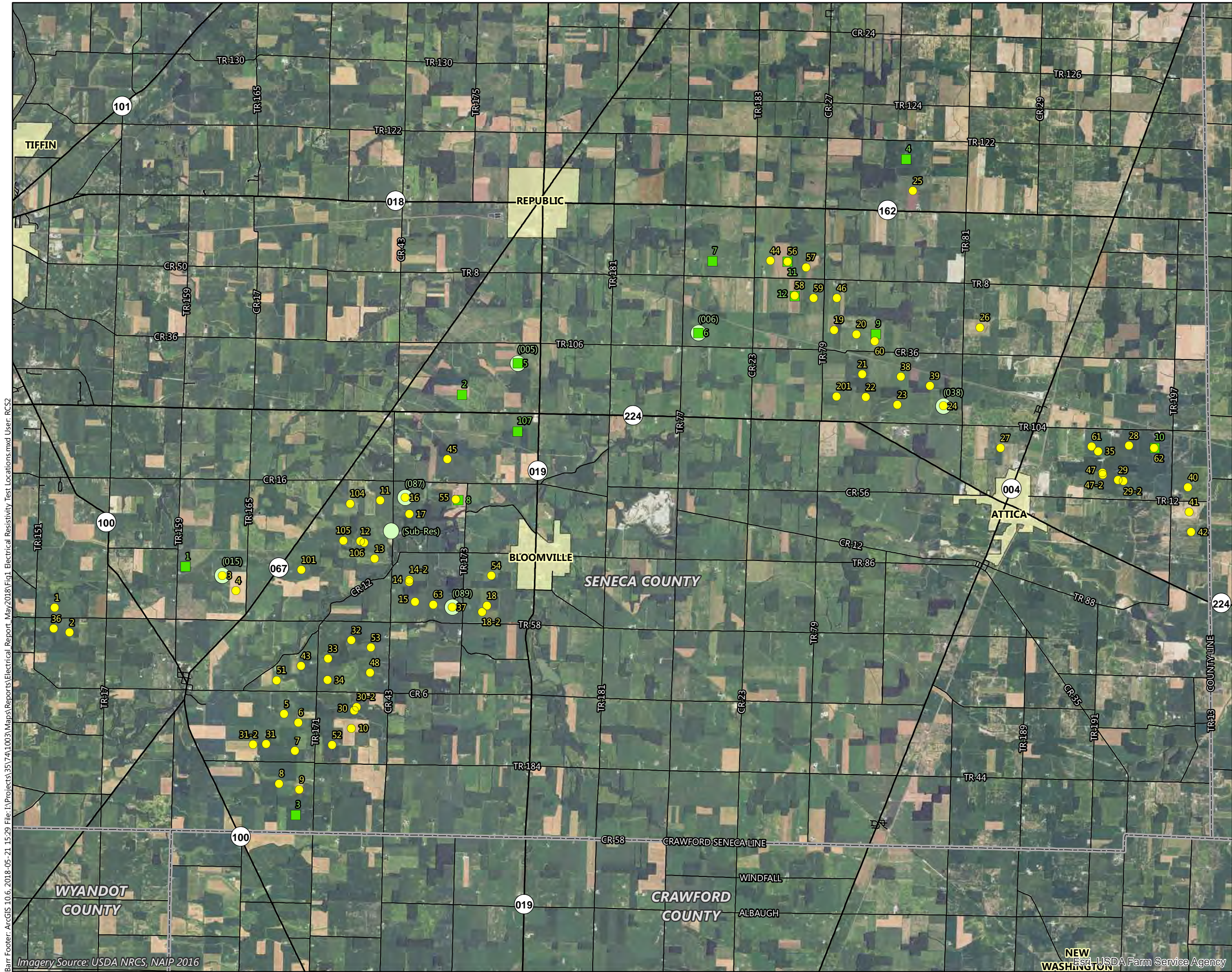
Sincerely,



William W. Kussmann

Attachments:

Electrical Resistivity Testing Results
Figure 1: Electrical Resistivity Testing Locations



- Turbine Location - 3.8 (4/4/2018)
- Turbine Location - 2.3 (4/4/2018)
- City Boundary
- County Boundary
- Electrical Resistivity Test Locations

*Note: Labeled with GEO- ID in parentheses.

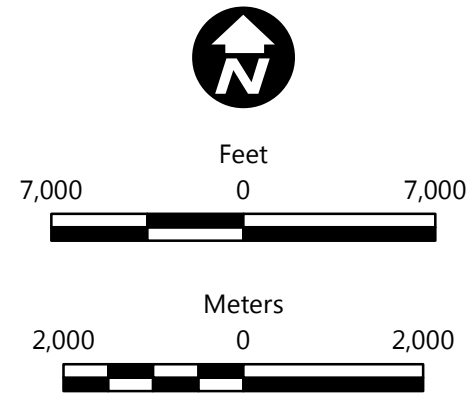


Figure 1

**ELECTRICAL RESISTIVITY
TEST LOCATIONS**
Seneca Wind Project
S-Power
Seneca County, Ohio

WENNER SOUNDING

PROJECT: Seneca Wind Project

Sounding No. 1

Date 4/26/2018

Observer IGM

Location GEO-038

Electrode Spacing	Resistance	Geometric Factor	Apparent Resistivity	Apparent Resistivity
"a"	V/I	$K=2\pi a$	$\rho_a=K(V/I)$	$\rho_a=K(V/I)$
feet	Ohms	feet	Ohm-feet	Ohm-cm
N/S Orientation				
2.5	6.900	15.71	108.38	3304.4
5	3.300	31.42	103.67	3160.7
10	1.551	62.83	97.43	2970.3
20	0.623	125.66	78.31	2387.6
40	0.309	251.33	77.64	2366.9
E/W Orientation				
2.5	7.700	15.71	120.95	3687.5
5	3.500	31.42	109.96	3352.3
10	1.512	62.83	95.01	2896.8
20	0.625	125.66	78.49	2393.0
40	0.293	251.33	73.66	2245.9

Cultural Features

None

Ground Cover

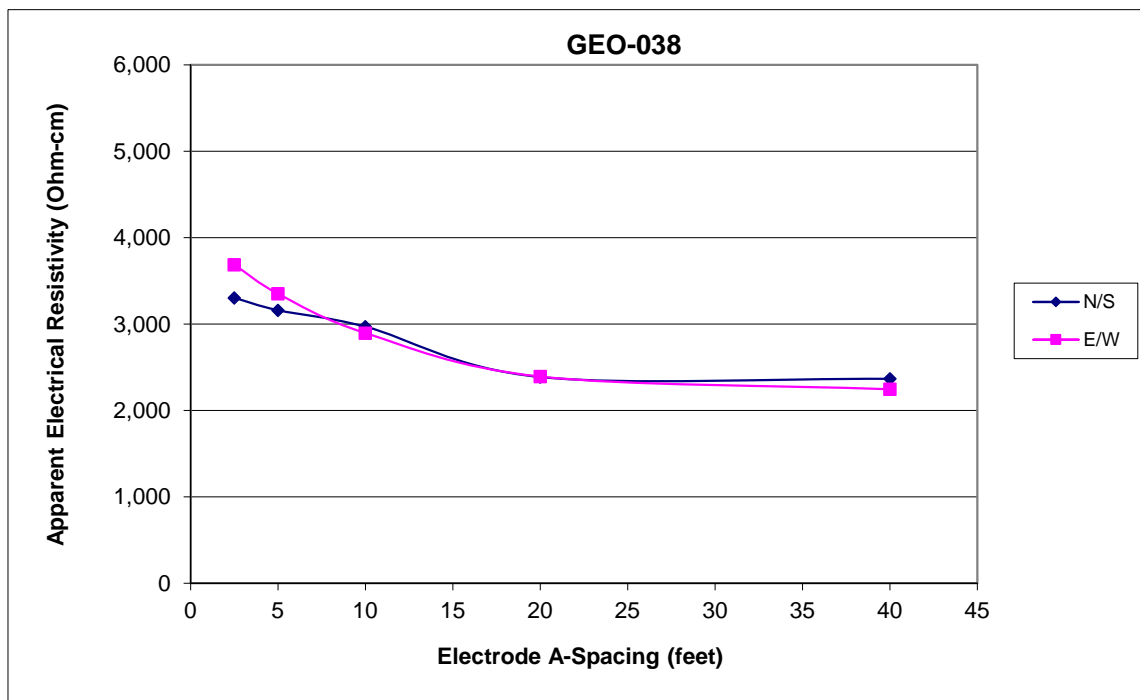
Bean crop

Weather

50F

Line Location and Bearing

NS/EW



WENNER SOUNDING

PROJECT: Seneca Wind Project

Sounding No. 2

Date 4/26/2018

Observer IGM

Location GEO-006

Electrode Spacing "a"	Resistance V/I	Geometric Factor $K=2\pi a$	Apparent Resistivity $\rho_a=K(V/I)$	Apparent Resistivity $\rho_a=K(V/I)$
feet	Ohms	feet	Ohm-feet	Ohm-cm
N/S Orientation				
2.5	7.500	15.71	117.81	3591.8
5	3.100	31.42	97.39	2969.2
10	1.496	62.83	94.01	2866.1
20	0.694	125.66	87.17	2657.7
40	0.368	251.33	92.44	2818.2
E/W Orientation				
2.5	7.800	15.71	122.52	3735.4
5	3.500	31.42	109.96	3352.3
10	1.491	62.83	93.69	2856.4
20	0.670	125.66	84.24	2568.4
40	0.362	251.33	90.85	2770.0

Cultural Features

None

Ground Cover

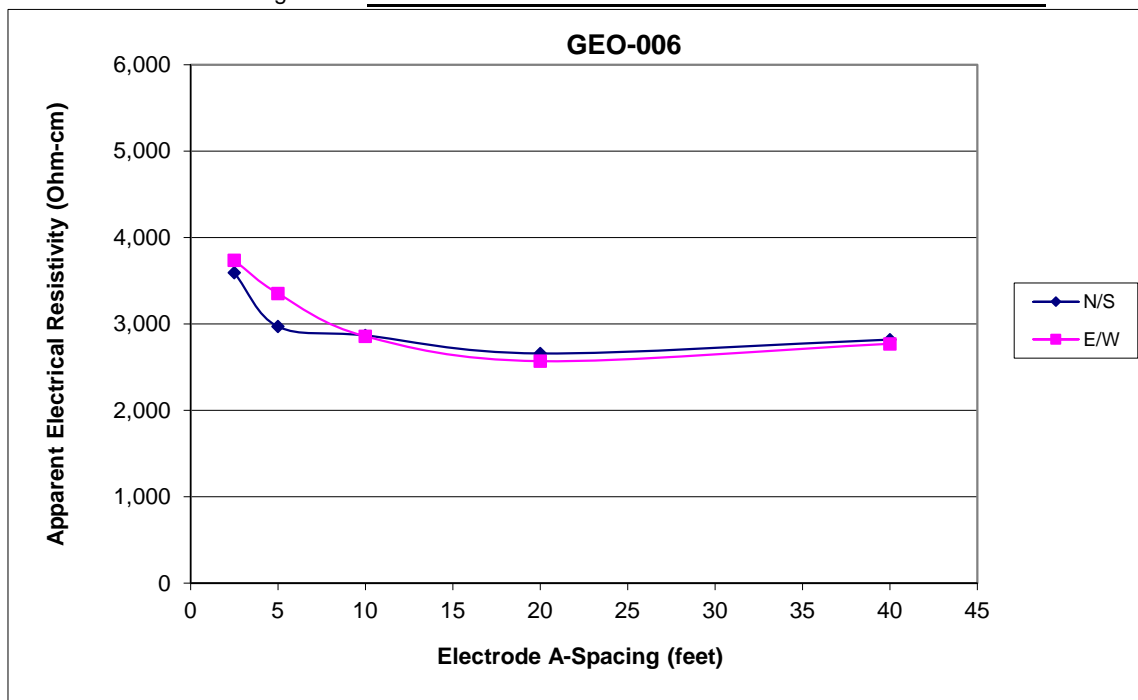
Corn field

Weather

50F

Line Location and Bearing

NS/EW



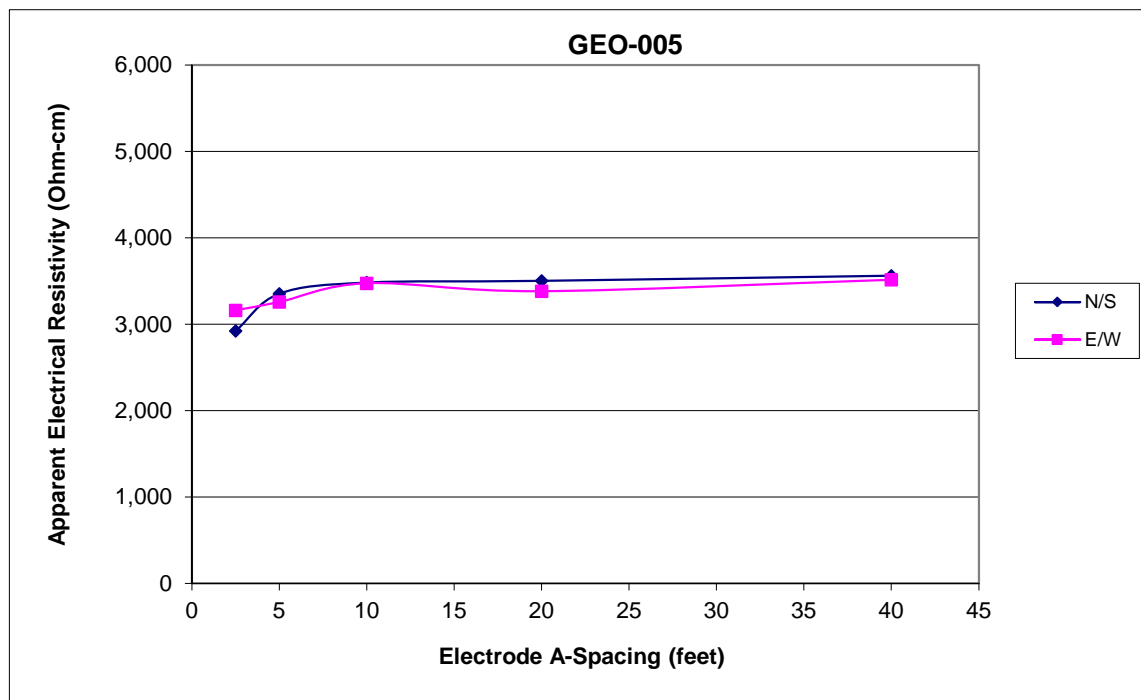
WENNER SOUNDING

PROJECT: Seneca Wind Project

Sounding No. 3 Date 4/26/2018
Observer IGM
Location GEO-005

Electrode Spacing	Resistance	Geometric Factor	Apparent Resistivity	Apparent Resistivity
"a"	V/I	$K=2\pi a$	$\rho_a=K(V/I)$	$\rho_a=K(V/I)$
feet	Ohms	feet	Ohm-feet	Ohm-cm
N/S Orientation				
2.5	6.100	15.71	95.82	2921.3
5	3.500	31.42	109.96	3352.3
10	1.817	62.83	114.18	3481.2
20	0.914	125.66	114.84	3501.3
40	0.465	251.33	116.79	3560.7
E/W Orientation				
2.5	6.600	15.71	103.67	3160.7
5	3.400	31.42	106.81	3256.5
10	1.813	62.83	113.94	3473.8
20	0.883	125.66	110.91	3381.4
40	0.459	251.33	115.26	3514.0

Cultural Features None
Ground Cover Bean field
Weather 50F
Line Location and Bearing NS/EW



WENNER SOUNDING

PROJECT: Seneca Wind Project

Sounding No. 4

Date 4/26/2018

Observer IGM

Location GEO-087

Electrode Spacing "a"	Resistance V/I	Geometric Factor $K=2\pi a$	Apparent Resistivity $\rho_a=K(V/I)$	Apparent Resistivity $\rho_a=K(V/I)$
feet	Ohms	feet	Ohm-feet	Ohm-cm
N/S Orientation				
2.5	8.900	15.71	139.80	4262.2
5	4.100	31.42	128.81	3927.0
10	2.100	62.83	131.95	4022.8
20	0.957	125.66	120.31	3668.0
40	0.548	251.33	137.83	4202.1
E/W Orientation				
2.5	8.477	15.71	133.16	4059.6
5	4.000	31.42	125.66	3831.2
10	2.100	62.83	131.95	4022.8
20	0.941	125.66	118.19	3603.3
40	0.549	251.33	137.98	4206.7

Cultural Features

None

Ground Cover

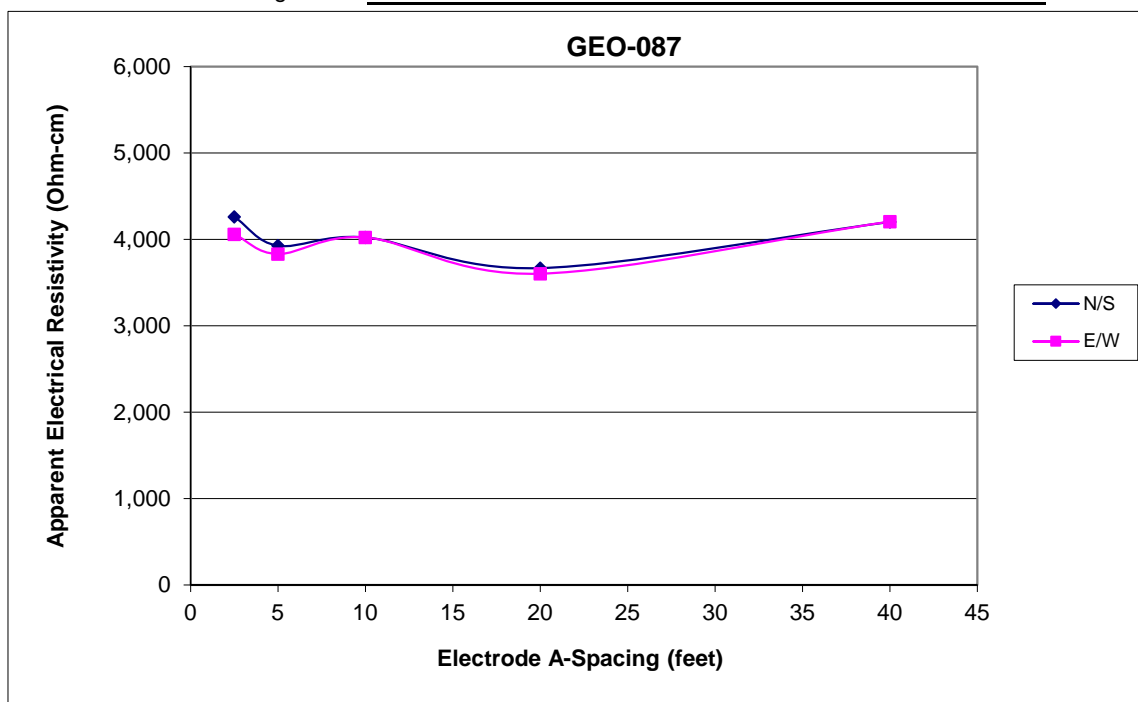
Cornfield

Weather

55F

Line Location and Bearing

NS/EW



WENNER SOUNDING

PROJECT: Seneca Wind Project

Sounding No. 5

Date 4/27/2018

Observer IGM

Location GEO-089

Electrode Spacing "a"	Resistance V/I	Geometric Factor $K=2\pi a$	Apparent Resistivity $\rho_a=K(V/I)$	Apparent Resistivity $\rho_a=K(V/I)$
feet	Ohms	feet	Ohm-feet	Ohm-cm
N/S Orientation				
2.5	13.264	15.71	208.35	6352.1
5	5.648	31.42	177.44	5409.7
10	3.067	62.83	192.71	5875.2
20	2.284	125.66	287.02	8750.5
40	2.112	251.33	530.80	16183.0
E/W Orientation				
2.5	16.737	15.71	262.90	8015.4
5	5.944	31.42	186.74	5693.2
10	3.018	62.83	189.63	5781.3
20	2.220	125.66	278.97	8505.3
40	2.031	251.33	510.45	15562.4

Cultural Features

None

Ground Cover

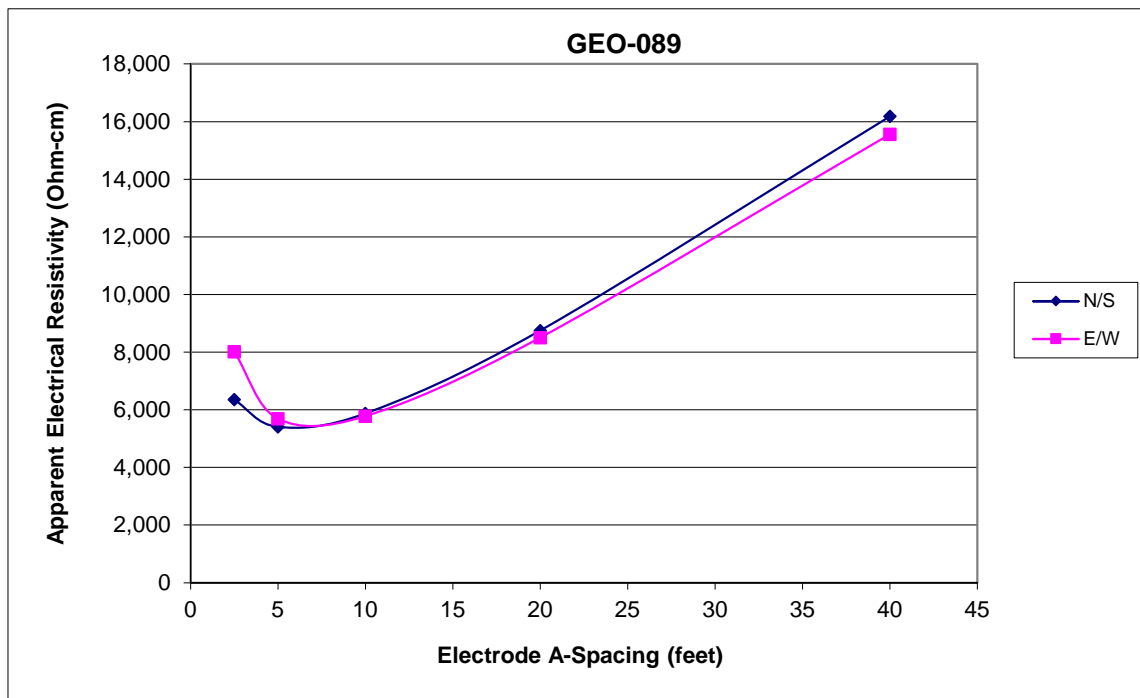
Grassfield

Weather

50F

Line Location and Bearing

NS/EW



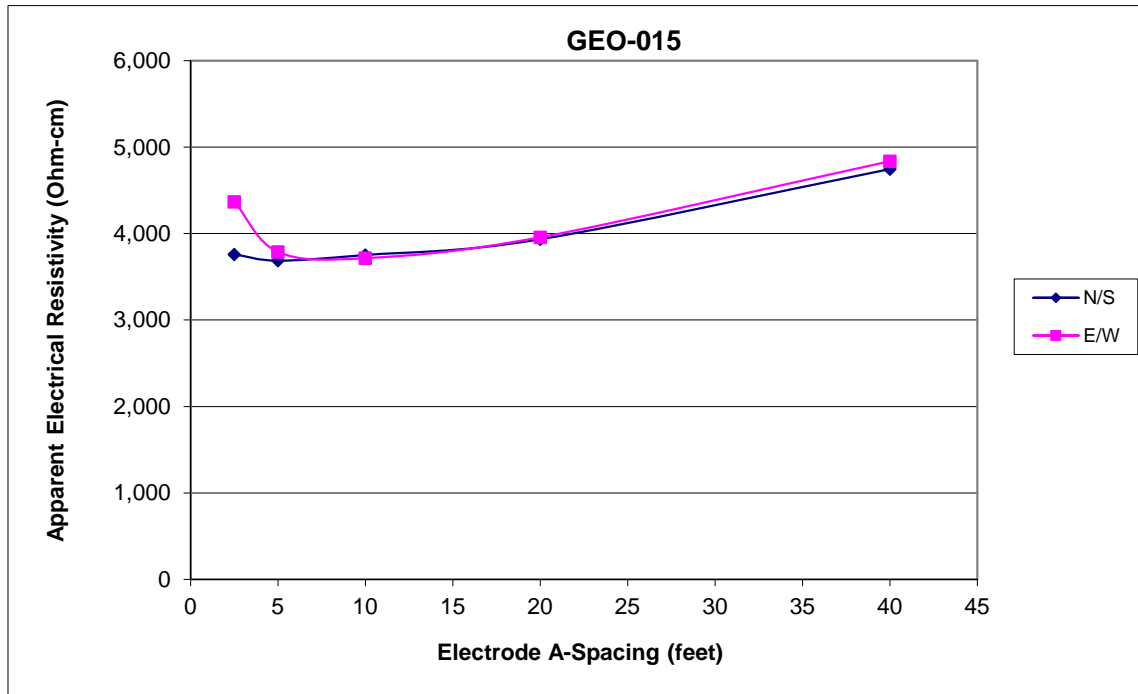
WENNER SOUNDING

PROJECT: Seneca Wind Project

Sounding No. 6 Date 4/27/2018
Observer IGM
Location GEO-015

Electrode Spacing "a"	Resistance V/I	Geometric Factor $K=2\pi a$	Apparent Resistivity $\rho_a=K(V/I)$	Apparent Resistivity $\rho_a=K(V/I)$
feet	Ohms	feet	Ohm-feet	Ohm-cm
N/S Orientation				
2.5	7.851	15.71	123.32	3759.9
5	3.850	31.42	120.95	3687.5
10	1.959	62.83	123.09	3752.7
20	1.027	125.66	129.07	3935.0
40	0.620	251.33	155.70	4746.9
E/W Orientation				
2.5	9.124	15.71	143.32	4369.5
5	3.956	31.42	124.28	3789.1
10	1.940	62.83	121.89	3716.3
20	1.033	125.66	129.82	3958.0
40	0.631	251.33	158.69	4838.1

Cultural Features None
Ground Cover Grass field
Weather 55F
Line Location and Bearing NS/EW



WENNER SOUNDING

PROJECT: Seneca Wind Project

Sounding No. 7

Date 4/27/2018

Observer IGM

Location SUB-Res

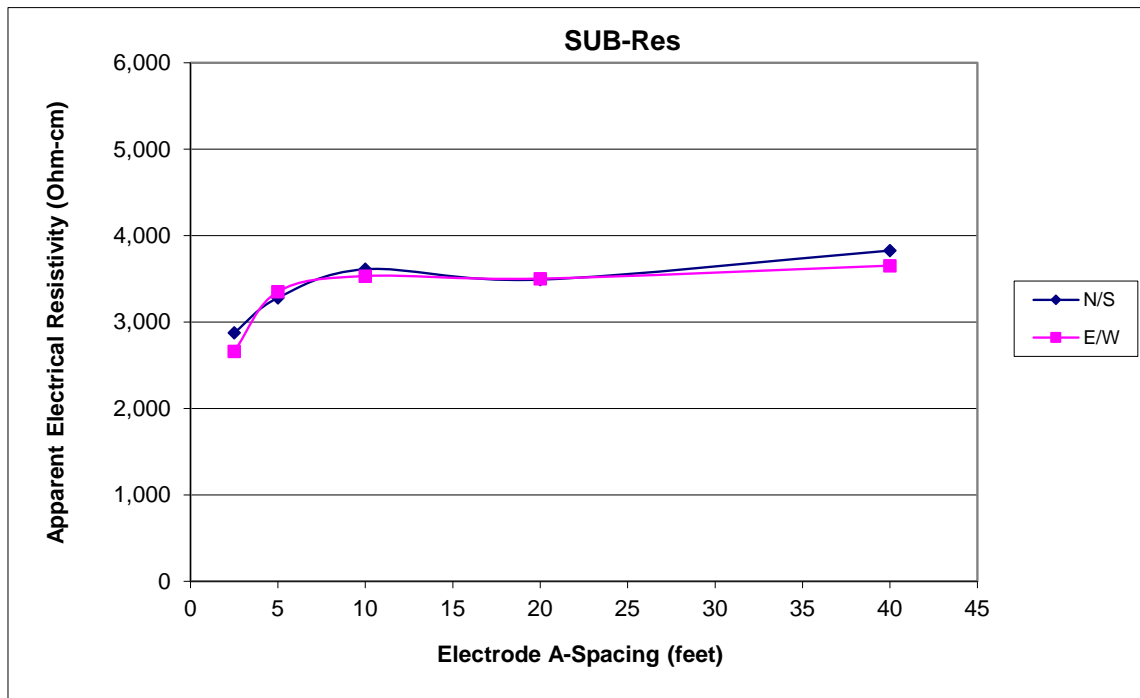
Electrode Spacing "a" feet	Resistance V/I Ohms	Geometric Factor $K=2\pi a$ feet	Apparent Resistivity $\rho_a=K(V/I)$ Ohm-feet	Apparent Resistivity $\rho_a=K(V/I)$ Ohm-cm
N/S Orientation				
2.5	6.005	15.71	94.33	2875.8
5	3.425	31.42	107.60	3280.5
10	1.885	62.83	118.44	3610.9
20	0.911	125.66	114.50	3491.0
40	0.500	251.33	125.54	3827.4
E/W Orientation				
2.5	5.559	15.71	87.32	2662.2
5	3.499	31.42	109.92	3351.4
10	1.844	62.83	115.86	3532.4
20	0.914	125.66	114.88	3502.5
40	0.477	251.33	119.81	3652.7

Cultural Features None

Ground Cover Grass field

Weather 55F

Line Location and Bearing NS/EW



Electrical Resistivity Test Results
Summary Table
Seneca Wind Project
Seneca County, Ohio
35741003.00

Summary of Electrical Resistivity Test Results:

	Electrode "a" Spacing [feet]	GEO-005 Resistivity	GEO-006 Resistivity	GEO-015 Resistivity	GEO-038 Resistivity	GEO-087 Resistivity	GEO-089 Resistivity
		$\rho_a = K(V/I)$	$\rho_a = K(V/I)$	$\rho_a = K(V/I)$	$\rho_a = K(V/I)$	$\rho_a = K(V/I)$	$\rho_a = K(V/I)$
		Ohm-cm	Ohm-cm	Ohm-cm	Ohm-cm	Ohm-cm	Ohm-cm
N/S Orientation	2.5	2,921	3,592	3,760	3,304	4,262	6,352
	5	3,352	2,969	3,688	3,161	3,927	5,410
	10	3,481	2,866	3,753	2,970	4,023	5,875
	20	3,501	2,658	3,935	2,388	3,668	8,750
	40	3,561	2,818	4,747	2,367	4,202	16,183
E/W Orientation	2.5	3,161	3,735	4,369	3,688	4,060	8,015
	5	3,257	3,352	3,789	3,352	3,831	5,693
	10	3,474	2,856	3,716	2,897	4,023	5,781
	20	3,381	2,568	3,958	2,393	3,603	8,505
	40	3,514	2,770	4,838	2,246	4,207	15,562
Average:		3,360	3,019	4,055	2,877	3,981	8,613

Statistical Summary - Apparent Resistivity Averages (Ohm-cm)				
Electrode "a" Spacing [feet]	Max	Min	Mean (All Directions)	St Dev (All Directions)
2	8,015	2,921	4,268	1,469
5	5,693	2,969	3,815	863
10	5,875	2,856	3,810	1,036
20	8,750	2,388	4,109	2,188
40	16,183	2,246	5,585	4,884

Apparent Resistivity Average - Turbines - Ohm-cm:	4,317
--	--------------

Appendix F

Thermal Resistivity Report

May 24, 2018

Mr. Mike Goodwin
Construction Manager
sPower
2180 South 1300 East, Suite 600
Salt Lake City, Utah 84106

**Re: Soil Thermal Resistivity Testing Results
Seneca Wind Project
Seneca County, Ohio**

Dear Mr. Goodwin:

Barr Engineering Co. (Barr), under contract and authorization from sPower, collected soil samples from select locations at the Seneca Wind Project site in April of 2018. Thermal resistivity laboratory testing was completed on the samples in April and May of 2018. This letter presents the methods and results of the requested testing.

Methods

Six bulk soil samples were collected at select turbine locations and one bulk soil sample was collected the proposed substation location. Sample locations were selected by Barr with approval from sPower Barr prior to beginning the sample collection. Bulk samples were obtained from below the highly organic topsoil at depths of 1 to 3 feet below the ground surface. Bulk samples were placed in 5-gallon buckets and sealed in the field to preserve the in situ moisture content. The samples were transported to Soil Engineering Testing, Inc. (SET) of Bloomington, Minnesota, for laboratory testing.

Barr personnel verified the test sites based on the coordinates using a hand-held GPS unit, which was generally very close to the staked locations. The sample locations are shown on [Figure 1](#). Coordinates of each test location and sample depth can be found in [Table 1](#).

Table 1 Thermal Resistivity Sample Information

Geotech ID (Turbine ID-4/4/18 Array)	Sample Depth [feet]	UTM Coordinates, Zone 17N [m] [NAD83]	
		Northing	Easting
GEO-005 (2.3-5)	1-3	4550876.8	330232.5
GEO-006 (2.3-6)	1-3	4551577.3	334368.0
GEO-015 (3.8-3)	1-3	4546020.6	323462.6
GEO-038 (3.8-24)	1-3	4549899.6	339979.5
GEO-087 (3.8-16)	1-3	4547804.6	327652.3
GEO-089 (3.8-37)	1-3	4545293.9	328728.7
SUB-Res	1-3	4547035.0	327329.0

Laboratory testing included measurement of the soil's in-situ moisture content, standard Proctor density and optimum moisture content, and thermal dryout characteristics. All of the bulk samples collected were re-compacted to a density equivalent to approximately 90 percent of standard Proctor maximum dry density near the optimum moisture content for testing. The soil specimens were compacted in four layers in test molds (75 mm diameter by 150 mm high) to minimize contact resistance at the soil/probe interface and to provide for a uniform density.

Thermal dryout characteristics were measured according to ASTM D5334 using a laboratory type thermal needle held central and vertical in the base plate. Thermal resistivity measurements were conducted starting at the remolded moisture content of the soil sample to the totally dry condition. At the end of each drying stage, samples were sealed and brought to thermal equilibrium with the ambient air to maintain uniform moisture re-distribution through the sample. Tests were conducted using a KD2 Pro Thermal Property Analyzer. The instrument was calibrated prior to testing.

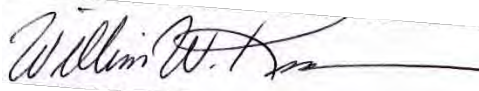
Results

The resulting laboratory soil thermal resistivity measurements presented with full dryout curves are included in the attachments.

Closing

Thank you for the opportunity to provide this service. Please call me at 952-832-2797 with questions or requests for additional information.

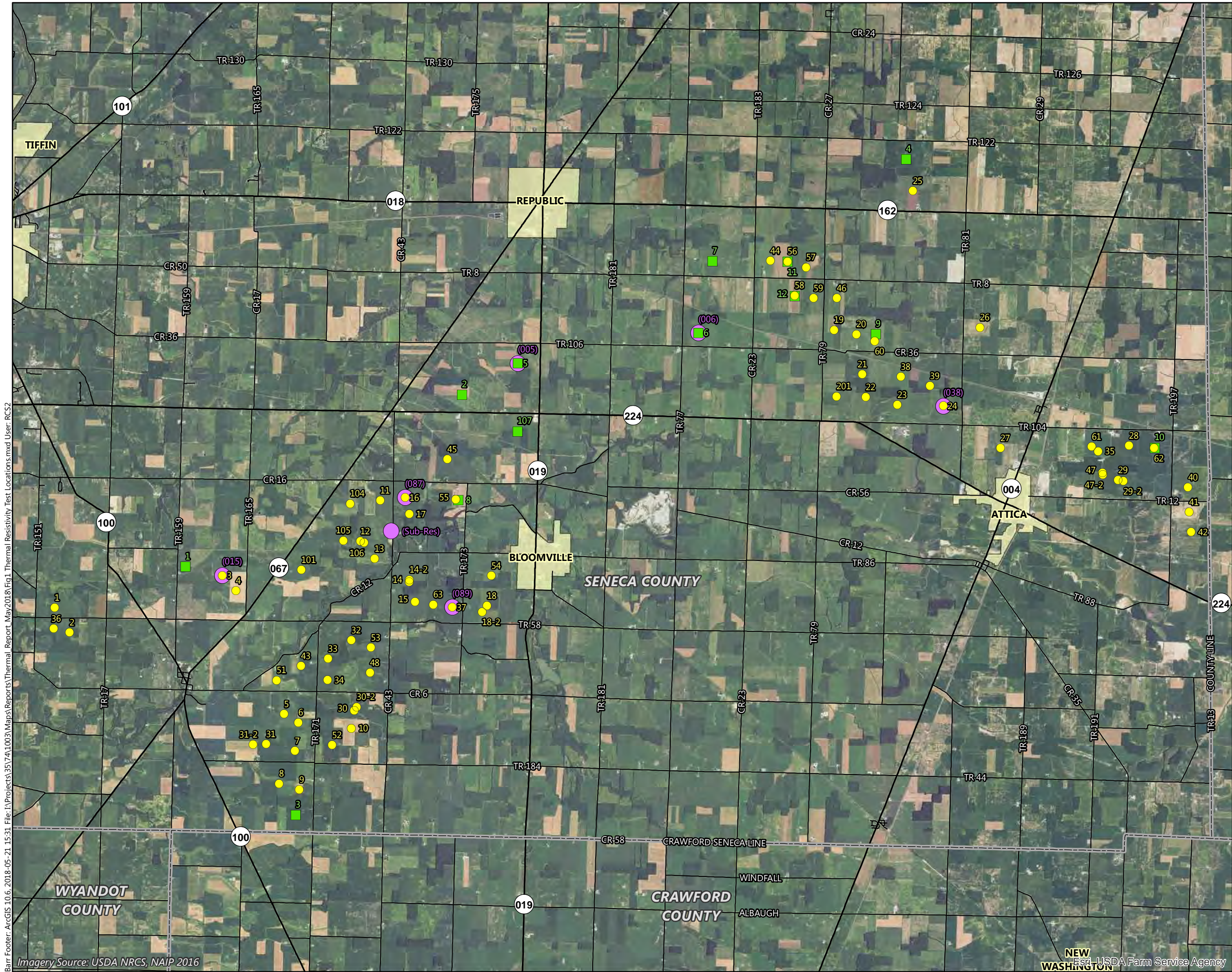
Sincerely,



William W. Kussmann
Barr Engineering Co.

Attachments:

Figure 1: Thermal Resistivity Test Locations
Thermal Resistivity Test Results



- Turbine Location - 3.8 (4/4/2018)
- Turbine Location - 2.3 (4/4/2018)
- City Boundary
- County Boundary
- Thermal Resistivity Test Locations

*Note: Labeled with GEO- ID in parentheses.

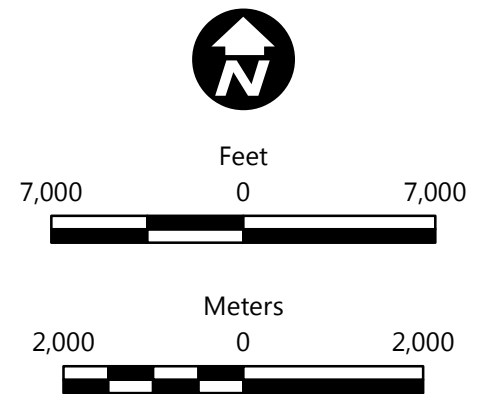


Figure 1

**THERMAL RESISTIVITY
TEST LOCATIONS**
Seneca Wind Project
S-Power
Seneca County, Ohio

Thermal Resistivity Report ASTM D:5334

Project: **Seneca Wind**

Job #: **11376**

Client: **Barr Engineering Company**

Date: **5/21/18**

Boring	Specimen Type	Depth (ft)	Type	Classification	Initial Conditions			Dry
					Dry Density (PCF)	WC (%)	Thermal Resistivity (°C-cm/W)	Thermal Resistivity (°C-cm/W)
Geo-005	Reconstituted	1-3	Bulk	Lean Clay (CL)	93.8	24.7%	62	171
Geo-006	Reconstituted	1-3	Bulk	Lean Clay (CL)	92.9	23.3%	66	175
Geo-015	Reconstituted	1-3	Bulk	Lean Clay (CL/CH)	92.3	23.7%	63	161
Geo-038	Reconstituted	1-3	Bulk	Lean Clay (CL)	92.6	24.5%	66	170
	Specimens reconstituted to approximately 90% of maximum standard proctor density near the greater of the as received or optimum moisture content.							

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<http://www.soilengineeringtesting.com>

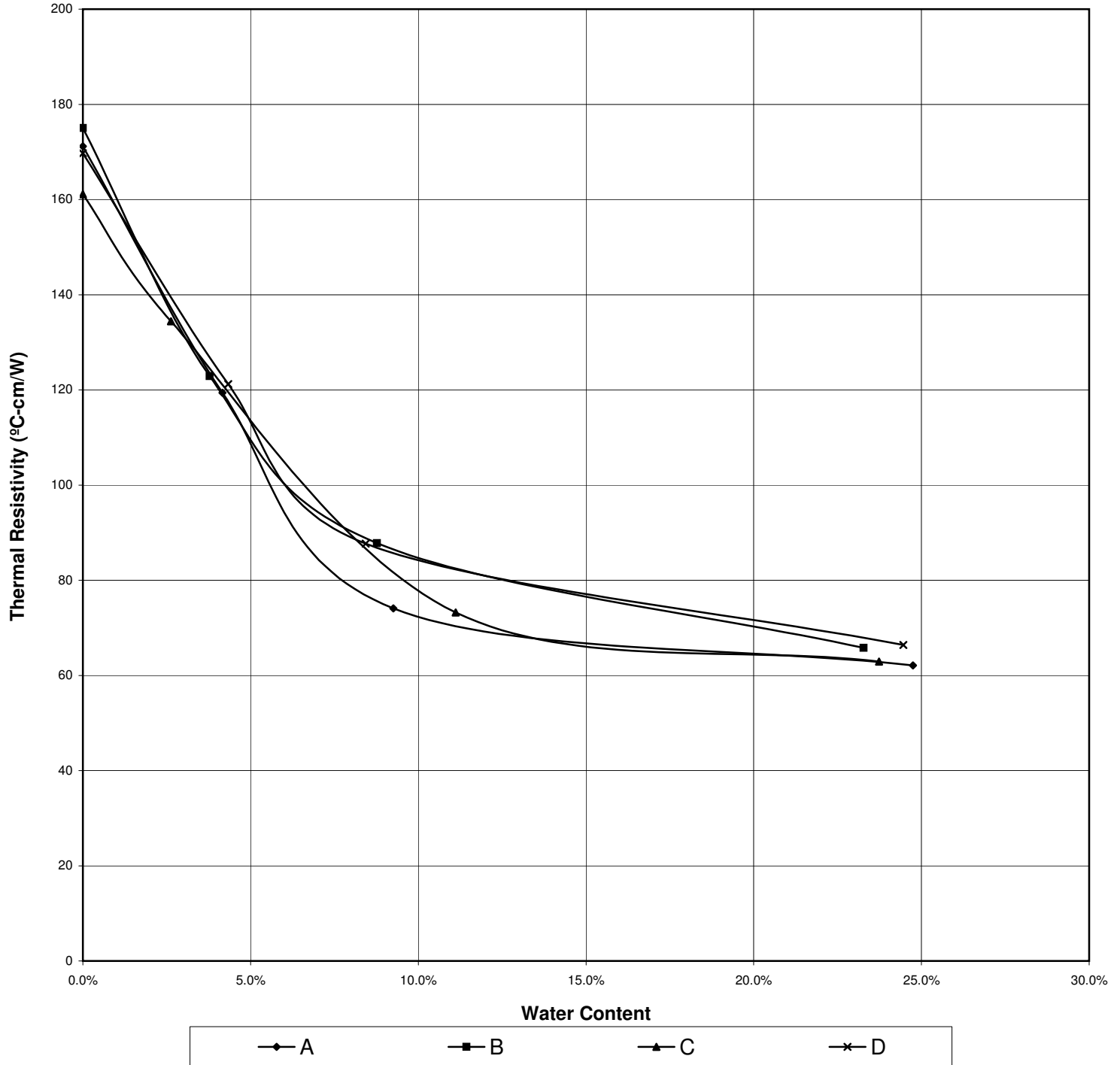
Thermal Resistivity Report ASTM D:5334

Project: Seneca Wind
 Client: Barr Engineering Company

Job: 11376
 Date: 5/21/18

	Boring	Depth (ft)
Specimen A:	Geo-005	1-3
Specimen B:	Geo-006	1-3
Specimen C:	Geo-015	1-3
Specimen D:	Geo-038	1-3

Thermal Dryout Curves (Water Content vs. Resistivity)



Thermal Resistivity Report ASTM D:5334

Project: **Seneca Wind**

Job #: **11376**

Client: **Barr Engineering Company**

Date: **5/21/18**

Boring	Specimen Type	Depth (ft)	Type	Classification	Initial Conditions			Dry
					Dry Density (PCF)	WC (%)	Thermal Resistivity (°C-cm/W)	Thermal Resistivity (°C-cm/W)
Geo-087	Reconstituted	1-3	Bulk	Lean Clay (CL/CH)	92.6	22.9%	74	195
Geo-089	Reconstituted	1-3	Bulk	Lean Clay with sand and a trace of gravel (CL)	94.7	21.9%	61	174
Sub-1	Reconstituted	1-3	Bulk	Lean Clay (CL/CH)	90.7	25.4%	68	180
	Specimens reconstituted to approximately 90% of maximum standard proctor density near the greater of the as received or optimum moisture content.							

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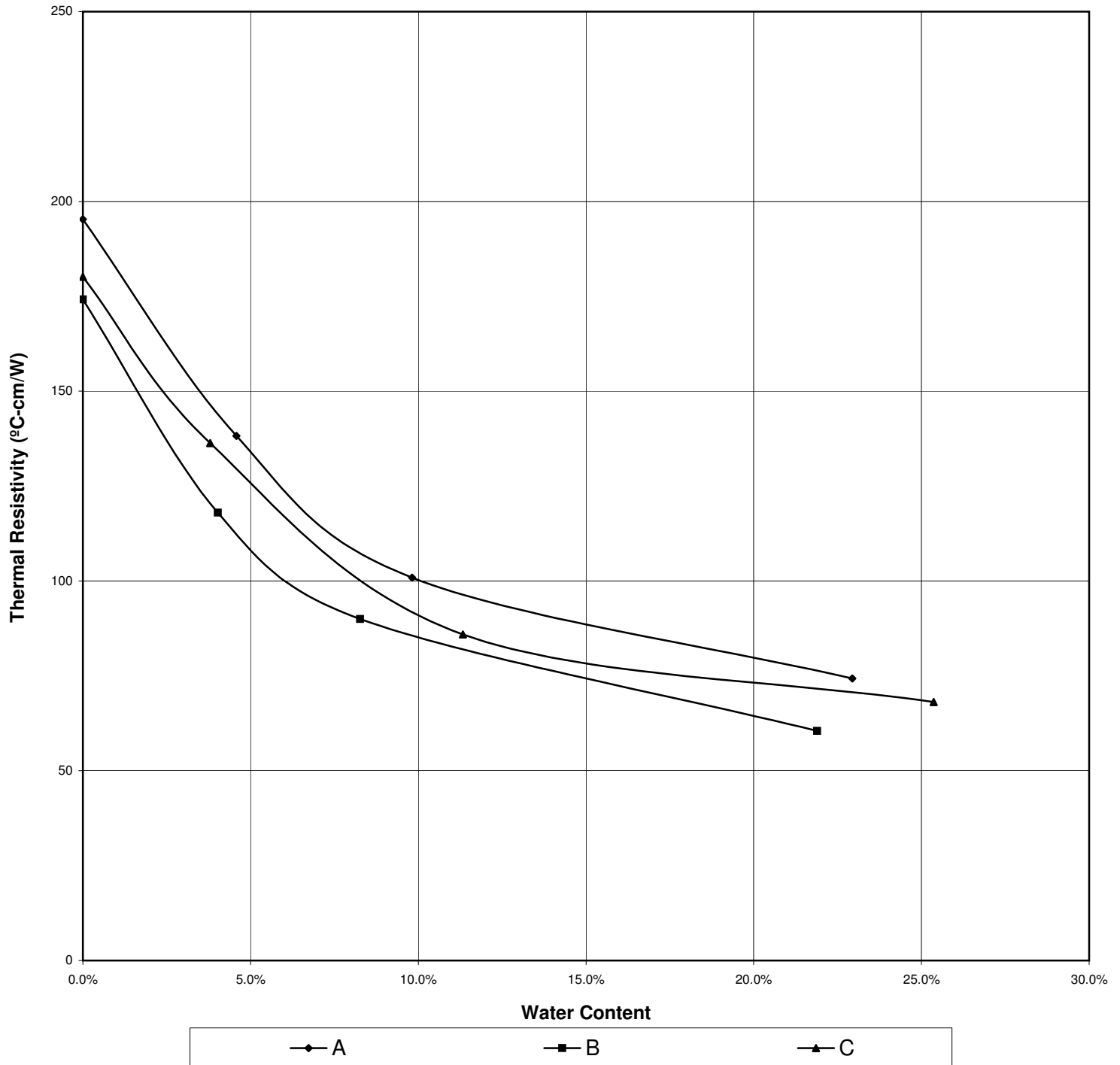
Thermal Resistivity Report ASTM D:5334

Project: Seneca Wind
Client: Barr Engineering Company

Job: 11376
Date: 5/21/18

	Boring	Depth (ft)
Specimen A:	Geo-087	1-3
Specimen B:	Geo-089	1-3
Specimen C:	Sub-1	1-3

Thermal Dryout Curves (Water Content vs. Resistivity)



Moisture Density Curve ASTM: D698, Method B

Project: **Seneca Wind**

Date: **5/4/18**

Client: **Barr Engineering Company**

Job No. **11376**

Boring No. **Geo-005**

Sample:

Depth(ft): **1-3**

Location:

Soil Type: **Lean Clay (CL)**

As Received W.C. (%): **24.9**

LL:

PL:

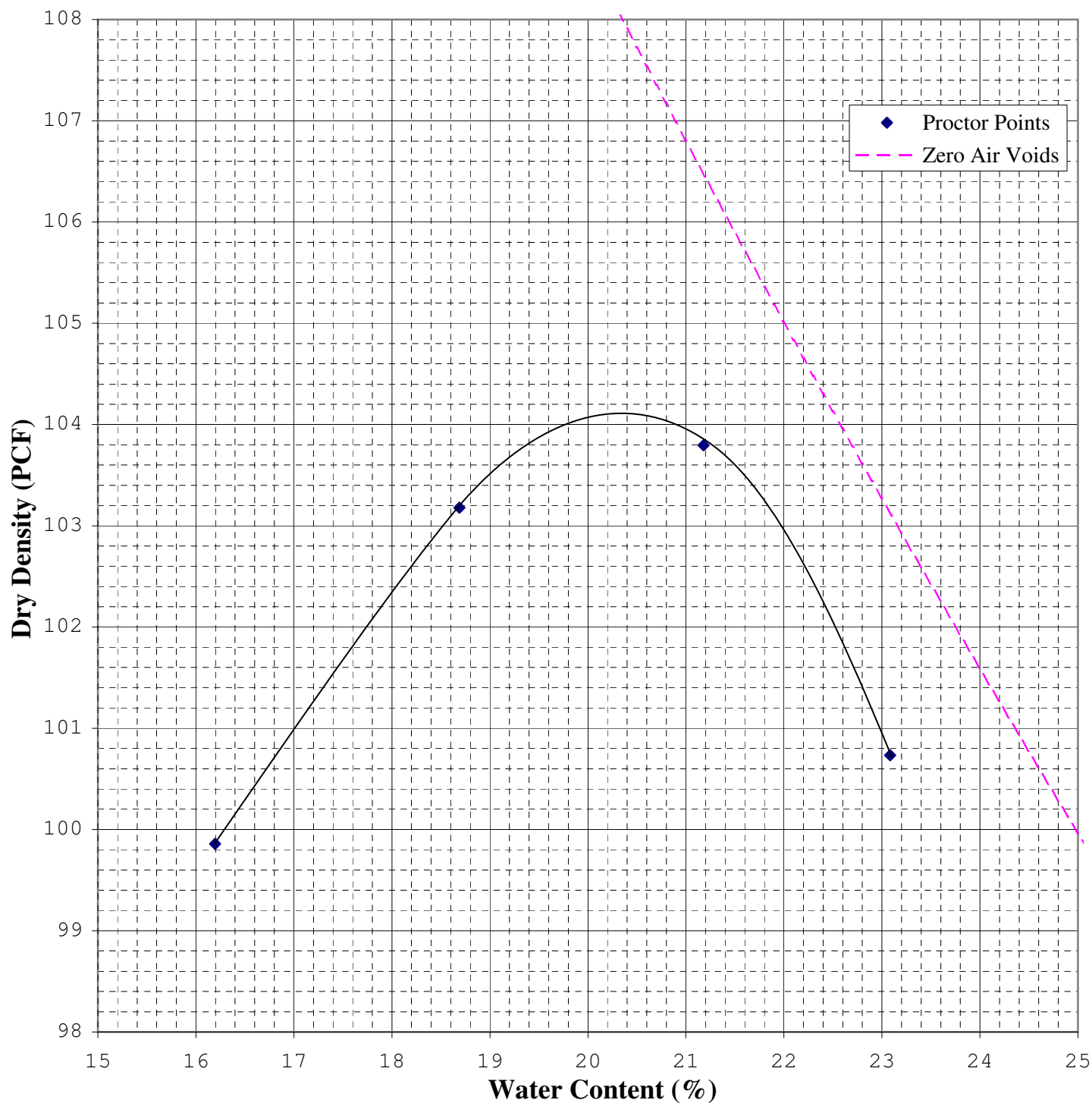
PI:

Specific Gravity: **2.67**

*Assumed

Maximum Dry Density (pcf): **104.1**

Opt. Water Content (%): **20.4**



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Moisture Density Curve ASTM: D698, Method B

Project: **Seneca Wind**

Date: **5/4/18**

Client: **Barr Engineering Company**

Job No. **11376**

Boring No. **Geo-006**

Sample:

Depth(ft): **1-3**

Location:

Soil Type: **Lean Clay (CL)**

As Received W.C. (%): **23.4**

LL:

PL:

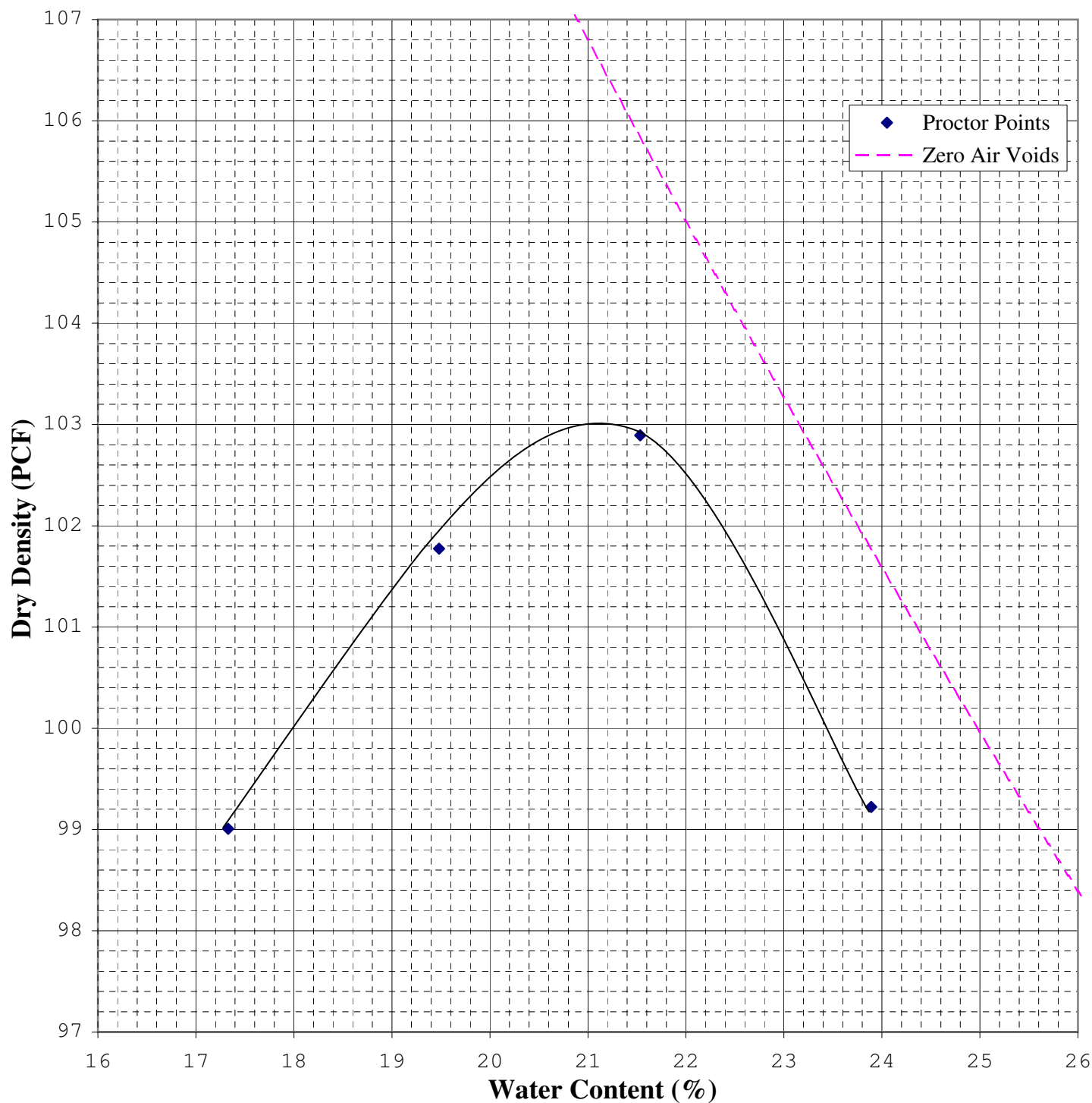
PI:

Specific Gravity: **2.67**

*Assumed

Maximum Dry Density (pcf): **103.0**

Opt. Water Content (%): **21.1**



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Moisture Density Curve ASTM: D698, Method B

Project: **Seneca Wind**

Date: **5/4/18**

Client: **Barr Engineering Company**

Job No. **11376**

Boring No. **Geo-015**

Sample:

Depth(ft): **1-3**

Location:

Soil Type: **Lean Clay (CL/CH)**

As Received W.C. (%): **23.9**

LL:

PL:

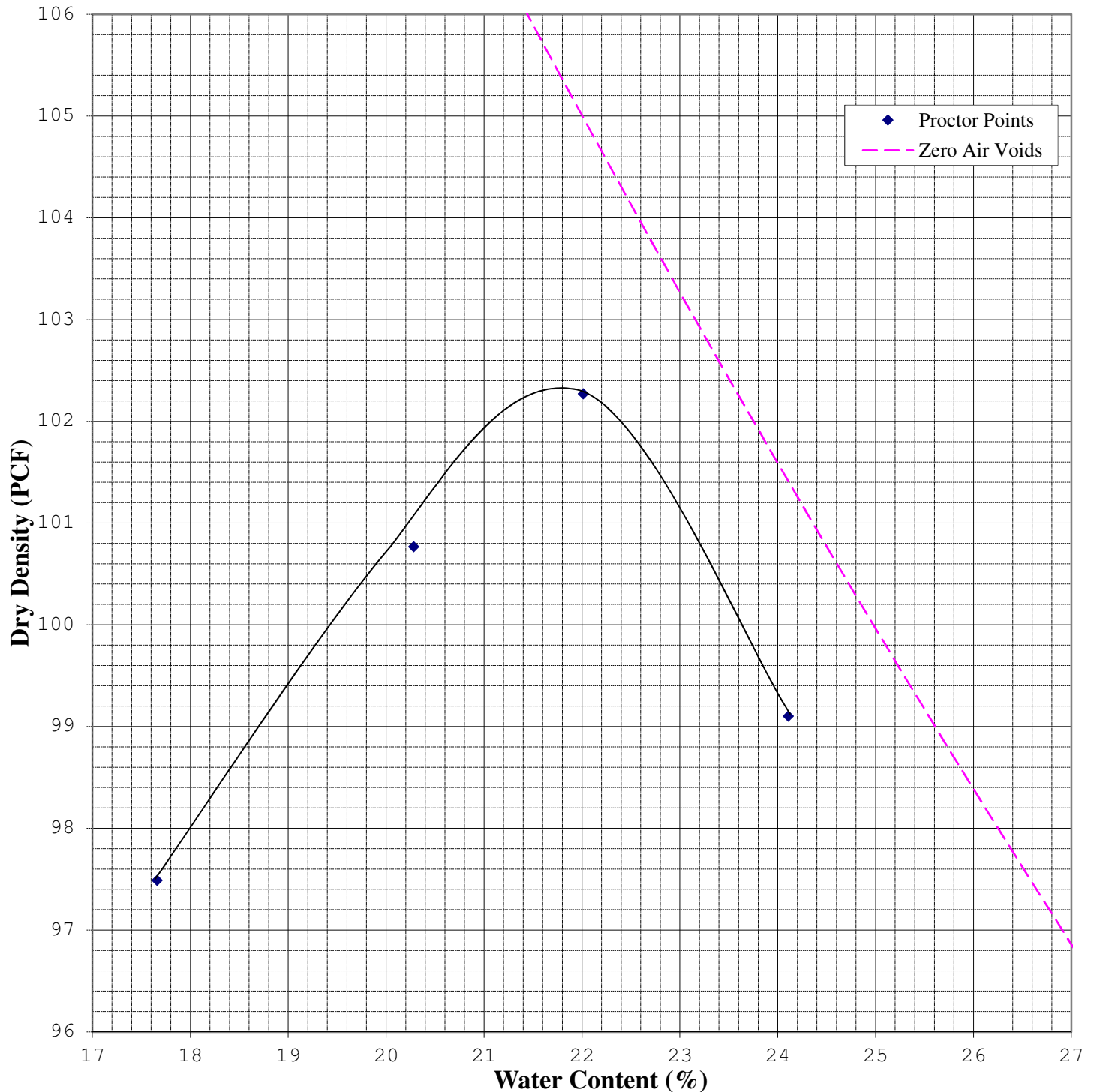
PI:

Specific Gravity: **2.67**

*Assumed

Maximum Dry Density (pcf): **102.3**

Opt. Water Content (%): **21.8**



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Moisture Density Curve ASTM: D698, Method B

Project: **Seneca Wind**

Date: **5/6/18**

Client: **Barr Engineering Company**

Job No. **11376**

Boring No. **Geo-038**

Sample:

Depth(ft): **1-3**

Location:

Soil Type: **Lean Clay (CL)**

As Received W.C. (%): **24.8**

LL:

PL:

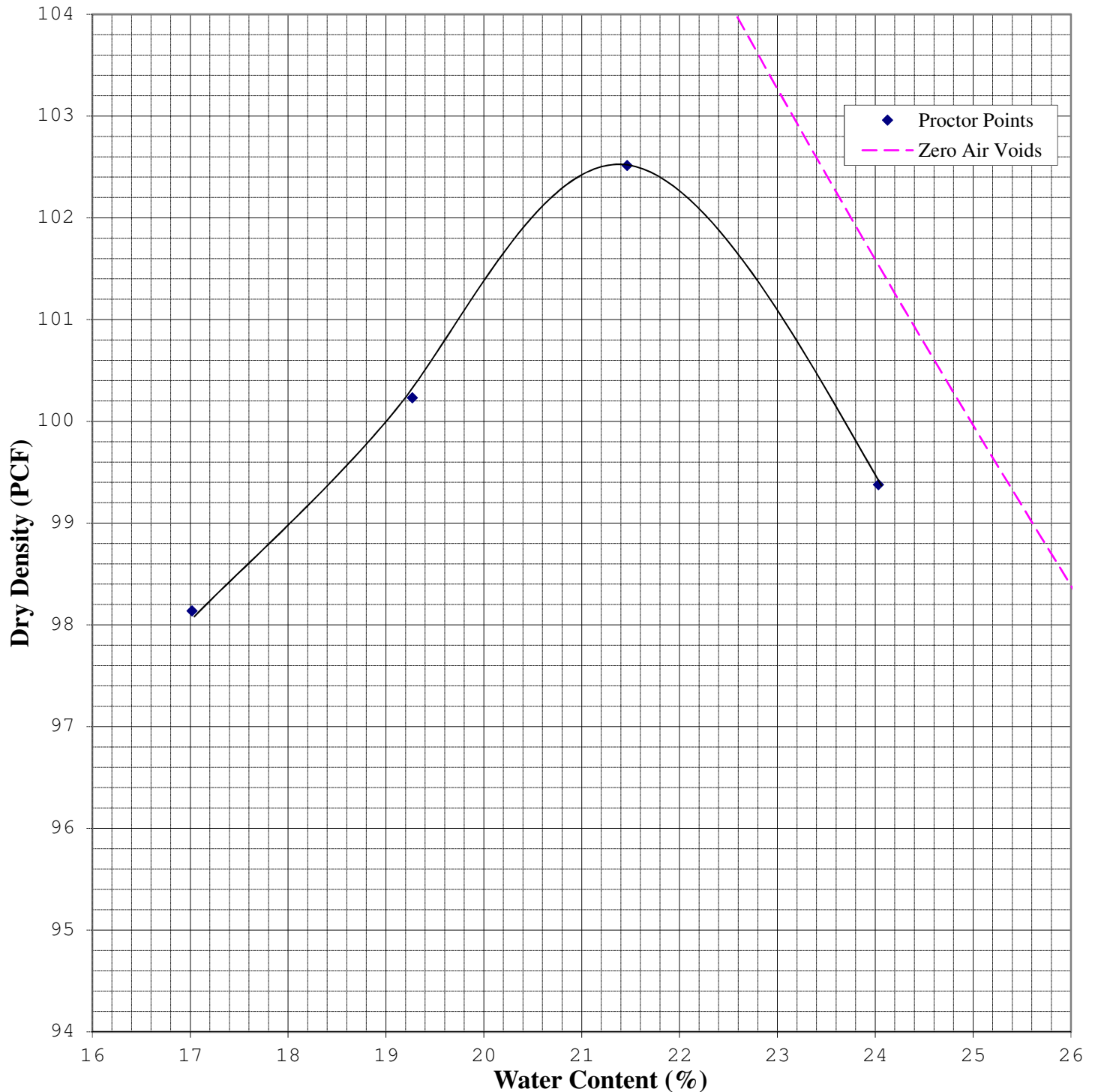
PI:

Specific Gravity: **2.67**

*Assumed

Maximum Dry Density (pcf): **102.5**

Opt. Water Content (%): **21.4**



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Moisture Density Curve ASTM: D698, Method B

Project: **Seneca Wind**

Date: **5/4/18**

Client: **Barr Engineering Company**

Job No. **11376**

Boring No. **Geo-087**

Sample:

Depth(ft): **1-3**

Location:

Soil Type: **Lean Clay (CL/CH)**

As Received W.C. (%): **23.1**

LL:

PL:

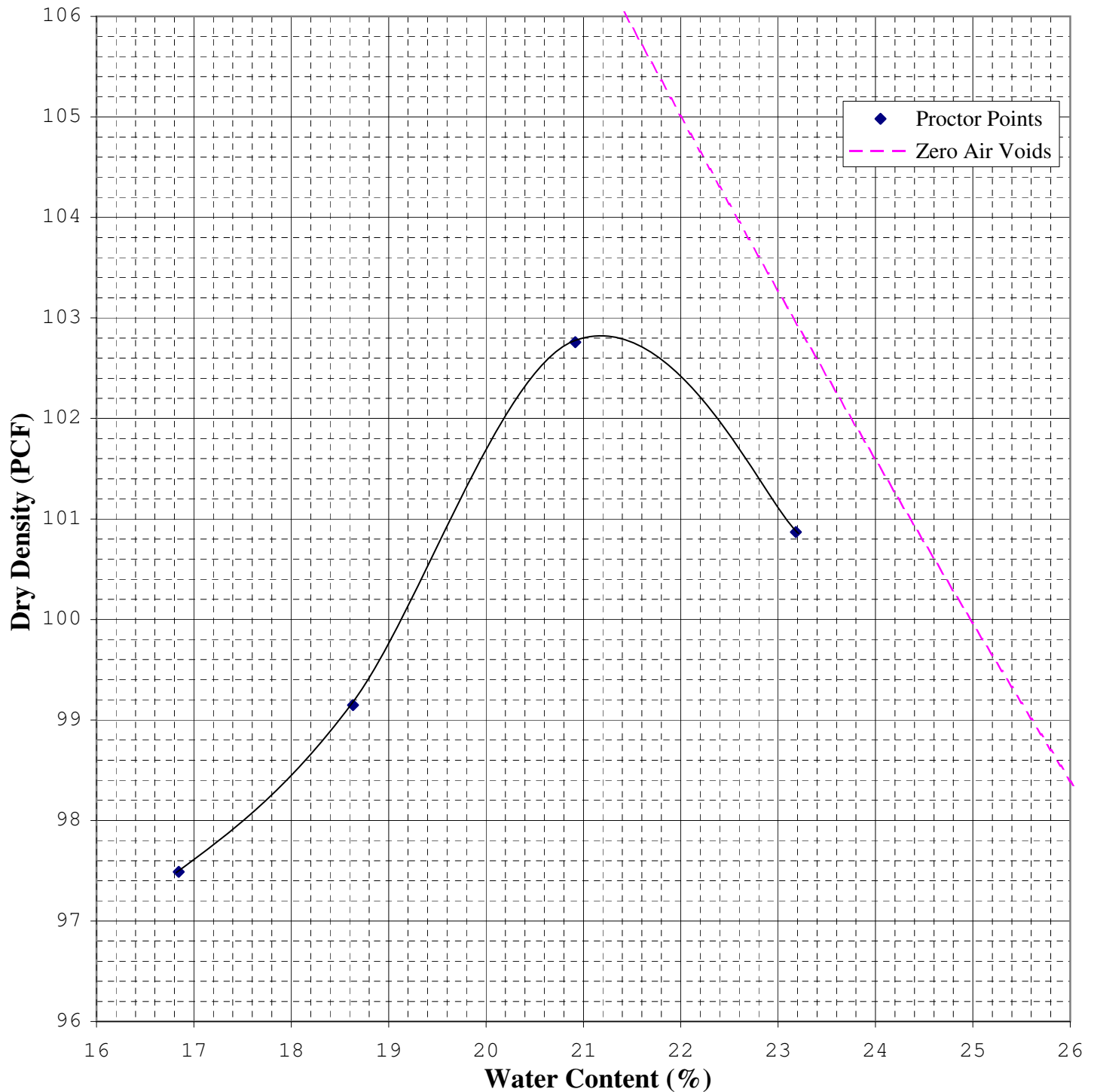
PI:

Specific Gravity: **2.67**

*Assumed

Maximum Dry Density (pcf): **102.8**

Opt. Water Content (%): **21.2**



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Moisture Density Curve ASTM: D698, Method B

Project: **Seneca Wind**

Date: **5/4/18**

Client: **Barr Engineering Company**

Job No. **11376**

Boring No. **Geo-089**

Sample:

Depth(ft): **1-3**

Location:

Soil Type: **Lean Clay w/sand and a trace of gravel (CL)**

As Received W.C. (%): **22.2**

LL:

PL:

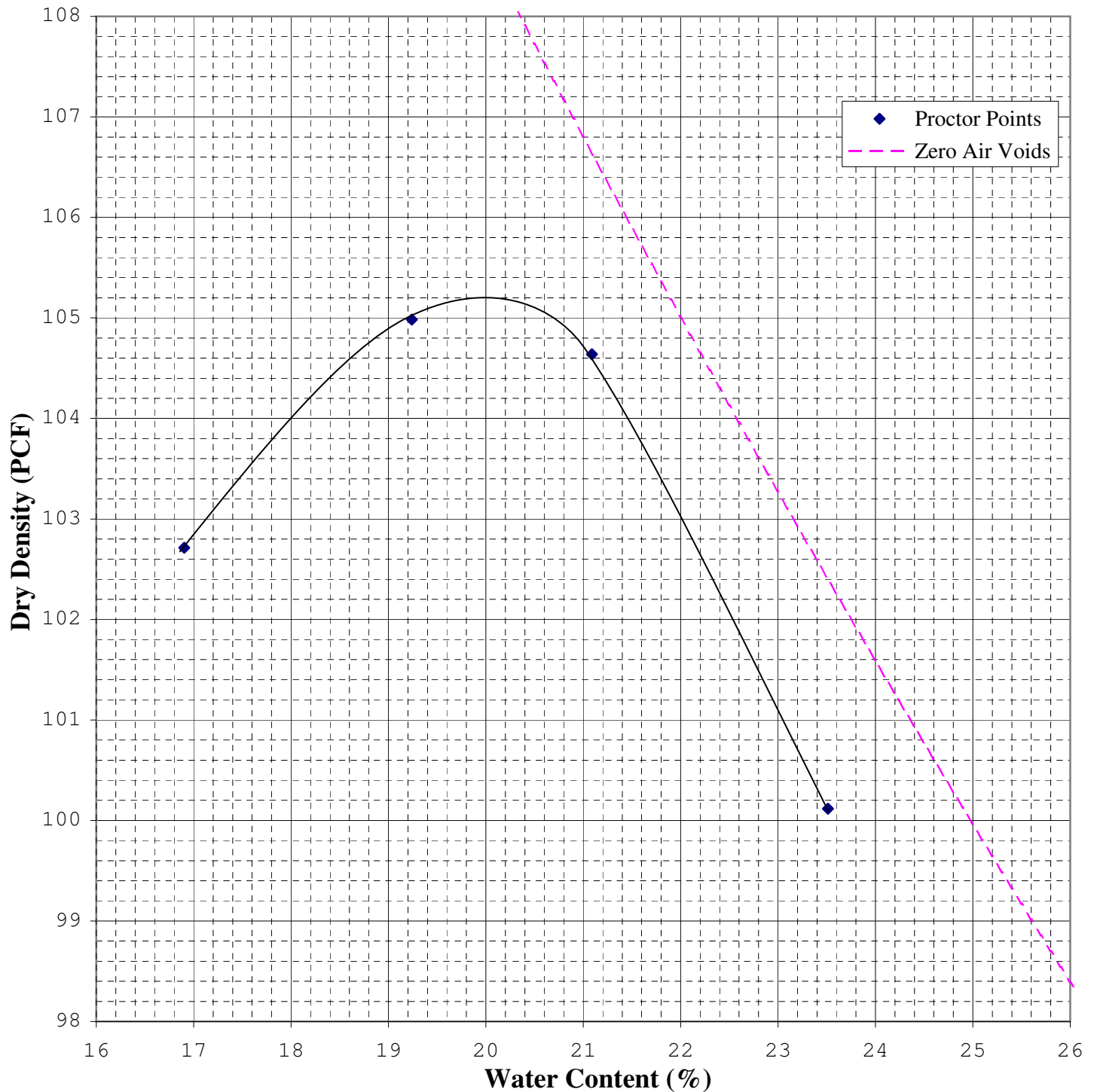
PI:

Specific Gravity: **2.67**

*Assumed

Maximum Dry Density (pcf): **105.0**

Opt. Water Content (%): **20.0**



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Moisture Density Curve ASTM: D698, Method B

Project: **Seneca Wind**

Date: **5/6/18**

Client: **Barr Engineering Company**

Job No. **11376**

Boring No. **Sub-1**

Sample:

Depth(ft): **1-3**

Location:

Soil Type: **Lean Clay (CL/CH)**

As Received W.C. (%): **25.8**

LL:

PL:

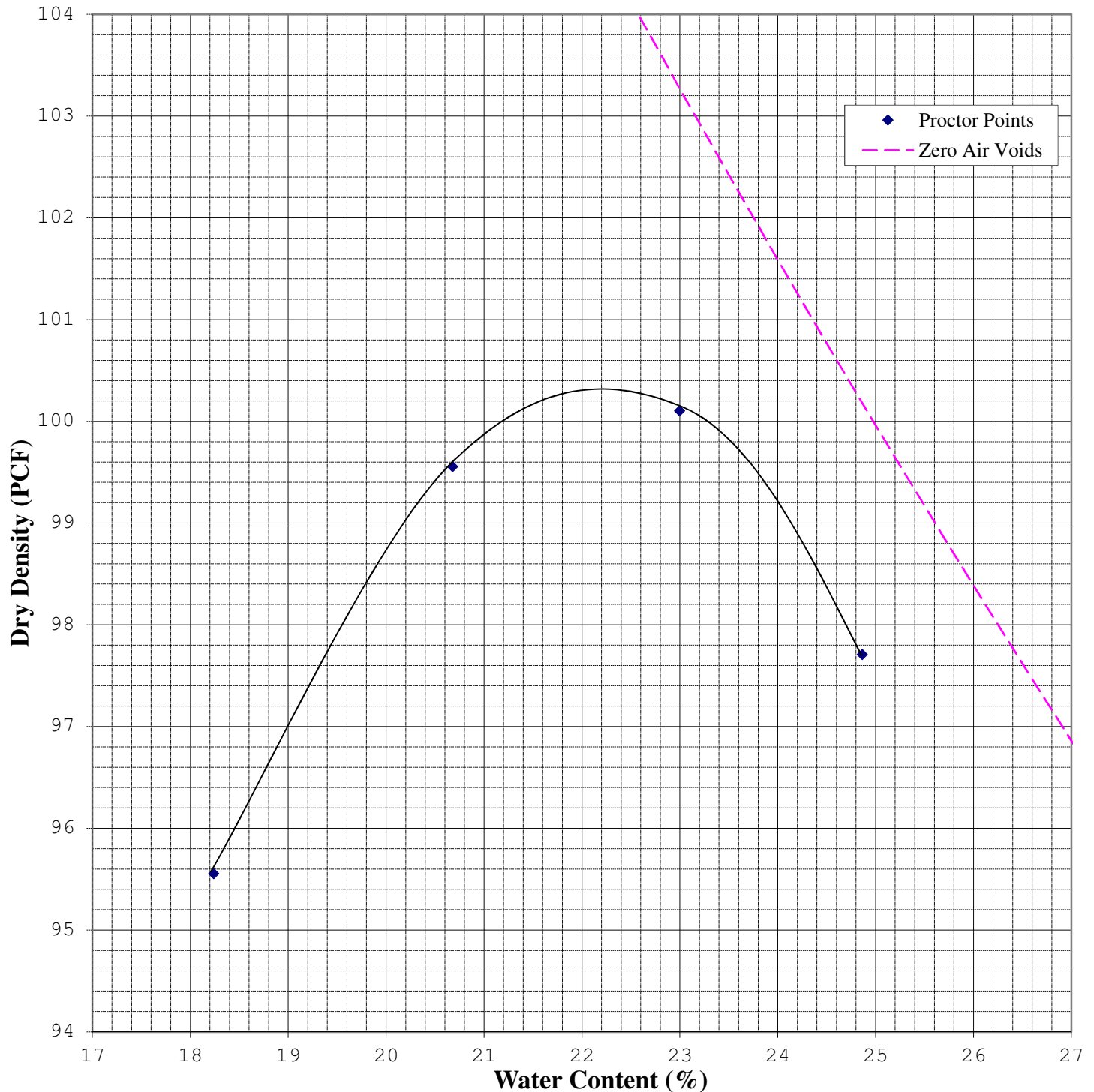
PI:

Specific Gravity: **2.67**

*Assumed

Maximum Dry Density (pcf): **100.3**

Opt. Water Content (%): **22.2**



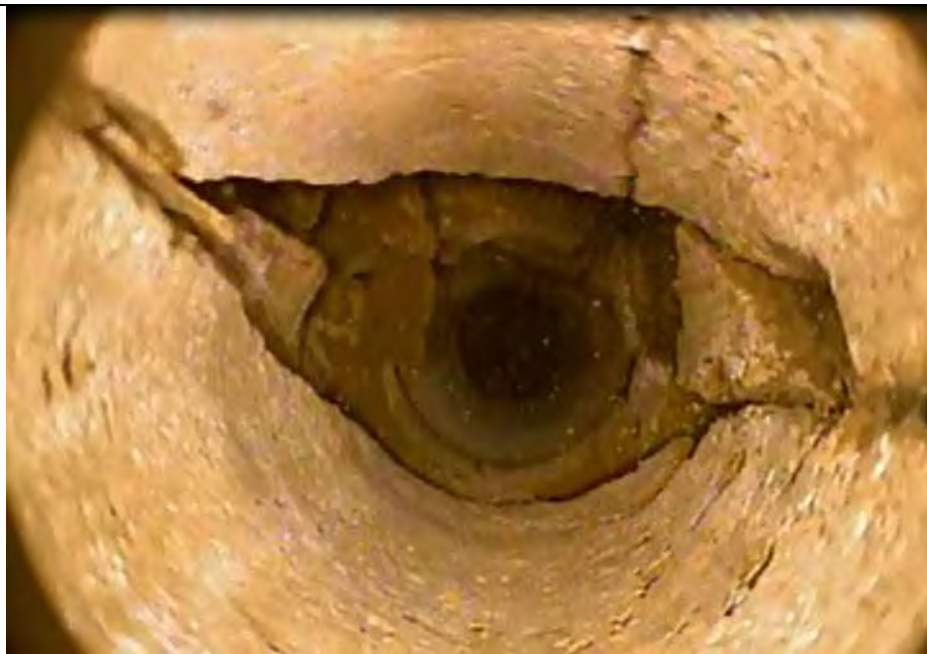
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Appendix G

Photographs of Downhole Video Footage



GEO-021, 7:10

28 ft, Very broken rock with missing pieces, has back



GEO-021, 7:52

30 ft, View of void observed during drilling, collapsed borehole



GEO-021, 10:21

31.5 ft, View of tubular opening, camera could not see end of tube

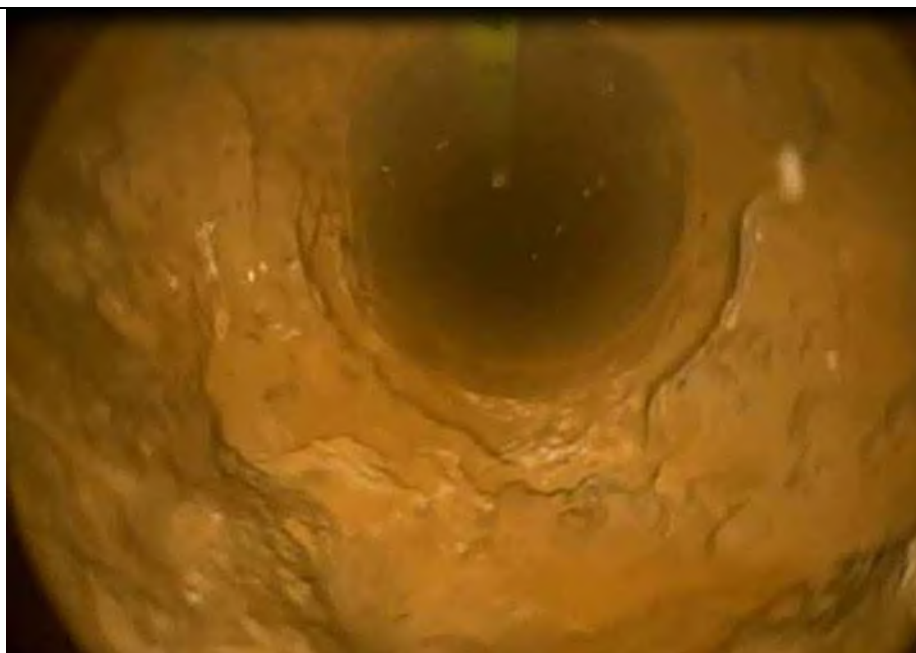


GEO-022, 2:16

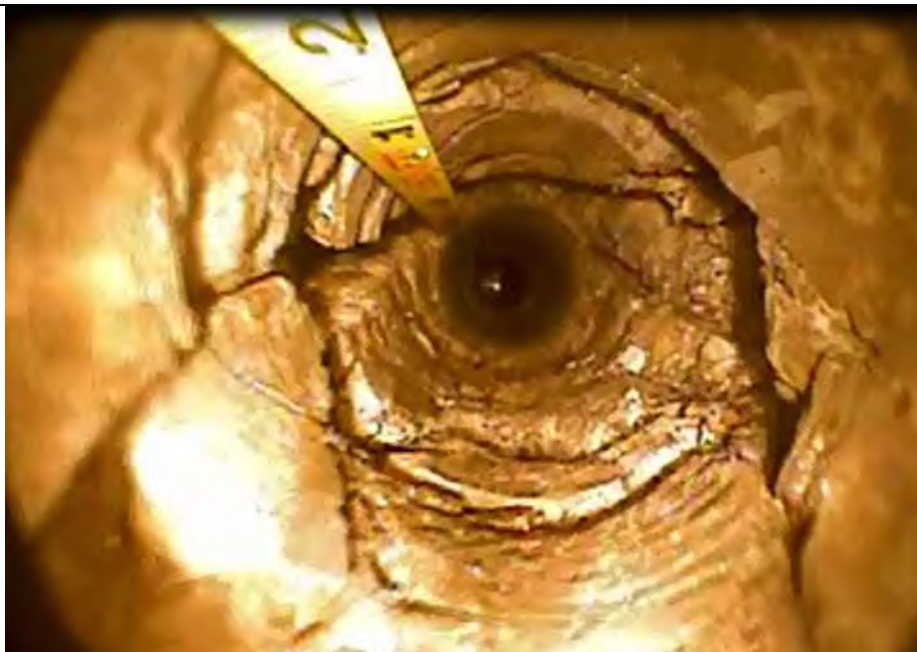
26 ft, Large washout, had back



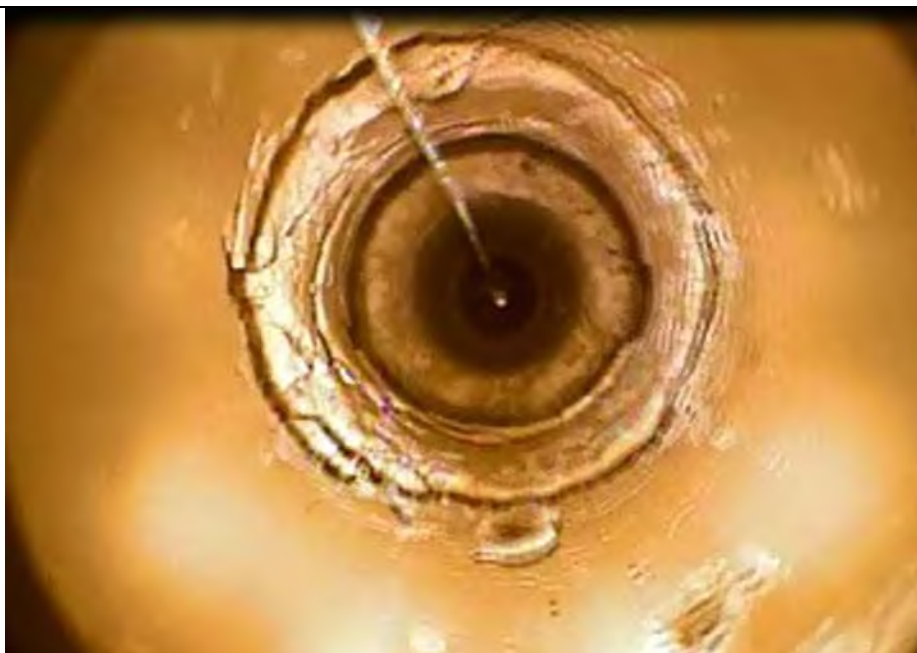
GEO-022, 3:14
37 ft, Large washout, had back



GEO-022, 4:31
40 ft, Large washout, view of sidewalls with back



GEO-028, 2:25
9.5 ft, Where water loss occurred. Highly fractured rock.



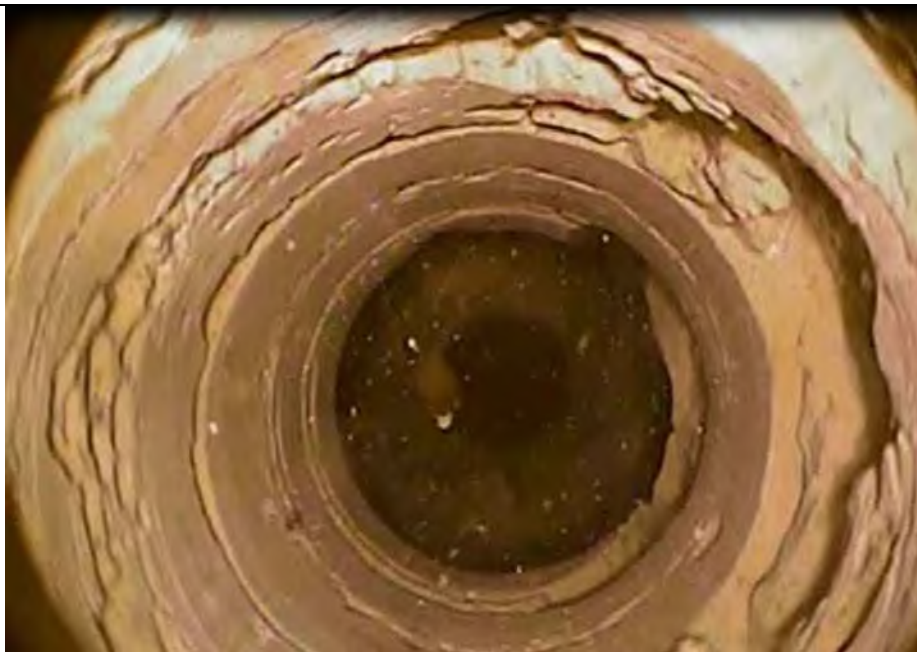
GEO-028, 3:36
15.5 ft, 3-inch opening, lots of gravel sized pieces within



GEO-008, 6:28
33 ft, View of thin opening in sidewall, had back



GEO-047, 4:20
Typical borehole in upper 40 feet. Fractures and small openings have backs



GEO-047, 13:05
58 ft, view of “void” noted on log



GEO-047, 13:50
58 ft, view of sidewall of “void”, rugged dissolution features, had back



GEO-048, 5:53

26 ft, typical open fracture, had back, several observed to 35 ft in this location



GEO-048, 13:44

45-46 ft, rugged borehole.



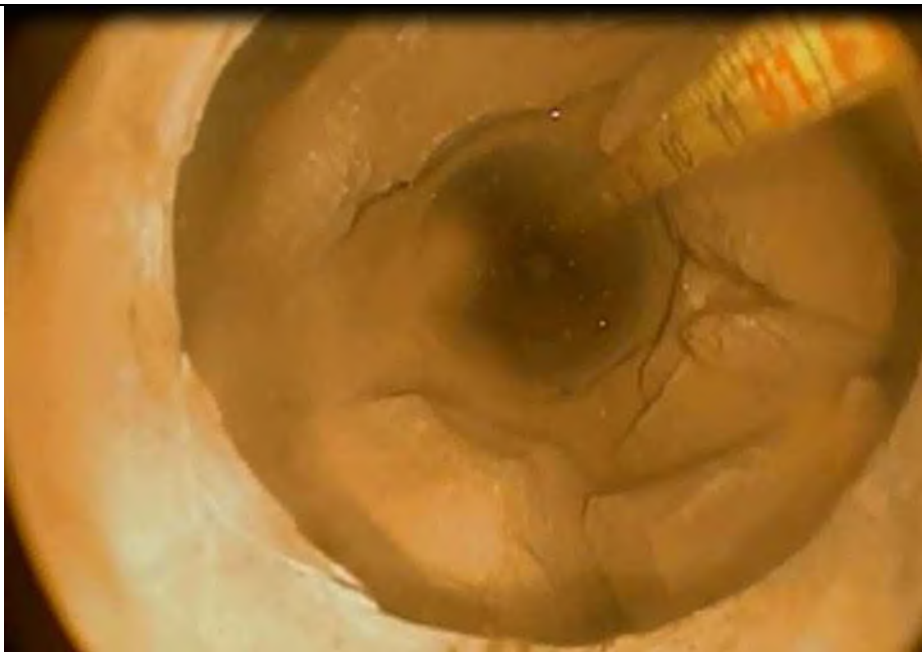
GEO-048, 19:37

58.5 ft, washout observed near depth where geotechnical log indicated, collection of rock debris at base of hole, possible small “void”

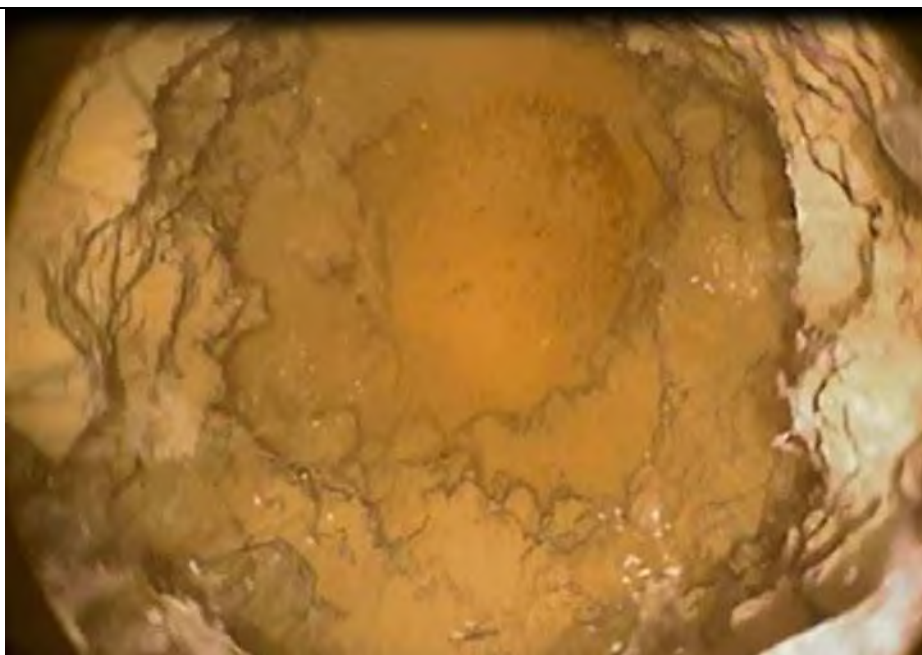


GEO-060, 2:20

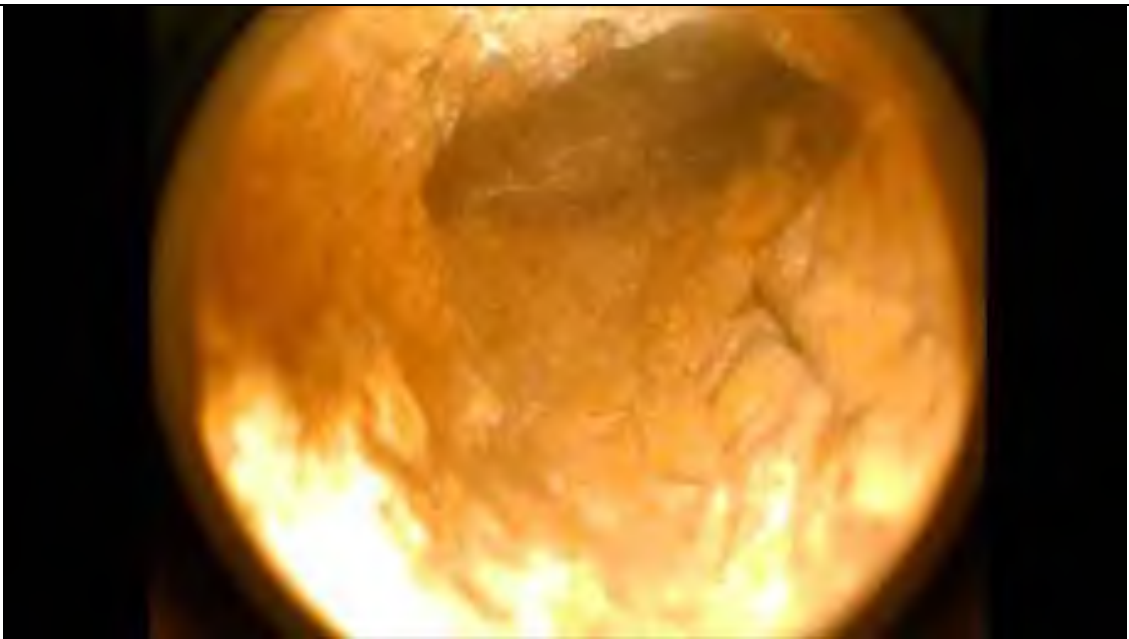
15 ft, View of thin opening, note soil on bottom of opening, likely thin soil filled void



GEO-060, 6:43
25 ft, Collection of platy rock debris



GEO-060, 12:56
58 ft, view of bottom of borehole where “voids” were noted, rugged dissolution features, had back



GEO-066, 7:27

31.5 ft, smaller void, 1" high, 2-3" deep. May be open on right side



GEO-066, 4:35

19.5 ft, typical opening ranging from 2 to 4 inches, frequently observed in video from 10 to 40 feet



GEO-066, 23:22
19.5 ft, typical view looking in to openings, had back



GEO-066, 8:16
31 ft, "void" identified in geotechnical boring



GEO-066, 12:28

42 ft, rugged borehole with large opening and missing pieces, dissolution features



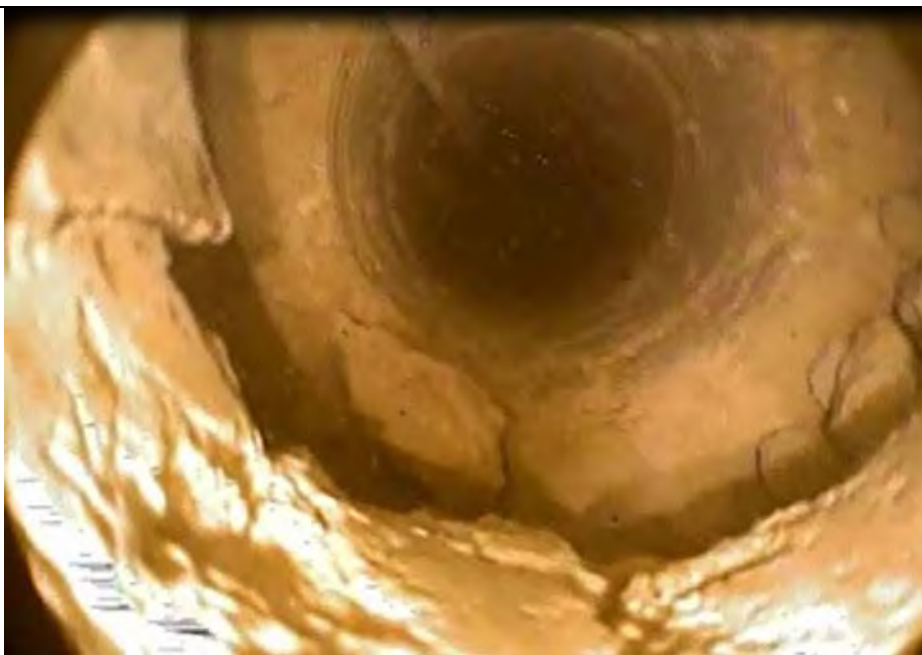
GEO-066, 20:35

43 ft, view of sidewall of opening, had back



GEO-078, 6:41

36 ft, where circulation was lost during coring, too narrow to see into opening



GEO-078, 4:17

18 ft, small opening with soil and gravel infill and missing pieces, had back



GEO-086, 2:04
11.5 ft, 2-inch opening

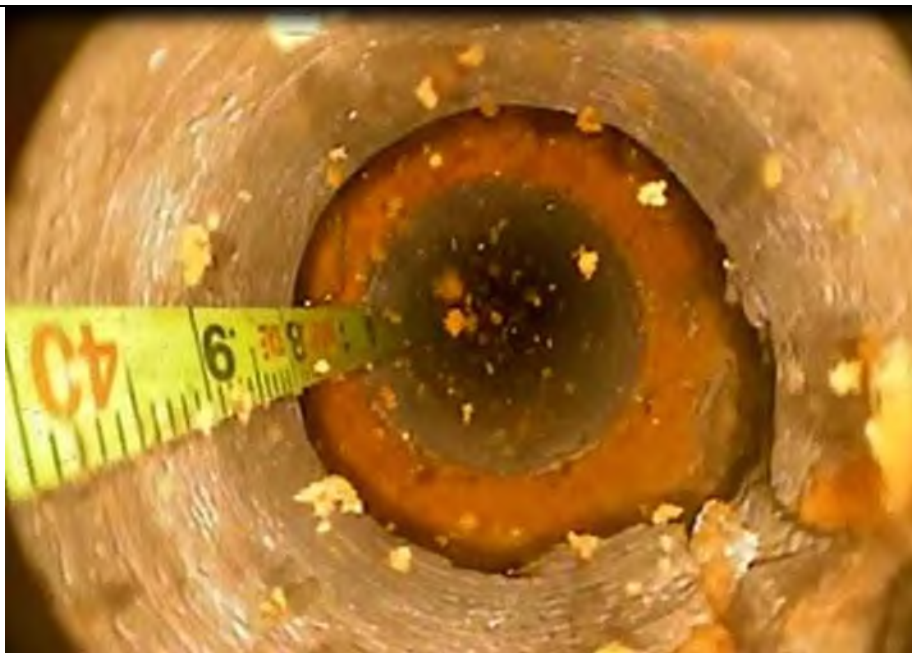


GEO-086, 3:21
15.5 ft, highly fractured, rugged borehole with pieces missing, near depth where drilling fluid circulation was lost



GEO-086, 9:46

38 ft, small opening, similar observed at 14 and 25 feet



GEO-096, 6:51

20.5 ft, small opening, similar observed at 16.5, 26, 28, 32.5, 43, 43.5, 44.5, 48, and 49.5 ft



GEO-096, 7:04
20.5 ft, view into opening, had back



GEO-096, 14:28
42.5 ft, near where drilling fluid circulation was lost, small opening with some missing limestone pieces

Appendix H

Further Analysis of Lower Strength Zones

Appendix H: Further Analysis of Lower Strength Zones

Further analysis of the following turbine locations was performed.

1.1.1 GEO-007

A slightly lower strength zone was identified from about 25 to 26.5 feet in the soil boring completed for investigated location GEO-007 through SPT testing. Further review of the soil boring indicates that the other SPT tests indicated that the undrained shear strength met the design value. The results of a laboratory unconfined compressive strength test indicated that the undrained shear strength just below the lower strength layer was approximately 2,800 psf, which meets the design value. The lower strength zone is anticipated to be relatively thin, and is not likely to lead to a bearing capacity failure. Therefore, no soil remediation is recommended at this location.

1.1.2 GEO-008

A slightly lower strength zone was identified from about 25 to 26.5 feet in the soil boring completed for investigated location GEO-008 through hand penetrometer testing. Further review of the soil boring indicates that the other SPT and hand penetrometer tests indicated that the undrained shear strength met the design value. The results of a laboratory unconfined compressive strength test indicated that the undrained shear strength just above the lower strength layer was approximately 2,200 psf, which meets the design value. The lower strength zone is anticipated to be relatively thin, and other measures of shear strength indicate that the strength will meet the design value, and is not likely to lead to a bearing capacity failure. Therefore, no soil remediation is recommended at this location.

1.1.3 GEO-009

A slightly lower strength zone was identified from about 30 to 31.5 feet in the soil boring completed for investigated location GEO-009 through hand penetrometer testing. Further review of the soil boring indicates that the other SPT and hand penetrometer tests indicated that the undrained shear strength met the design value. The results of a laboratory unconfined compressive strength test indicated that the undrained shear strength just above the lower strength layer was approximately 2,400 psf, which meets the design value. The lower strength zone is anticipated to be relatively thin, and other measures of shear strength indicate that the strength will meet the design value, and is not likely to lead to a bearing capacity failure. Therefore, no soil remediation is recommended at this location.

1.1.4 GEO-036

A slightly lower strength zone was identified from about 20 to 21.5 feet in the soil boring completed for investigated location GEO-036 through hand penetrometer testing. Further review of the soil boring indicates that the other SPT and hand penetrometer tests indicated that the undrained shear strength met the design value. The results of a laboratory unconfined compressive strength test indicated that the undrained shear strength just below the lower strength layer was approximately 2,100 psf, which meets the design value. The lower strength zone is anticipated to be relatively thin, and other measures of shear strength indicate that the strength will meet the design value, and is not likely to lead to a bearing capacity failure. Therefore, no soil remediation is recommended at this location.

1.1.5 GEO-042

A slightly lower strength zone was identified from about 30 to 37 feet in the soil boring completed for investigated location GEO-042 through SPT testing. Further review of the soil boring indicates that the other SPT tests indicated that the undrained shear strength met the design value. The results of a laboratory unconfined compressive strength test indicated that the undrained shear strength within the lower strength layer was approximately 1,800 psf, which meets the design value. The lower strength zone is anticipated to be relatively thin, and is not likely to lead to a bearing capacity failure. Therefore, no soil remediation is recommended at this location.

1.1.6 GEO-047

A slightly lower strength zone was identified from about 8 to 20.5 feet in the soil boring completed for investigated location GEO-047 through SPT testing. The results of the soil boring indicate that the sand in that interval is relatively loose, and may be subject to excessive settlement. Barr performed a settlement analysis, and the results indicate that the estimated settlement will be less than the typical threshold for industry standards. Therefore, no soil remediation is recommended at this location.

1.1.7 GEO-052

A slightly lower strength zone was identified from about 20 to 35 feet and 55 to 56.5 feet in the soil boring completed for investigated location GEO-052 through SPT testing. Further review of the soil boring indicates that the other SPT tests indicated that the undrained shear strength met the design value. The results of a laboratory unconfined compressive strength test indicated that the undrained shear strength within the lower strength layer was approximately 2,300 psf, which meets the design value. The lower strength zone at 55 feet is anticipated to be relatively thin and located well below the embedment depth of the foundation, and is not likely to lead to a bearing capacity failure. Therefore, no soil remediation is recommended at this location.

1.1.8 GEO-056

A slightly lower strength zone was identified from about 25 to 35 feet and 40 to 41.5 feet in the soil boring completed for investigated location GEO-056 through SPT testing and hand penetrometer testing. Further review of the soil boring indicates that the other SPT and hand penetrometer tests indicated that the undrained shear strength met the design value. The results of laboratory unconfined compressive strength tests indicated that the undrained shear strength within the lower strength layers ranged from approximately 1,700 to 2,900 psf, which meets the design value. Based on an overall consideration of the field and laboratory test results, no soil remediation is recommended at this location.

1.1.9 GEO-059

A slightly lower strength zone was identified from about 45 to 46.5 feet and 60 to 61.5 feet in the soil boring completed for investigated location GEO-059 through SPT testing. Further review of the soil boring indicates that the other SPT and hand penetrometer tests indicated that the undrained shear strength met the design value. The results of laboratory unconfined compressive strength tests indicated that the undrained shear strength within the lower strength layer was approximately 4,000 psf, which meets the

design value. The lower strength zones are anticipated to be relatively thin and located well below the embedment depth of the foundation, and is not likely to lead to a bearing capacity failure. Therefore, no soil remediation is recommended at this location.

1.1.10 GEO-062

A slightly lower strength zone was identified from about 40 to 41.5 feet in the soil boring completed for investigated location GEO-062 through hand penetrometer testing. Further review of the soil boring indicates that the other SPT and hand penetrometer tests indicated that the undrained shear strength met the design value. The lower strength zone is anticipated to be relatively thin, and other measures of shear strength indicate that the strength will meet the design value, and is not likely to lead to a bearing capacity failure. Therefore, no soil remediation is recommended at this location.

1.1.11 GEO-070

A slightly lower strength zone was identified from about 30 to 36.5 feet in the soil boring completed for investigated location GEO-070 through hand penetrometer testing. Further review of the soil boring indicates that the other SPT and hand penetrometer tests indicated that the undrained shear strength met the design value. The results of a laboratory unconfined compressive strength test indicated that the undrained shear strength just above the lower strength layer was approximately 2,200 psf, which meets the design value. The lower strength zone is anticipated to be relatively thin, and other measures of shear strength indicate that the strength will meet the design value, and is not likely to lead to a bearing capacity failure. Therefore, no soil remediation is recommended at this location.

1.1.12 GEO-077

A slightly lower strength zone was identified from about 20 to 21.5 feet and 40 to 41.5 feet in the soil boring completed for investigated location GEO-077 through SPT and hand penetrometer testing. Further review of the soil boring indicates that the other SPT and hand penetrometer tests indicated that the undrained shear strength met the design value. The results of a laboratory unconfined compressive strength test indicated that the undrained shear strength just below the lower strength layer at 20 feet was approximately 2,100 psf, which meets the design value. The lower strength zones are anticipated to be relatively thin, and other measures of shear strength indicate that the strength will meet the design value, and is not likely to lead to a bearing capacity failure. Therefore, no soil remediation is recommended at this location.

1.1.13 GEO-079

A slightly lower strength zone was identified from about 35 to 36.5 feet in the soil boring completed for investigated location GEO-079 through hand penetrometer testing. Further review of the soil boring indicates that the other SPT and hand penetrometer tests indicated that the undrained shear strength met the design value. The results of a laboratory unconfined compressive strength test indicated that the undrained shear strength within the lower strength layer was approximately 2,900 psf, which meets the design value. The lower strength zone is anticipated to be relatively thin, and other measures of shear

strength indicate that the strength will meet the design value, and is not likely to lead to a bearing capacity failure. Therefore, no soil remediation is recommended at this location.

1.1.14 GEO-082

A slightly lower strength zone was identified from about 20 to 21.5 feet in the soil boring completed for investigated location GEO-082 through hand penetrometer testing. Further review of the soil boring indicates that the other SPT and hand penetrometer tests indicated that the undrained shear strength met the design value. The lower strength zone is anticipated to be relatively thin, and other measures of shear strength indicate that the strength will meet the design value, and is not likely to lead to a bearing capacity failure. Therefore, no soil remediation is recommended at this location.

1.1.15 GEO-083

A slightly lower strength zone was identified from about 15 to 16.5 feet in the soil boring completed for investigated location GEO-083 through SPT testing. Further review of the soil boring indicates that the other SPT tests indicated that the undrained shear strength met the design value. The results of a laboratory unconfined compressive strength test indicated that the undrained shear strength just below the lower strength layer was approximately 2,300 psf, which meets the design value. The lower strength zone is anticipated to be relatively thin, and is not likely to lead to a bearing capacity failure. Therefore, no soil remediation is recommended at this location.

1.1.16 GEO-091

A slightly lower strength zone was identified from about 35 to 36.5 feet in the soil boring completed for investigated location GEO-091 through hand penetrometer testing. Further review of the soil boring indicates that the other SPT and hand penetrometer tests indicated that the undrained shear strength met the design value. The results of a laboratory unconfined compressive strength test indicated that the undrained shear strength just above the lower strength layer was approximately 3,700 psf, which meets the design value. The lower strength zone is anticipated to be relatively thin, and other measures of shear strength indicate that the strength will meet the design value, and is not likely to lead to a bearing capacity failure. Therefore, no soil remediation is recommended at this location.

1.1.17 GEO-092

A slightly lower strength zone was identified from about 45 to 46.5 feet in the soil boring completed for investigated location GEO-092 through hand penetrometer testing. Further review of the soil boring indicates that the other SPT and hand penetrometer tests indicated that the undrained shear strength met the design value. The results of a laboratory unconfined compressive strength test indicated that the undrained shear strength just below the lower strength layer was approximately 2,500 psf, which meets the design value. The lower strength zone is anticipated to be relatively thin, and other measures of shear strength indicate that the strength will meet the design value, and is not likely to lead to a bearing capacity failure. Therefore, no soil remediation is recommended at this location.

1.1.18 GEO-093

A slightly lower strength zone was identified from about 27 to 29 feet in the soil boring completed for investigated location GEO-092 through laboratory testing. Further review of the soil boring indicates that the other SPT and hand penetrometer tests indicated that the undrained shear strength met the design value. The results of a laboratory unconfined compressive strength test indicated that the undrained shear strength in that interval was approximately 1,000 psf, which does not meet the design value. Barr performed a two-layer bearing capacity analysis for the weaker soils, and determined that the resultant bearing capacity would meet the design value. Therefore, no soil remediation is recommended at this location.

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Summary: Application Appendix H Part 2 electronically filed by Teresa Orahood on behalf of Dylan F. Borchers